

## PUBLIC WORKS DEPARTMENT.

## REPORT

ON

## THE GANGES CANAL

BY
CAPT. J. CROFTON, R. E.,

WJTH


ESTIMATES AND PLANS. $/ / 4 / / 5$

IN THREE VOLUMES.

VOL. I.-REPOR'T.

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# REPORT on the Ganges Canal, by Captain J. Crofton, R. E., on Special Duty, Ganges Canal,-dated 23rd November 1864. 

## Extract from the Proceedings of the Hon'ble the Lieutenant Governor, North-Western <br> Prooinces, in the Public Works Department, under date the 10th of December 1864.

Copy of a letter No. 4293, dated Roorkee, the 23rd November 1864, from the Ohief Engineer of Irrigation Works, North-Western Provinces, to the Secretary to the Government of the North-Western Provinces, in the Public Works Department.
Sir,
I have the honor to forward herewith in original Captain Crofton's letter No. 44 of this date, with his Report on the Ganges Canal works and collateral subjects, and the estimates and plans called for in letter No. 496, dated 1st February 1864, from the Secretary to Government of India, Public Works Department, to the Secratary to Government, North-Western Provinces, Public Works Department, copy of which was forwarded to this Office with your letter No. 781A, dated 1st March 1864.

2 nd.-As it is of importance that not a day should be lost in obtaining the orders of Government on Captain Crofton's project now submitted, I have judged it best to send it in at once without waiting to study it in every detail. I have therefore merely read it carefully through, but I think I may say that I entirely agree with Captain Crofton in his view of the present state of the Ganges Canal, and of the best mode of putting it into good working order, for navigation as well as for irrigation. Differences may, and no doubt will, occur in actually carrying out the proposed works, but I believe the total amount of his estimate will not be exceeded. Knowing this, Government will probably have no difficulty in giving the necessary sanction for the preparation of working drawings, and for the collection of materials at sites of works proposed to be built or altered.
$3 r d$.-Plan drawing and collection of materials would thus progress simultaneously, and on their completion we might have a working "bee," (as it is called in America), and assemble every available Engineer from the North-Western Provinces and from the Punjab. They should bring with them workmen and tools. To each Engineer, on arrival, would be allotted 1, 2, 3 or more works, according to their nature. The Engineer would study the working drawings and the work itself on the ground, and examine and check his material. As soon as this had been done, and everything reported clear, the Canal would be laid dry, and every man would go in at it. Thus emulation would give a spur to the work, and it would in all probability be got through in a minimum space of time. This arrangement is proposed merely for work below the water line, all work above the water line, or that can be executed without closing the Canal, being supposed to be done beforehand.

4th.-Captain Crofton has, in accordance with instructions conveyed in para. 2 of Government letter No. 496, prepared two alternative projects:-
I. For remodelling the present Canal, at an estimated cost of Rupees 52,68,063.

And, II. For the construction of a supplementary Canal, at an estimated cost of Rupees 81,43,858.
5th.-In the cost of Project I. is included the sum of Rupees $13,48,213$, the estimated loss to Government in Canal Revenue, in enhanced land Revenue, and in compensation to Zemindars (to enable them to dig wells) supposing the Canal to be closed for a year during the progress of the work. No allowance of this nature has been made in the case of Project II. But as that project necessarily includes the remodelling of the present Canal between Hurdwar and Roorkee, it may, I think, be doubted
whether, in a comparison of the cost of the alternative projects, we can safely calculate on being able to carry out Project II. without closing the Canal, or, in other words, whether we can fairly exclude the consideration of possible loss during the execution of that project.

6th. - However, even if we do exclude this consideration, the difference of cost of the two projects is still Rupees 28,75,795 in favor of Project $I$. This difference, in addition to the reasons adduced by Captain Crofton for preferring Project I., is, I think, decisive in its favor.

7th.-With reference to Navigation (see para. 4 of Government letter No. 496, under reply), Captain Crofton has provided for navigation in the remodelled Channels, no separate channels of any length being found necessary. He has also worked out the cost of a separate Navigation Channel from Roorkee to Cawnpore, from which the following comparison is obtained:-
$\left.\begin{array}{lrlllllll}\text { A. } & \text { Proportion of cost of Navigation arrangements } & \text { on } & \text { present } \\ \text { Channel remodelled, Roorkee to Cawnpore } & \ldots & \ldots\end{array}\right\}$ Rs. $15,71,531$
The above sums (A and C), representing proportion of cost of Navigation arrangements on present channel remodelled and on alternative line respectively, are included in the total estimated cost of Projects I. and II. as given in para. 3 of this letter.

8th.-Regarding the possibility of reinforcing the Ganges Canal in the rainy season with the waters of the Jumna river (see latter part of para. 4 of Government letter No. 496), Captain Crofton's observations show that it would not pay to take out at any place higher up the Jumna than the point where it receives the waters of the Hindun Nuddee, some 16 miles below Delhie; and he is of opinion that, if taken out there, or at any lower point, it would probably be better to keep the Canal thence derived entirely separate from the Ganges Canal, as far as irrigation is concerned. A Canal thence derived might irrigate during the rains the country between the East Kalee and the Eesun Nuddees, and it would afford a supply of water for continuing the navigation to Allahabad throughout the year.

9th.-Further remark on the project now submitted, beyond the points touched on in the Government letter under reply, appears to be unnecessary at present.

10th.-You are aware that Captain Crofton, thougb nominally reporting to this Office, has really had the undivided and independent charge and responsibility of the preparation of these projects. I may, however, express my sincere admiration of the energetic and able manner in which he has in so short a time accomplished a most arduous task. He has had to design not only an entirely new line of Canal, but also the works and the complicated managements required for remodelling an old one. The first-mentioned portion of his task is mere child's-play compared with the last. In work of this nature one moves as it were in fetters. An intimate knowledge of existing arrangements has first to be acquired down to the very smallest details, and the requisite additions and alterations have to be dovetailed in with them in such a manner that the projected work shall be capable of being carried out at the smallest possible cost to Government, and without disturbing existing irrigation. I believe no one who has not attempted work of this nature can form any idea of the amount of thought, anxiety, and real toil it involves.

11th.-Captain Crofton has recorded his cordial appreciation of the assistance afforded him by Messrs. J. Hair and G. W. Mossop, and by Lieutenant W. Shepherd, R. E., and he has detailed the work done by each. I beg to recommend them to the favorable notice of Government.

12th. -The mode in which the services of these Officers (not set free for other work) have been disposed of will be reported separately.

13th. -Mr. Kelly continues to quarry stone in the neighbourhood of Hurdwar. The quantity of serviceable stone to be had at any one place is very small, and there are not
very many places within easy reach of the works, or of water carriage where it can be had. About ten thousand $(10,000)$ cabic feet of roughly squared serviceable stone have already been landed at sites of works, and Mr. Kelly hopes to land thirty thousand ( 30,000 ) more before the lst of May next, at from 12 annas to 1 Rupee a cubic foot. Mr. Kelly also has done good work.

> I have the honor to be, \&c.,
(Signed) J. H. DYAS, Captain, R. E., Chief Engr., Irrigation Works, N. W. P.

No. 5509 A of 1864.
Order.-Ordered that a copy of the foregoing, with the Report in original, be forwarded to the Secretary to the Government of India in the Public Works Department, with the intimation that Captain Crofton has been permitted to take the plans himself to the Secretary, without any preliminary examination of them in this Office.
C. J. HODGSON, Liedt.-Col., R. E., Offg. Secy. to the Goot. of the N. W. P., in the P. W. D.
$\left.\begin{array}{c}\text { P. W. Dept., N. W. P., } \\ \text { CAMP Koomirs: } \\ \text { The 10th of December, 1864. }\end{array}\right\}$

Trom-Captan J. Crofton, R. E., On Special Duty, Ganges Canal, to Captain
J. H. Dras, R., E., Chief Engineer of Irrigation, N. W. Provinces,-(No. 44 of
1864-65.)
Dated Roorkee, 23rd November 1864.
Sir,
I have the honor to forward the Report on the Ganges Canal Works and collateral sabjects, with the estimates called for in letter No. 496, dated 1st February 1864, from the Secretary to the Government of India, to the Secretary to the Government, N. W. P., Public Works Department, forwarded with your predecessor's Docket No. 4472, dated 3rd March last.
2. On the 15th February last I was relieved by Lieatenant Moncrieff, r. в., of the charge of the Eastern Jumna Canal, having proceeded to Roorkee on the 13th, where two of the Officers, whose services had been previously placed at my disposal, Mr. J. Hair, Executive Engineer of the Bolundshuhur Division, Ganges Canal, and Lieutenant Shepherd, r. e., joined me.

As it was desirable to complete the field work before the regular setting in of the hot winds, when the accuracy of levelling operations becomes questionable, the establishments of the Ganges Canal, both European and Native, were subsequently indented on for a further supply of levellers.
3. To Mr. Hair was assigned the levelling for the alternative line from Roorkee to the Bolundshuhur Branch head, while Lieutenant Shepherd took the cross-sectioning in the Doab between the Hindun and West Kalee Nuddee. Mr. Hunt, assistant to Mr. W. Dodsworth, Supervisor of land measurements, Ganges Canal, carried on the alternative line levels from the Bolundshuhur Branch head to near Khoorja; also the trial level fröm opposite Mozuffernuggur vid Khutowlee to the Futtehgurh Branch, as well as the line for the proposed navigation channel from above Deobund to Meerut ; the remainder of both lines down to the Nanoon Regulating Bridge and that from the Futtehgurh Branch opposite Anoopshuhur were completed by Native Surveyors attached to Mr. Dodsworth's establishment. The levels from the Jumna at he head of the Eastern Jumna Canal and the Railway crossing opposite Saharunpoor, joining
on to Mr. Hair's line near Deobund, were taken by Mr. T. Jones, Assistant Engineer, Northern Division, Ganges Canal.

All this field work was completed by the latter end of April. In the mean time I had marched over the course of the proposed alternative line from RoorkeetoSirdhanna, and examined carefully the channel and works on the main line of the canal from the heads to below Allyghur, as well as the Cawnpore terminal works. The time at my disposal would not admit of a more extended inspection of the branch canals, as I was obliged to be back at Roorkee on the 4th April, for a committee on canal revenue matters previously ordered by Government.
4. In the beginning of May, we, (i. e., Mr. Hair, Lieutenant Shepherd and I) reached Mussoorie, our head quarters for the hot season; Mr. Mossop joined us on the 20th June from the Punjaub.

The Ganges Canal was closed for repairs at the latter end of July, when I accompanied the late Superintendent General Colonel Turnbull, in his inspection of the works from Roorkee to Sirdhanna, and the last seven miles of the Cawnpore Branch, and subsequently examined the line between Myapoor and Roorkee. The submission of these projects has been somewhat delayed by the late closure of the canal, as it was impossible to settle the detail of alterations required for remodelling without knowing the actual condition of the works below the water line. Some of the details of information gathered by the Executive Officers, during the closure, have only come to hand within the last fortnight.
5. The levels taken specially for the projects now submitted have all been plotted on a scale of 1 mile to theinch, and put together on sheets of double elephant paper. They are shewn on a reduced scale in Plan No. II., which has been compiled chiefly from the "Atlas Sheets" of India and the Revenue Survey district maps. The other maps are compiled from this or existing records in the Canal Office at Roorkee.
6. The sections of the Ganges shown in Plan VIII. at Sookertal and Bysoomah, were taken, the former by Mr. Hair, the latter by Mr. Hunt; those at Gurmooktesur and Anoopshuhur, as well as all on the Jumna, were obtained from the Railway Department.
7. It only remains for me to state briefly the part taken by each in the preparation of the projects. Each surveyor or leveller plotted his own work. To Mr. Mossop is due the drawing up of the sections and compilation of most of them, as well as the three Maps which head the list.

The whole of the estimating was done by Mr. Hair and Lieutenant Shepherd, to whom the plans, and several of the designs of the Masonry works, are also due. To all three gentlemen every credit is to be given, for the very efficient manner in which they have performed their share of the work ; Mr. Hair's engineering experience, gained both in India and England, has been especially valuable in working out the detail of the projected works and alterations. Without the incessant labour and attention they have given to it, the work, I have no hesitation in saying, could not have been completed for months to come.

The Officers of the Ganges Canal have contributed their quota of aid throughout the investigation. To them, and to our late ever-to-be-lamented Chief, Colonel A. D. Turnbull, much of the information, and several useful suggestions contained in the following Report, are due.

I have the honor to be,

$$
\mathrm{Sir},
$$

Your most obedient Servant,
J. CROFTON, Captain, в. в.,

On Special Duty, Ganges Canal.

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## REPORT.

1. Tre subjects to be taken up in this Paper will be found briefly described in the accompanying Memorandum (Appendix A), forwarded for guidance by the Government of India. They may be classed generally under four heads.

1st. The possibility of rectifying existing evils on the Ganges Canal by alterations and additions to the present channels and masonry works.

2nd. The comparative advantages and disadvantages of effecting this by remodelling the present line in the Ganges Khadir, as far as Roorkee, where the Canal enters the bangur or high land of the Doab, thence dividing the supply as suggested by Sir Proby Cautley, and carrying a certain proportion of it in a new channel to run to the westward between the Hindun and West Kalee Nuddee, which should reunite with the original line at some point lower down its course, beyond which any alterations to existing works could be effected without much difficulty.

3rd. Whether a separate Canal for navigation only would be preferable to adapting the works of irrigating channels to that purpose ; and-

4th. The feasibility of drawing an extra supply of water from the Jumna for an autumn crop, during the months when its volume is in excess of the requirements of the Eastern and Western Jumna Canals.
2. Before entering into the details of any of these questions, it will be advisable to describe briefly the condition of the Canal as it is.

The Tabular Statement (Appendix B) and the longitudinal sections (Sheets 1 to 20 Plan IV), together with the general plans of the course of the Canal, will, it is believed, supply all further minutiæ necessary to a clear understanding of the state of the works. In the longitudinal sections, the ground line shewn is that of the natural surface at the time of excavation of the channel ; the straight black lines below, connecting the floorings of the bridges, show the level of the bed as originally excavated, the present level of the lowest line of channel opposite milestones and at bridges and falls being marked by an uneven black line either above or below this, as silting or erosion has taken place. Sir Proby Cautley's Report of 1860 (hereafter referred to as the "Ganges Canal Report") gives such ample detail of the existing masonry works that plans of any of them to accompany this Paper have been considered unnecessary, except for the purpose of elucidating extensive proposed alterations. All levels are reduced from a datum 200 feet above the flooring of the Myapoor Regulating Bridge, the original zero from which the levels in the Ganges Canal Report were calculated; the reason of this alteration being to admit of the levels of any part of the Doab up to the base of the hills being reduced to a common datum line without the introduction of

Subjects of the Report.

Present condition of Canal to be first described.
plus or minus signs, which might possibly lead to errors hereafter. This datum, by connections made here and there with permanent benchmarks, is $1121 \cdot 37$ feet above that of the series of levels brought up from the mean sea level at Kurrachee by the officers of the Great Trigonometrical Survey.

Head Works on the Ganges not included.
3. The Bridge at Myapoor below Hurdwar being the present regulator for admission of the supply of water, and the temporary arrangement for drawing it from the Ganges being sufficient for existing requirements, no remarks will be offered here on the channels above that point. Alterations to them or the Escape Dan attached to the Bridge will be more appropriately considered in conjunction with the question of a permanent Dam across the river, for which we have not sufficient data at present.

## STATE OF WORKS FROM HEAD TO ROORKEE.

Channel from Myapoor to Roorkee.
4. From Myapoor to Kunkhul Bridge there has been little erosion on either bed or sides, the soil being chiefly shingle and earth. From this point to the Rutmoo torrent at the 13th mile, the soil is more or less mixed with sand, and deepening and widening has taken place extensively in parts, such as between Jowalapoor Bridge and the Upper Bahadoorabad Fall, and between the Puttree and Rutmoo Torrents; the channelling out here varies from 3 to 5 feet in depth, and in places the berm on both sides has been entirely cut away. From the Rutmoo to Muhewur Bridge,-the commencement of the Solanee Valley Aqueduct-the channelling and side cutting has been very trifing, the soil of the ridge through which the line passes being generally of a stiff tenacious clay. The bed of the Solanee Aqueduct, between the masonry revetments from Muhewur to the Roorkee Bridge, has been deepened on an average $3 \frac{1}{2}$ feet (vide annexed sketches.) The erosion on this portion of the line was at one time much more serious when the supply of water passing down was considerably larger than during the last two years; the deepest holes appear to have been gradually silted up since then.

The berms or towpaths between the head at Myapoor and the Rutmoo Torrent do not appear to have been originally excavated to a uniform height above the bed; where the depth of excavation was more than 12 feet,-the minimum height of the towpath above the bed as originally determined, - they were formed on the level of the natural surface. Between the Jowalapoor Bridge and the Puttree Works this was of no consequence, navigation being provided for here in a separate channel; but the irregularity of height of towing track above the surface of water cannot fail to be, and is, a hindrance to the traffic. It was done solely from economical motives, as Sir P. T. Cautley has explained with reference to this and other portions of the Canal in his Report. From the Rutmoo to the Solavee Valley Aqueduct revetments, the berms are maintained at a uniform level of 12 feet above the bed. Holes have been eroded in the bed below the Jowalapoor Bridge and all the falls to the depthsshewn in the section.


In one instance, at the Upper Bahadoorabad Fall, the erosion extends considerably below the bottom level of the foundations; no injury, however, has resulted from this to any of the masonry works, the talus of boulders and crib work originally attached to each on the down stream side having transferred the excessive action of the current to a sufficient distance from the works themselves.
5. The four overfalls on this section have suffered little injury, except in the brick on edge coverings on and at the foot of the ogees, where the action of the stream is most violent. This, in the falls at the Ranipoor and Puttree Works, has been stripped off to a considerable extent ; the soundness of the brickwork (first-rate along this part of the Canal) has, however, prevented any dangerous results hitherto ; the remainder of these massive works are in capital order, and their action in passing off the floods of the torrents has, up to the present time, been most satisfactory. Silt has been deposited to some extent on the upper channel of both (most on the Ranipoor super-passage). No retrogression of level whatever has taken place in the beds of the torrents below.
6. At the Rutmoo, the Escape Dam is in fair working order ; below it, however, most serious retrogression of the levels of the torrent bed has been from the first taking place, and the Canal Works have only been preserved hitherto by temporary overfalls of piles and boulder work constructed, from time to time across the bed below, as shewn in the annexed sketch. In lieu of these a design for a permanent masonry overfall was, some months ago, submitted for the sanction of Government ; its construction -would probably secure the works from all future risk, and save any further outlay on this, the most troublesome and dangerous, portion of the Ganges Valley Works.

The Inlet Dam has been out of use for some time past, the water of the Canal being permitted to extend up the bed of the torrent : no inundation of the low lying adjacent lands has hitherto resulted from this, and it is clear from the longitudinal section along the banks that the surface level of the water at the Inlet Dam might be raised to 10 feet, the original full supply mark, without any risk of flooding the valley, which is similar in configuration to those of most siltbearing streams, where the level of the banks on either side falls away from the channel. The inundation, which usually. covers a large portion of the low lands in the valley in the rainy season, is caused by the floods of the torrent overflowing the banks at points considerably above the highest limit to which the Canal water at full supply would extend. The masonry passage for carrying drainage from the bed of the torrent above the Inlet Dam under the Regulating Bridge has, consequently, been closed for some time past. Much silt is constantly deposited in the bed of the torrent above the Inlet Dam owing to the diminution of waterway there, the effect of which is, and has been, very injurious, by contracting the channel of approach, and thus heading back the floods which have, on more occasions than one, cut into the banks of the torrents to a dangerous extent, and inundated a large area of the valley on the right bank of the Canal. It has also

Overfalls above the Rutmoo.

Rutmoo Works.
tended to concentrate the action on the tail flooring of the Escape Dam, by increasing the height of fall there in the surface of the flood waters. The highest flood yet observed has not risen above the level of the top of the Dam piers (or 10 feet above the Canal bed) except on occasions when the Canal also is running full ; the water has then been known to rise 4 feet over the Inlet piers.

The Sluice apparatus, on the Regulating Bridge below the Dam, seems to require some modification; experience has shown that the width of the gates and length of the sleepers ( 20 feet in the clear) is too great for economical working with reference to the depth of water to be controlled. The wooden gates in use are necessarily very heavy, the lifting gear must be correspondingly massive, and the number of men to work it large. If let fall when the supply of water is low, the vibration shakes the masonry, and, with a full supply, there is often considerable difficulty in closing the last two or three bays.

Myapoor Regulator.

Solanee Aqueduct.
7. At the Myapoor Regulating Bridge the inconvenience of the wideSluice gates has been still more felt, from the greater depth of flood water to be dealt with, and the more frequent necessity of closing ; the vibration from the fall of the gates here loosened the stone linings of the gate grooves so much that iron had some time since to be substituted, which seems to have stood fairly; the other evils, however, still remain, and unless the width be reduced, more powerful lifting machinery and stronger gates, must be substituted for those at present in use.
8. The Aqueduct over the Solanee torrent has stood well ; the state of the bed of the torrent above and below shows that the waterway under the Aqueduct is just sufficient, and no more ; there is no hole or retrogression of levels down stream, and little or no silt has been deposited except under the side arches, where a certain quantity would naturally be left on the subsidence of floods. The flooring of the Canal channel above requires some waterproof covering ; dripping still-continues through the arches, though less than at first, the effect of which has been to loosen a considerable surface of the outside plaster, and here and there bricks of an inferior description (of which it is next to impossible to prevent a few finding their way into so great a mass of brickwork) may be seen slowly decomposing from the same cause. It was at one time supposed that the pores of the brickwork would gradually fill up and so stop the percolation, but the fact is, that even the very best of brickwork is of too absorbent a nature to be proof by itself against the constant pressure of a head of water even much less than that passing over this Aqueduct ; the bricks too vary in quality according to the mode of burning ; in some experiments made a few years ago on the Canal works in the Punjab, first class bricks burnt in the Native kiln were found to absorb one-thirteenth of their weight of water, while bricks of similar appearance burnt in " flame" kilns, in which the fuel was wood, took up as much as one-seventh of their weight. Sir P. T. Cautley anticipated the evil, and suggested the measures to be adopted as time

LONGITUDINAL SECTION OF
SOLANEE VALLEY Aqueduct.


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and opportunity might offer (vide Ganges Canal Report, Vol. II., payes 518 and 521). The masonry revetments or ghats, which form the sides of the channel along the raised earthen embankment across the valley, do not call for much remark ; a breach occurred some years since along a short portion of the right bank ghat, from the heeling over inwards (towards the channel) of the wall A (vide annexed sketch) from a point some four or five feet below the level of the bed; the channel here had been considerably deepened, and the accident occurred just after the Canal had been laid dry ; the outer wall B was very little, if at all, affected. From the investigation made immediately after its occurrence, it appears to have been caused by the pressure of the earth filling between the walls which had become saturated by the percolation through the brickwork; when the counteracting pressure of the water in the Canal was removed, the thin wall gave way, there being no weep holes through it by which the drainage from the backing could find an exit. The bed all along between these revetments was to have been protected by a layer of boulders (vide Ganges Canal Report, Vol. II., page 453): this, however, has been deferred from economical notives, the actual protecting work being confined to a sloping talus of boulders and brick kiln rubbish, thrown down from time to time along the foot of the ghats.
9. The Bridges from the head to Roorkee are all in good order. The towing track is carried under the left side arch of each, where the navigation is carried on in the main channels, either by arched masonry passages, or, as at the Peeran Kuliar Bridge, by temporary wooden scaffulding.
10. The navigable channel from the Jowalapoor Bridge to the Puttree does not work satisfactorily, from the quantity of silt constantly being deposited in it, especially on the Reach between the lower Bahadoorabad and Puttree Locks; the velocity of current is necessarily less than that in the main channel causing the deposition of the silt which more or less enters with the supply at Jowalapoor Bridge ; the berm, originally excavated to a suitable level, is in places entirely cut away; in others, the encroachments of weeds have narrowed the channel, so that a single boat has hardly room to pass.
11. The headway for the passage of boats along this portion of the Canal was limited to 5 feet by the height under the arches of the Rutmoo Regulating Bridge, supposing the depth of water at full supply to be 10 feet as originally calculated; elsewhere the headway varies from $5 \cdot 34$ under the Puttree super-passage to 12 feet under the Bridges generally; the Bridge at Peeran Kuliar is, from the accident of its position, exceptionally high. The available headway at each work is obtainable from the tabular Statement (Appendix B) by subtracting the full supply depth in Canal from the total height of soffit of arches above the floorings.
12. The embankment works and cuts for controlling the vagaries of the hill torrents have worked, on the whole, most satisfactorily; breaches and consequent inundation of the adjacent lowlands liave, it is true, occurred more than once in the Rutmoo and Solanee Valleys, but these have been due to want

Bridges above Roorkee.

Bahadoorabad Lock Channel.

Available Headway.

Hill torrent embankments.
of timely repairs or trifling additions, which subsequent experience shewed to be necessary, to the original bunds and spurs. Careful watching can never be dispensed with over works of this nature, and a comparatively trifling expenditure on timely repairs and small additions, as may appear necessary from time to time, will prove to be the best economy in the long run.

## CONDITION OF WORKS FROM ROORKEE TO CAWNPORE AND ETAWAH BRANCH HEADS.

Channel from Roorkee
13. These 162 miles comprise the remaining portion of the main line proper, and it is here that the works have suffered most. The nature of the country traversed by the Canal has been so minutely described in the Ganges Canal Report (Vol. I, page 191 and passim) that a very few remarks on it will suffice here. From Roorkee to the neigLbourhood of Sirdhanna the soil is more or less sandy ; ridges of light sand cross the line near Guneshpoor, Toghulpoor and Bailra Bridges, and again a little below the Futtehgurh Branch Head, aud near the Khutowlee and Aternah Bridges; for some distance above and below the Chitowra and Sulawur Falls there is a large admixture of clay. About the Nanoon Bridge kunkur appears with the clay and sand, and from this to the Branch head below Allyghur the soil is fair, with occasional short intervals of light sand as at the Boolundshuhur Branch head, the Wallipoor and Urowlee Bridges, and again for some distance above the Cawnpore Branch head. "Oosur" makes its appearance about the Simra Falls, but to no great extent. The longitudinal section shows that erosion has taken place in the bed, more or less, all along the line ; between Hafiznuggur and Nirgajnee the depth of channelling is greatest, in some parts as much as 4 or 5 feet below the original bed, and this extends, with little diminution, as far as Chitowrah. Again, for some distance below the Sulawur Falls, the depth has bean increased by 3 feet and upwards. In the more sandy tracts, it will be observed that the declivity of the new bed is tolerably uniform, but the fall is less than that of the original channel. The width has altered little except through the sand hills; the edge of the towpath generally remains intact, the slopes abraded to a nearly perpendicular section. The annexed diagrams show the actual sections of channel at various points compared with those originally excavated. Other sectious have been taken at each milestone for a considerable distance from the heads, to furnish data for estimating, but 1 have considered it needless to encumber this Report with them. Wherever any cousideruble widening of the channels has taken place the bed is Ligher than either above or below; the most remarkable iustances of this occur at the sand hills above the Toghulpoor Bridge and a mile below the Futtehguih Branch Head, sketches of which are annexed. This natural formation gives, I think, a useful hint in projecting artificial channels, which will be again referred to in considering the measures to be adopted tor remodelling.

Below every fall the channel is widened for a considerable distance; in some cases, not only has the berm entirely disappeared, but a large width of the spoil banks has also been carried away.

CROSS SECTIONS
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## CROSS SECTIONS uf the <br> GANGES CANAL

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VII


CROSS SECTIONS
OF THE
GANGES CANAL

## Scales <br> Horisantal - sce to $)$ <br> Vertical $=200$ te 1 <br> SECTIONS <br> socc riut ibbove Bulrak sidac



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## GANGES CANAL

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GANGESCANAL



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Below the Simra Falls, and especially on the last few miles abuve the Cawnpore Branch Head, the berm is extensively cut into, in some places swept away quite up to the edge of the road bank.

The excavation on the portion from Roorkee to Sirdhanna as described in Ganges Canal Report (Vol. I, pages 191 and passim) trenches uniformly on the substratum of light sand; below this long intervals of firm clay are met with, as shewn in the sections at Newaree and Dumkoura Bridges, and here and there beds of kunkur. The velocity of current has been to, grent to admit of any silt deposits in the channel from the heads to the Cawnpore and Etawah Branch Regulator at the 181st mile.

Very deep holes have been formed, as the longitudinal section will show, below all the masonry works in the sandy tracts ; in some instances, as at the Hafiznuggur Falls and the Bailra Bridge, extending several feet lower than the bottom of the foundations, but, as was observed before with regard to the line on the Ganges Khadir, in no case has this affected the stability of the masonry works ; the talus of boulders or kunkur has everywhere proved its efficiency as a means of protection.
The berms or towpaths from Roorkee to the Boolundshuhur Branch Head at the 110th mile were not originally formed at a uniform height above the calculated full supply level of the Canal (Ganges Canal Report, Vol. I, page 240). In some places, as near Khutowlee, the height is much too great for towing. This, and the general condition of disrepair into which they have been suffered to fall from economical motives, makes tracking difficult with human labour, and quite impossible with cattle. Below the Boolundshuhur Branch Head the towpaths are at a uniform height above the bed, but there are the same obstructions as above from the cutting away and ravines in the sides all the way down.
14. The lock channels round the falls have nowhere worked satisfactorily from the constant tendency to silt up; this in the reaches above the locks might be obviated by keeping the lock gates or sluices, when not in use, constantly open (or partially so) so as to cause a scour along the channel ; but this would affect the working of the mills attached to the locks by lowering the head of water ; below the locks silt is deposited with a full supply in the Canal by the action of the backwater; with a low supply the velocity is excessive, causing considerable erosion irom both bed and banks ; the deepening of the bed of the main Canal has led to another evil with a low supply, by diminishing the depth of water at the extremities of the lock channels; boats are constantly obstructed in the upward passage by grounding at these puints.
15. The three escapcs on the upper portion of the line at Khutowlee, Aboo Nulla and Jannee Khoord have, for some time past, been practically useless from the accumulations of silt at their heads ; those at Moonda Khera and Kasimpoor work very fairly.
16. The Bridges are throughont in good order; where erosion seemed to be taking place on the down stream side to

Escape Heads.

Bridges.

## Branch Heads.

Locks.
a dangerous extent the boulder or kunkur protective talus has been supplement.d by piling and cribwork across the bed; no signs of settlement have appeared, nor have the floorings been injured in any way hitherto. The annexed sketches of one or two bridges will sufficiently illustrate the state of matters in these cases.
17. At the Regulating Bridges at the bifurcations of the branchea, the same difficulties have been experienced in working the sluice machinery as in the case of the Rutmoo Bridge described above; the width of the gates is inconveniently great for the depth of water to be dammed up; where sleepers are employed a section of eight inches square of salwood is not found sufficient to stand the pressure without bending.
18. The locks are generally in fair working order; the gate and sluice work here and there seems to require some modifications to ensure perfect efficiency : the wooden sluices or stop-cocks for filling and emptying the chambers, the idea of which was taken from the Italian works (Ganges Canal Report, Vol II, page 303) do not answer well with water soloaded with silt, as is always the case more or less in these Canals; they are constantly getting jammed up, and the silt is with difficulty removed unless the water in the chamber falls below the lower outlet.

The masonry passages at the heads of the lock channels have been found to be great obstructions to boats entering from the Canal ; their width is the same as that of the lock chambers to which the loading of the boats is consequently very closely adapted ; where the velocity of the boat is very small, as in passing through locks, this is of no consequence, but it is obvious that a much larger margin is necessary where that velocity is considerable, and the direction of the current, as at the entrances of these lock channels, is not in the central line of the stream. In one case, where the left revetment of the passage has been removed, the increased facility of entrance is evident. Boat Bridges across the main channel immediately below these entrances were projected by Sir P. T. Cautley vide Ganges Canal Report, Vol II, page 297) to prevent any risk of buats or rafts being carried down to the falls. Since the heading up of the water there however, by which the acceleration of velocity in the channels above the falls has been neutralised, at least so far up as the entrances to the lock channels, the necessity was considered to have passed away, and their construction has hitherto been held in abeyance. A boat is said once to have been allowed to go over one of the falls, but whether this occurred before or subsequent to the raising of the crests of the falls I know not. That boats can, with perfect safety, be brought down close to the present falls is shewn by the fact of a portion of the material for the repairs of a fall some months ago being thus carried to within 150 or 200 feet of the crest.
19. The headway at full supply on this portion of the line varied from 12 feet in some of the bridges above Sirdhanna to 5.0 feet under the lock bridge opposite the Pulra falls. Towing paths have been constructed under the left arches of all the bridges from Roorkee to Sirdhanna, the 75 th mile (inclusive) ; below this the lowness of the crowns and form of the arches


would not admit of it, and the communication along the towing path is kept up as described in the Ganges Canal Report (Vol II, page 319 and passim) by flank arches under the roadway behind the abutment. This, if the navigation is to be carried on by tracking, involves the necessity of unhooking the rope from the towing cattle at each Bridge, or at intervals of every two or three miles, often less; whatever arrangements might be adopted for carrying the rope under the Bridge, one of which was suggested by Sir P.T. Cautley in the passage above quoted, the delay thus caused must always be, and is, a great hindrance to free navigation ; the inconvenience is still more felt now, no method of passing the rope under any of the Bridges having been yet brought into use.
20. The falls on this section are ten in number (For their Overalls Muhmoodwaterways, \&c., vide Appendix B.) viz.:-

1. Hafiznuggur (or Asoffnuggur).
2. Muhmoodpore.
3. Nirgajnee (or Bailra).
4. Jaolee.
5. Chitoura.
6. Sulawur.
7. Bhola.
8. Mussoorie (or Dasna).
9. Pulra.
10. Simra (or Jooeon).

The first seven have suffered injury, more or less severe, in their lower floorings from the action of the water; in oue or two cases the brick on edge covering to the ogees was stripped off, but timely repairs and protection saved the evil from spreading. Not to encumber this Report too much with detail, a few general remarks on each, and an account of the present state of and injuries sustained by one, will, I think, answer every purpose. In the annexed diagram is shewn a plan and section of the Muhmoodpoor Fall, where the action of the water appears to have been most violent, and the injuries to masonry and channel most serious. The flooring in all the chambers was more or less extensively ploughed up ; in the left chamber the floor, the least thickness of which is 6 feet, was cut quite through, and a hole some 15 feet deep eroded in the sandy substratum; the brickwork under the brick on edge covering was found to be very inferior, some of it so bad as to be dug out without difficulty by a phaorah; the masonry of the ogees was comparatively little injured, owing to its better construction and the less violent action of the water there; in places, however, the floor at the top of the ogees, where the water falls over the temporary weir from a height of 4 or 5 feet, was stripped of its brick on edge covering. These latter injuries were repaired by a fresh coating of brick on edge, covered with planking, to protect the green masonry from the scour of the water. The holes in the flooring were filled up with hard material, on which was laid sound brickwork, sandstone from the Delhi quarries forming the covering over all in the most exposed portions, the rest being faced with brick on edge as before, on which were fastened down bars of wood and iron to secure it from being torn up before the mortar had time to harden
properly, as it was necessary to turn the water into the Canal with as little delay as possible after the completion of the repairs; with the view of diminishing the scour over the injured parts, temporary weirs were constructed, as shewn in the diagram, across and close to the extremity of the flooring. These repairs were executed about the close of the year 1862, and have stood, with a few comparatively trifling additions, up to the present date. The hole in the bed below the tail revetments is, as will be seen by a reference to the longitudinal section, somewhat deeper than the lower foundations of the work, though the well foundations were here carried down to a depth of 32 feet below the bed level. With the view of preventing further erosion so close to the work, a temporary weir was thrown across the channel a few hundred feet lower down; this has apparently answered its purpose for the present, though matters are still in a very unsatisfactory state here, as in some of the other falls. The curved terminations to the tail revetments, adopted in this and all the other falls except Nirgajnee, have not fulfilled the projector's expectation (vide description in Ganges Canal Report, Vol. II., page 169); they cause apparently violent eddies, by which the bed and banks below are in every case extensively eroded.
21. The Hafiznuggur Falls were damaged about the same period similarly to those above described, but to a comparatively trifling extent; the brickwork on the injured floorings was renewed, and the work, at the closure in August last, appeared in fair order. The action of the water here is fully as violent as at Muhmoodpoor, but the masonry is very superior, especially the brick on edge covering. The cutting away of the sides of the channel below the tail walls is more extensive than at any fall except Muhmoodpoor; and the channelling out of the bed appears, from the longitudinal section (Plan IV.) to be the deepest along the whole line.

The Nirgajnee Fall is in better condition than any of the first seven, owing doubtless to the greater depth of backwater on the flooring, which is due to the very trifing extent of channelling down stream, where the bed is on a stratum of clay, and the extra width to which the channel was excavated originally. The terminations of the tail revetments were here formed in a curve outwards, instead of towards the centre of the stream as in the other overfalls, thus forming a large basin, in which the accelerated velocity of the water shot down the ogee fall is destroyed, and the boils and eddies lose themselves.

That the back water, and that alone, has been the salvation of this fall is evident, for the brickwork is very inferior, especially that on the ogee curves.

The injuries to the Jaolee Falls were little less serious than to those at Muhmoodpoor, and an additional difficulty presented itself in executing the repairs in the height of the springs above the lerel of the lower flooring. The brick on edge covering was in places bulged upwards as if blown up from below, and water was seen to spirt through the side walls 3 or 4 feet above the flooring; in making the repairs, small holes were left through the thickness of the floor for the escape of these springs, which has apparently stopped
the tendency to blow up. No issue of water from the side walls was apparent when I saw the work at the commencement of the late closure in August ; shortly afterwards, however, I was informed by the officer in charge, springs showed themselves in the old spots, owing probably to heavy and continued rain. The covering of the injured flooring is for the most part of Delhi sandstone carefully laid, yet, spite of every care in the construction and selection of the stones, blocks weighing from 13 to 15 cwt . were washed out when the water was re-admitted into the Canal last September. This has occurred here as well as at other falls before, and in every instance when the supply of water in the Canal was less than usual.

The Chitowrah Falls seemed to stand fairly until just hefore the last closure, when the flooring in the left chamber gave way; and, on the Canal being laid dry, a large hole was found right through the masonry. The old brickwork of this fall is little better than at Muhmoodpoor; in fact, from Muhmoodpoor to Sulawur inclusive, the falls seem to have. been constructed with less care than any other works on the Canal.

The floorings at Sulawur have long been in a damaged state; in the closure of April 1863, there was not time for completing the repairs, and blocks of stone were thrown in loosely, which, with few exceptions, were found to have remained unmoved when the Canal was again laid dry in August last. From the diminished volume of water to be dealt with here, however, and the better quality of soil in the channel, the injuries do not seem to have extended. A few blocks of stone, as at Jaolee, were displaced at the point of greatest scour, and turned over on the top of the flooring below. This stone covering was relaid last August. The brick on edge covering in this fall was very inferior indeed; some of the mortar joints on the ogees were so wide as to admit of a hand being passed with ease between the bricks, nor was the mortar even of tolerably fair quality.
At Bhola the injuries to the floorings were never of much consequence; comparatively trifling repairs have sufficed to keep the works efficient.
The three remaining falls at Mussoorie, Pulra and Simra have remained in good order from the first, owing chiefly to the diminished volume of water passing over them, but also to the better quality of the workmanship.

## WORKS ON CAWNPORE BRANCH.

22. The longitudinal section shows erosion in the bed more or less from the head to the 60th mile, but nowhere to any serious extent. Below this, to the Cawnpore lockage, silt is deposited generally; on many of the bridge floorings to a depth of two feet. The channel here and watercourses are the receptacles for the silt deposits brought down from the upper portion of the Canal ; many of the latter, even at this early period of their existence, are lined for some distance from their heads by large ridges of silt, the result of the

Channel.

Masonry Works.

Available Headway.
necessarily frequent periodical clearances. A fall constructed in the flooring of the Keylunpoor Bridge about 3 miles from the head, has been the cause of much erosion from bed and banks both above and below. With the view of lessening the evil, weirs were built some time since across the floorings of the three bridges next in order, of heights proportioned to the distances of each and the fall to be overcome, thus distributing the fall over a greater length. This, however, though supplemented by lines of piling at intervals across the bed, has only had the effect of reducing the action of the water at the Keylunpoor Bridge ; the velocity of the current is still greater than bed and banks can stand, besides being a great obstruction to the navigation.

The berms or towpaths along a large proportion of the line, especially in the "Oosur" soil, need repairs much if they are to be used for tracking.
23. The masonry works throughout are in good condition. The lower chamber of the Cawnpore lockage nearest to the river, for some time subsequent to the completion of the works, gave much trouble, from the difficulty of working hinged gates where silt was constantly being deposited to a considerable depth on the floorings. The adoption of vertical iron drop gates has, however, effectually obviated the difficulty ; and for the past year or so there has been no obstruction to the passage of rafts or boats.
24. The headway at full supply under the bridges on the line varied originally from $7 \frac{1}{2}$ to 6 feet. This was considerably diminished at several points on the lower portions by the silt deposits on the bed ; as, for instance, at the Bhawunt and Aima Bridges, where the water, I was informed by the late Executive Engineer, Mr. Anderson, has often been known to stand 8 feet above the floorings, thus leaving only $5 \frac{1}{2}$ feet clear headway under the arches. The Aima Bridge in particular proved so great an obstruction to the passage of laden boats, that it was lately found necessary to remove the brick arches, and substitute iron girders at a. higher elevation.

## ETAWAH BRANCH.

25. The declivity of the bed on this Branch was originally projected (vide Ganges Canal Report, Vol. II, page 377 ) at 1 in 4224 (or $1-25$ feet per mile) from the head to the 54th mile ; below this to the terminal lockage, at 1 in 5280 ; the superfluous fall in the ground surface being disposed of by an 8 feet overfall at the Nubbeepoor Bridge, mile 154 (vide Ganges Canal Report, Vol. II, page 375, and Atlas Plate V). It has been excavated, however, on declivities somewhat greater than these, and the overfall at Nubbeepoor dispensed with, the flooring of the present bridge being 5.5 feet lower than the crest of the projected overfall. The section for the first 51 miles shows trifing erosion in the bed, below which to the end silt is more or less deposited ; in some places the depth of the deposit is as much as 2.8 feet. The berms below the Gikror Escape, 57 th mile, are in fair order ; above that they have been cut
into here and there. About five-sixths of the calculated full supply appears to be the maximum which has hitherto been admitted, and this irregularly, alternating week about with the Cawnpore Branch, as the supply in the Main Canal which reaches the Branch Head at Nanoon is seldom sufficient, in the height of the irrigating season, to give their full allowance to both branches simultaneously.
The works (both excavation and masonry) are complete down to the Kerown Escape, 170th mile. The channel has been excavated for three miles further to the head of the terminal lockage, which has been commenced. Irrigation, however, has not yet been much developed below the 119th mile (Devipoora Bridge), and at the Jheenjuck Bridge, 131st mile, ceases entirely.
Holes of considerable depth have been eroded below some of the bridges on the upper portion ; but as the bed has not been deepened generally for any length, undermining of the foundations may be effectually prevented by a short talus of kunkur blocks.
26. The obstructions to navigation in absence of towpaths under bridges, insufficient headway, \&c., are similar to those on the Cawnpore Branch.

## FUTTEHGURII BRANCH.

27. The works on this Branch have been completed, with the exception of locks, only as far as the 40th mile; below this, as far as the 80th mile opposite Anoopshuhur, works are in progress. All were reported by the Executive Engineer, Mr. Parker, in good order, at the date of the closure of the Canal in August last. The supply of water hitherto admitted being only sufficient for the irrigation down to the 40th mile, none of the works have yet been fully tested. The first 17 miles from the head are excavated in soil similar to that above Sirdhanna on the main line ; it is therefore tolerably certain that erosion of bed and banks will take place along this portion when the full supply is admitted.

## KOEL AND BOOLUNDSHUHUR BRANCHES.

28. Of the two remaining branches originally projected, the Koel has not yet been commenced; the Boolundshuhur has been completed for eleven miles as at first designed, below which it has been divided into two forks, each about 17 miles in length as finished up to date; these forks, however, can hardly be termed Branch Canals; they are rather large rajbuhas or main water-courses.
29. Now as to the causes of the defective state of the works on the main line, the primary one has been, and is, undoubtedly the excessive velocity of the current, owing to the original projection of the bed on too great a declivity; and in the carlier period of the

Boolundshuhur and Koel Branches.

Cause of defective condition of Channels.

Canal's existence, to the absence of means for counteracting the accelerated velocity caused by the undue depression of the surface of the water at the overfalls. The normal mean velocities of the current, where unaffected by overialls, varied as originally calculated (vide Ganges Canal Report, Vol. I, page 243) from 4.04 to 3.53 feet per second. In observations for discharge taken by myself last spring at various points along the line, I found mean velocities varying from $4 \cdot 7$ feet per second below the Myapore Regulating Head to 3 feet at Ruorkee; and in the central portions, and even as far down as the head of the Cawnpore and Etawah Branches at the 181st mile, 4 feet and upwards per second (vide cross sections at para. 13); and this with only about 2 -3rds of the full supply of water in the Canal; these are evidently greater than the soil, more or less sandy, can stand without erosion, and the results are evident in the extensive channelling out of the bed, and, in some parts, widening of the channel. Sir. P. T. Cautley anticipated the occurrence of this channelling to some extent (vide Ganges Canal Report, Vol. II, pages 158 and passim) and his idea ras, that the bridge floorings would "act as retainers of the slopes so that were a retrogression of levels equal to the whole amount of the fall between two bridges to take place, the maximum declivity upon the flooring could not exceed 3.75 feet," these bars being situated at intervals of three miles or less. Bars across the bed at sufficiently short intervals would doubtless prevent erosion, but in this case it is clear that the bridges are too far apart for the parpose. A reference to the longitudinal section (Plate IV), will show that the bed has been ehannelled out in the sandy tracts on a tolerably uniform deolivity, but less than that originally projected, from the foot of one fall to the crest of the next, and that the bridge floorings project from this bed without affecting its general level : the observed consequence of which is that the surface of the stream all along is depressed below its normal level : the velocity of the carrent is uniform except quite close to the bridges where the raised floorings act as weirs, heading up the water slightly and causing a rush through, which entails in some cases very violent action on the bed immediately down stream. The heading up, which produces accelerated velocity through the arches, is very slight in every case; at some bridges it can hardly be detected by the levelling instrument.

When the Canal was first opened the water was allowed to pass freely over the crests of the overfalls which were laid on the level of the bed of the earthen channel ; erosion of bed and sides for some miles up rapidly followed, and it soon became apparent that means must be adopted for raising the surface of the stream at these points; planks were accordingly fixed in the grooves above the bridge arches, or temporary weirs formed, over which the water was allowed to fall ; in some cases the surfaee of the water was thus raised considerably above its normal height, causing a backwater in the channel above, as in the reach from the Rutmoo Regulating Bridge to the Hafiznuggur Falls, where the water stands on the three bridges above the Solanee Aque-


Height of top of planking above crest of Fall
$\ln$ bay $N!\quad I=2 \cdot 5$
. . $\quad I I=3 \cdot \theta$
. . $\quad \boldsymbol{I I}=2.4$
$I V=1 \cdot 8$
$V=3 \cdot 1$
Depth of wuter above upstiresm culwoders
duct at about $6 \frac{1}{2}$ feet at the same time that the guage at Roorkee shows 7 feet, and that above the Hafiznuggur Falls $7 \cdot 8$; this, however, has been beneficial in diminishing the velocity of current due to the excessive declivity of the bed for a considerable distance above the overfalls. The widening of channel immediately below the falls is obviously due to the accelerated velocity there from the shoot over the ogees and deficiency of backwater.
30. The bed between the Myapoor Regulator and the of Channel from MyaKhunkul Bridge was projected on a declivity of 1 in 2640 (or 2 feet per mile), the velocity due to which has not yet caused any serious erosion in the stiff soil there. Below Khunkul Bridge and onwards to the Ranipoor super-passage the soil is more or less light and sandy, and though the declivity of bed was here reduced to 1 in 3403 (or about 1立 feet per mile) the velocity of the current is so accelerated by the stream from above the Khunkul Bridge that bed and banks have been and are still being extensively cut into.
81. The deficiency of backwater, caused by the general depression of the surface of the stream below, on the floorings Deficiency of backwater of the overfalls, has evidently been one, if not the chief cause of their failure; the injuries have generally been observed to take place with a low supply in the Canal ; in other words, when the depth of the backwater on the fall floorings was less than usual; and in a report dated 18th of last month (September) from Lieutenant Forbes, the Superintendent of the Northern Division of the Canal, he states that "in each of the instances in which damage has occurred to the Jaolee Falls, it has invariably happened when the supply in the Canal on the Roorkee gauge has been reduced to $5 \frac{1}{2}$ feet or less; when the Canal was opened on the 27th ultimo, the supply was kept up to 7 feet, it was then lowered to 6 feet, and eventually to $5 \frac{1}{2}$ feet, and within three or four days (I write from memory) the damage took place." The Nirgajnee Fall, it has been noted above in para. 21 , is in better condition than any of the others in the sandy tract below Roorkee, though the brickwork is no better than in those which have been seriously damaged, the only difference observable in their condition being the greater depth of the backwater on the Nirgajnee flooring. The ploughing up of these floors has, with few exceptions, been observed to commence at the point of greatest scour, or meeting place of the backwater with the stream shot off the ogee curve; the shoot or scouring has undoubtedly been lessened by raising the crests of the Falls, though increased action has thereby been brought on the floors above the ogee curves; the few instances in which injuries to the masonry have resulted from this have, however, been easily repaired, and further extension prevented by a covering of wood or similar material.
To illustrate the effect of the raising of the crests in altering the velocity and direction of the falling water, and thus lessening the extent of scour on the floorings, a sketch of the Pulra Falls is annexed, showing, from actual measure-
menta, the positions of the meeting pointa of the backwater and shont from the fall in each bary, with the depths of both meanored by myself. The length of the sconr, it will be obeerven, is in inverne proportion to the height of raising of che cresta.


For instance, at the Nanoon Regulating Bridge, the depth in the main channel up stream at fuil aupply would ty cat colation be 8 feet, while that in the Cawnpore Branch inmediately below the Bridge would be 6 feet, which, as the bed of the latter is formed on a continoons declivity with the main Canal, gives a fall in the surface at that point of 2 feet.
To neutralize this no provision has been made in the construction of the Bridge, and to it mast be attribated, in great measure, the extensive erosion of the bed and banks for a considerable distance above and below, though the 9 feet fall at the Keylunpoor Bridge on the Cawnpore Branch has also had its share in producing the evil.
invering merial of 33. The use of stone or other similar hard material, in the first instance, as a covering for the brick work would, in all probability, have preserved the fall floorings from injory, but the outlay for such 2 purpose at the time of the construction of the worke, when land carriage alone was available, and the distances from which any material of the kind was procurable were long, would have been enormons, greater than the projector considered himself justified in incurring, expecially when he had had long experience of good brick work having lasted well in similar circumstances elsewhere.

To show what brick on edge facing will stand if of firstrate material and workmanship; at the Ghoona Falls, on the Eastern Jumna Canal, where the depth of fall is 8 feet, I found the velocity of the carrent on the lower flooring below the ogee curve to be 15 feet per second, and at the Belka Falls on the same Canal where the drop is 15 feet in two descents, one over an ogee carve, the other immediately in advance down an inclined plane, the velocity at the foot of the latter was found by the Superintendent, Lient. Moncrieff, to be not less than 20 feet per second.

Now the maximum velocity on the flooring of the Muhmoodpoor Falls described above, calculated from the discharge in the channel at Roorkee and the observed depth on the flooring of the fall, cannot exceed 14 feet per second. It is clenr from this, I think, that inferior workmanship and material have had a large share in the causes of injury to these falls. The greater quantity of silt in the Ganges Canal water will also account, in some measure, for the difference in the wearing qualities of the works on the two Canals.

The action of the springs at the Jaolee Falls might, perhaps, have been obviated by a layer of puddle under the masonry of the flooring.

The above remarks will, I trust, explain with sufficient clearness the nature of the defects which impair the efficiency of the Canal as an irrigating conduit ; others there are, but they are solely connected with its navigable capabilities.
34. The headway under the Bridges available for the "through" traffic is limited, it will be observed from the description of the works before given, to 5 , or at most $5 \frac{1}{2}$ feet, by the state of some of the bridges on the Cawnpore Branch. Six feet appears to have been considered the minimum limit in the construction of the works generally (vide Ganges Canal Report, Vol. II, page 73), but even this subsequent experience has proved to be too little.

The absence of towing-paths under all the bridge arches below Sirdhanna, the 75th mile on the main line, has been another source of great obstruction, as well as the irregularity of height of the towpaths or earthen berms along the line from Roorkee to the Boolundshuhur Branch Head at the ll0th mile. These last, I understand from Sir P. T. Cautley, were to have been cut down to a uniform level as time and opportunity permitted, but from motives of economy, this, as well as repairs to the existing berms the whole way from the Heads to the Cawnpore Terminus, has hitherto been held in abeyance.
35. I omitted above to mention that the erosive action was much greater some three years ago, from the larger volume of water then allowed to pass down for the relief of the parched districts, during the late terrible famine. By observations taken in the beginning of 1861 by Mr. Login, then the Superintendent of the Northern Division, and his Assistant Mr. Hair, the discharge immediately below the Myapoor Regulating Bridge amounted to 6,710 cubic feet per second, while the volume obtained by me, at the same place last spring, with the water standing at the maximum height allowed during the past two years or thereabouts, was only 4,813 cubic feet per second. The channelling out of the bed was then, in consequence, deeper than it is now. The longitudinal section does not shew the full depth to which the evil at one time extended, the bed having silted up latterly to some extent, owing to the reduced velocity generally. It has also been observed, that silt is more or less deposited in the few days previous to the closure of the Canal, when the supply is being gradually reduced.
36. The unanimous opinion of the Committee of Engineer Officers assembled in January last, to consider the question was, it will be seen from their Memorandun (Appendix A), that it is hopeless to expect a permanent rectification of the evils above enumerated without a reduction of the velocity of the current-the primary cause of those evils-to one which the soil will stand without erosion. Nor do I suppose their judgment will be questioned by any one acquainted with the present condition of the Canal.

Headway under Bridges insufficient.

Reduction of velocity the only permanent remedy.

The erosion would not so much affect the stability of the works were it confined merely to holes in the bed immediately below; a talus of hard material will, in every case as shewn by the facts, transfer the erosive action to a safe distance from the floorings; it is the channelling out of the bed on long lines which produces the dangerous action. Attempts have been made at various times to arrest this by driving lines of stakes at intervals across the bed, their tops cus off level with the original bed, but they have hitherto failed to effect the object in view, and the similar temporary expedients adopted below the floorings of the falls and bridges where undermining was apprehended, require constant and anxious care to keep them efficient.

The reduction of the velocity necessary to the efficiency of the Canal simply as a water carrying channel, will also facilitate navigation, which is now much obstructed by the excessive velocity at certain points on the main channel. In the Cawnpore Branch, except for the few miles near the Head, no complaints have been made on this score.

Another improvement of no slight importance which will undoubtedly result from this reduction of velocity is the diminution of the silt now carried down in such enormous quantities to the lower districts of the Canal. That this is chiefly, if not entirely, due to erosion from the banks and bed above, is evident from the fact that the water fur seven or eight months in the year when the largest supply is required for irrigation, enters from the Ganges at the Heads clear as crystal; by the time it reaches Roorkee, the extremity of the Ganges Valley, it has assumed a leaden colour; from Roorkee downwards it gradually becomes thicker and thicker, the colour changing to that of pea soup as it passes through the light sandy soil, nor does it appear to part with much of the matter thus taken up till it has advanced some miles down the Cawnpore and Etawah Branches.

The Futtehgurh Branch, for the first 17 miles, will, I have before observed, require some alteration to admit of the full supply being safely passed down; with this exception, remedial measures (affecting the Canal as an irrigating conduit) are necessary only on the main line from the Heads to the bifurcation below Allyghur, a distance of 181 miles.
Proposenh for rectifeation.
37. Supposing the volume of water in the Canal to remain as projected the reduction of velocity can be effected only by diminishing the declivity of the bed, or increasing the width of channel section, or by a combination of both. Above Roorkee (the 19th mile) from the difficult nature of the country and the magnitude of the works remedial measures must obviously be confined to the existing channel; below that point two courses are open, one to continue the remodelling down to the Allyghur (or Nanoon) bifurcation; the other, (suggested by Sir P. T. Cautley) to divide the supply by carrying a new channel to the westward on the water-shed between the Hindun and West Kalee Nuddee Rivers, which should rejoin the present line at some suitable point lower down its course. Sir P. T.

Cautley proposed that this alternative channel should have a capacity of 3,000 cubic feet per second, leaving 3,750 cubic feet per second (the full supply passing the Solanee Aqueduct being 6,750 cubic feet per second) to be conveyed by the present line. A nother proposition was subsequently made by Colonel Strachey to carry the alternative line across the present channel above Meerut leading it into the Futtehgurh Branch, whence it would issue lower down and rejoin the present channel above Nanoon. The principle of both, however, is the same, $i$. e., division of the supply.

It will be shewn hereafter that the present channel at Roorkee can safely carry only 1,870 cubic feet per second; if this be much exceeded, and certainly with a supply of 3,750 cubic feet per second, it would be necessary to reduce the declivity of the bed, and the number of overfalls required in consequence would be nearly the same as in remodelling the channel for the full supply, though the height of each would be less.

Now, as the main difficulty and expense of the construction of such works with a running Canal are in the laying of the foundations, it appears to me unadvisable, on the score of economy, to attempt any remodelling of the existing line involving a reduction of declivity of bed, short of enabling it safely to carry the entire supply; any necessary repairs to the present bed and works can be easily effected by closing the Canal for a month or, so at a time without injury to the irrigation.

The two projects, then, to be worked out are :-
1st. The remodelling of the existing line to enable it to carry the full supply.

2nd. The remodelling of the line above Roorkee, and the construction below that of an alternative line, of sufficient capacity to convey the difference of volume between the full supply and that which the existing line can safely carry without alteration of declivity, with the repairs and additions necessary to the latter.

Sir Proby Cautley's views are given in detail in the copy of his memo. dated 8th April, attached to Appendix A.

Before touching on the relative merits or demerits of either, I think it will be best to give the detail of each as now submitted. Provision has been made in both for uninterrupted navigation from the Heads to the Cawnpore Terminus, the cost of which is shewn separately in the abstracts to the estimates.
38. The first point to be determined was the velocity of current which the soil will stand. This has been fixed, from actual observations, given in detail in Appendix D , at 2.5 feet per second for the worst soil we have to deal with, such as the Toghulpoor sand hills at the 37 th mile; at 2.7 for sandy soil generally; and at 3 feet per second for soil such as is met with below Sirdhanna. Now, in laying down these data as the safe velocities with a full supply, it is not intended to assert that no erosion will take place anywhere, but that the extent will, under any circumstances, be trifling, and

Velocities of current recommended.
with a diminished supply, silt will, to a certain extent, be deposited. Perfect freedom from the latter evil can never be expected; the alternate rising and falling of the supply in the Canal will always cause abrasion of banks more or less; besides which, the water from the Ganges entering at the Heads during the rainy months is never entirely free from silt.
39. The best form of overfall is the next point to be considered.

The objects to be kept in view in any design for such works are, I conceive-

1st. To disturb the normal condition of the stream, both above and below the fall, as little as possible.

2nd. To reduce the increased action of the falling water on the masonry works to a minimum.

The first object will be attained by maintaining the depth of the water on the crest of the fall, or close to it, the same as in the channel above, where the velocity of current is "in train" or uniform ; and by so deadening the accelerated relocity immediately at the foot of the fall, that the water should issue from the masonry work with the normal velocity in the channel below.

Now the only means of maintaining a constant depth above the fall, or the declivity of the water surface up to the fall uniform, is by contracting the waterway at that point. This may be done from above, from the bed, or from the sides. The first would cause an excessive scour on the bed close to the fall, and is otherwise objectionable; the second is the method adopted on the Ganges Canal, by raising the crests of the falls; and the third by narrowing the width of the crest, which was proposed some time since, and a design submitted, for the Futtehgurh Branch Falls, by Mr. J. Parker, the Superintendent of that Division of the Canal. All these constructions have, I believe, been previously in use elsewhere ; but the most efficient we know of yet is a modification of the last described, designed by Captain Dyas, the projector of the Baree Doab Canal Works in the Punjaub (now Chief Engineer of Irrigation in the North-Western Provinces), in which the water falls through an inclined grating similar to the teeth of a comb, a full description of which will be found in the copy of his memo. (Appendix E). By regulating the spaces between the bars of the grating, the water can be kept at any required height, and the velocity of the water in the channel above is maintained sensibly uniform till within a few feet of the fall ; its greatest advantages, however, lie in the division of the water into thin films, and the extension of the space over which it falls, thus materially diminishing the action at the foot of the fall.

As to the form to be given to the actual overfall, much difference of opinion has existed. Sir P. T. Cautley has given his views on the question, and reasons for preferring the ogee curve to either an inclined plane or a vertical drop in the Ganges Canal Report (Vol. II, page 159). Subsequent experience, however, has shewn clearly the superiority of the
latter. The scour of water, especially when loaded with silt, over an ogee curve or an inclined plane, must inevitably, in the course of time, wear out the hardest material, besides the evil effects of the accelerated velocity for a considerable distance in the channel below in the erosion of the bed and banks, which is well exemplified in the case of the Muhmoodpoor Falls (vide diagram at para. 20.)

On the Eastern Jumna Canal, it is true, the falls of this form have acted efficiently for many years; but spite of the best of workmanship, and the comparatively small body of water passing over them, repairs more or less are periodically necessary. On the Baree Doab Canal in the Punjaub, all our falls with one exception were built with a vertical drop ; and this, constructed on the general design of the Ganges Canal Fulls, is the only one which has been a source of constant trouble from the first. A vertical drop fall, with a cistern of sufficient width at the foot, is undoubtedly the best form, by giving a wide and deep basin for the backwater, in which the falling water may expend its violent action before issuing from the masonry work. It also effects a second desideratum in diminishing the impact of the falling water on the masonry.
The action of water falling thus has been compared to that of a pile being driven close to masonry, which is felt to vibrate with every blow of the ram. This is not, however, a fair comparison; the actual result being analogous to the impact of one elastic body in motion on another at rest, in which the moving body loses a considerable portion of its velocity, the other acting as a cushion or buffer. What the depth or surface area of the lower water should be to protect the masonry from all injurious action we have no data yet to determine accurately; but from Captain Dyas's memorandum, above referred to, it will be seen that the dimensions adopted in his works have answered with the supply of water hitherto flowing in the Baree Doab Canal.
It is clear that, in a vertical fall with a "grating," the action below must be much less than in either a "weir" fall, or one in which the water passes through a single contracted opening; the form possesses also the additional advantage, incalculable in a navigable channel, of preventing any acceleration of the surface velocity, further than a few feet from the actual crest of the fall. The size of the basin below must be the same for all.
40. The headway for navigation nuder the bridges has been fixed at a minimum of 10 feet for the following reasons:-1 found, by enquiry among the boatmen on the Ganges at Cawnpore, and measurements on their boats, that the highest load above the gunwale they carry even on the river is limited to three bales of cotton laid one over the other, the thickness of a bale of unpressed cotton being $2 \frac{3}{4}$ feet generally; this would give a clear height of $8 \frac{1}{4}$ feet, and if the deck or gunwale of the boat when laden be 1 foot above the water surface the total height should be 94 feet. Ten feet, therefore, appears ample for every purpose, and this would admit of the passage of small steamers and passenger boats

Headway necessary for narigation.

On English navigable canals the height allowed is generally from 10 to 11 feet.
Dimensions of locks. 41. The proper dimensions, especially the width, to be given to the lock chambers were not so easily determined. With the boats at present in use on the Canal, varying in width up to 12 feet. unpressed cotton, the most bulky material hitherto carried, is loaded on the deck or gunwale to a width of 15 feet, or three bales, each bale being about 5 feet in length; the present locks being 16 feet wide, this passes easily.

If large and powerful steamers are to be employed, i6 feet is not sufficient. Judging from Canal works in America and elsewhere, nothing under 20 or even 25 feet will answer ; yet screw steamers navigate the Forth and Clyde Canal throughout, where the width of the locks is only 15 feet. Small steamers, and tracking by men or cattle, seem to me most suitable to the present condition and habits of the natives of the country; and as the navigation will be carried on chiefly, if not entirely, by them, I have retained Sir P. Cautley's width of lock as sufficient for all purposes. These two last points, however, are open to discussion.
Separate lock channels 42. Separate lock channels round the falls have been not advisable, and why. avoided, except in one or two instances, throughout the projects now submitted, from their unavoidable tendency to silt, especially in the lower reaches. With a velocity in the channel maintained uniform up to the fall crest. and gratings or other protection, there can be no risk in attaching the lock chamber to the side of the fall, and the construction is much less expensive.
General measures re- 43. The measures, then, recommended for remodelling commended for remodell- the existing line are generally as follows:-

1. Reduntion of the declivity of bed, and increase to the sectional area of channel.
2. Construction of new overfalls, with vertical drop and gratings.
3. Alteration to existing falls ; making the drop vertical, and attaching gratings.
4. Raising bridges, \&c., wherever the headway is insufficient.
5. Constructing towpaths under the left arches of all bridges now without them.
6. Repairing the berms throughout.
7. New sluice apparatus at all Regulating Bridges, and means of conuteracting the effect of difference of surface level above and beiow. The main difficulty in projecting the alterations has been to utilize existing works to the utmost extent, each bridge and fall requiring separate consideration ; the new levels of the bed have been adjusted chiefly with reference to the depth of the bridge foundations. Whatever is deficient in the following details will, I think, be found in the Tabular Statement (Appendix B.)
Volume of water to be 44. The volume of water to be provided for and its distriprovided for and its distri- bution has been assumed as originally determined by Sir P. T. bution

Cautley (vide annexed Statement), with the addition above
（ 23 ）
Distribution of Water in Original Project for Ganges Canal．

| LOCALITIES． | Position in Miles from Head． |  | Lengti in Miles． |  |  | Discharge C．Ft．p．Second． |  |  |  | REMARKS． |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 号 | $\stackrel{8}{8}$ | $\begin{aligned} & \text { すid } \\ & \substack{0 \\ \hline} \end{aligned}$ |  | 品 |  |  |  |  |  |
| Main Line．－Heads to Futtehgurh Branch |  |  |  |  |  |  |  |  |  |  |
|  | 0 | 50.5 | 50.5 | 23.0 | 27.5 | 8.0 | 330 | 6750 |  | Rajbuha outlets confined to overfalls． |
| Head．．．．．．．．．．．．．．．．．． | $50 \cdot 5$ | 110.3 | 59.8 | 59.8 | $\cdots$ |  | 330 | 5180 | J |  |
| ＂Thence to Koel Branch Head．． | 110.3 | 152.0 | 417 | 41.7 | ．．． | 8.0 | 328 | 4330 |  |  |
| ＂Thence to Nanoon Branch Head | 152.0 | 180.9 | 28.9 | 28.9 | ．．． | 8.0 | 232 | 3482 |  |  |
| Cawnpore Branch ．．．．．．．．．．．．．．．．．． | 0 | $170 \cdot 0$ | 170 | 170 | ．．． | 8.0 | 1360 | 1610 | $\{$ | 50 c ．ft．reserved for navigation between Eesun and Rinde． 200 c．ft．for navi－ gation． |
| Etawah Branch ．．．．．．．．．．．．．．．．．．．．． | 0 | $180 \cdot 0$ | 180 | 180 | ．．． | 8.0 | 1440 | 1640 |  | 200 c ．ft．reserved for ņavigation． |
| Futtehgurh Branch ．．．．．．．．．．．．．．．．．． | 0 | 155.0 | 155 | 155 | ．．． | 8.0 | 1240 | 1240 |  |  |
| Boolundshuhur Branch ．．．．．．．．．．．．．．． | 0 | 65.0 | 65 | 65 | $\cdots$ | 8.0 | 520 | 520 |  |  |
| Koel Branch ．．．．．．．．．．．．．．．．．．．．． | 0 | 65.0 | 65 | 65 | $\cdots$ | 8.0 | 520 | 520 |  |  |

the Rutmoo of the approximate expenditure in irrigation and loss by absorption on the line between that and the Heads. This, according to Mr. Login's experiments above quoted, amounts to 430 cubic feet per second; and my own observations taken last spring give about the same proportion. The maximum discharge at Myapoor should therefore be $6750+430=7180$ cubic feet per second. Between the Rutmoo and Roorkee the absorption must be very inconsiderable ; irrigation there is none, and the capacity of the Solanee Aqueduct is an absolute limit to the supply at that point ; the full supply of the original project, 6750 cubic feet per second is (vide Appendix B.) the maximum which can pass there. The actual expenditure for irrigation in the sandy tracts below Roorkee is much above that originally allotted being upwards of 800 cubic feet by Mr. Login's observations (instead of 330 cubic feet); the total approximate diminution in the 30 miles to the Futtehgurh Branch Head being 1004 cubic feet per second. My own experiments give a somewhat less result; they were not, however, very satisfactory, as I could not obtain the discharge at the lower extremity with the rajbuha heads closed above, the Canal not being considered in a fit state to carry the additional supply with safety. The abnormal rate of diminution of supply on this portion of the line will not, however, I think, affect the quantity originally calculated on as available for the lower districts. When the soil of the irrigated districts becomes fully saturated, a very small supply will be sufficient to maintain the saturation ; and the allowance of water for irrigation can be regulated by periodical closures of the watercourses. as has long been customary on the older Canals in these Provinces.

## REMODELLING FROM MYAPOOR TO ROORKEE.

Atterations in ChanneL 45. The present waterway of the bridges, 165 feet, has been taken as the limiting width of the cross section of the earthen channel except in the reach from the Rutmoo to the Roorkee Bridge, where the available area between the ghat revetments on the Solanee valley embankment has been taken as the basis of calculation.

The reach above the Khunkul Bridge has been left in statu quo. The calculated velocity at full supply will be about 5 feet per second, which the stiff soil there will apparently stand ; below this, and on to Roorkee, the safe velocity has been assumed at a little over 3 feet per second, which I found to be the maximum in the present channel, where the bed and banks appeared to stand fairly.

From the crest of the Ranipoor Falls the bed will be carried backward on the new declivity of 1 in 8,500 , producing a drop at the Khunkul Bridge of 3.07 feet. The depth of water in this reach being 13 feet, while that above the bridge is 9 feet, the surface at full supply of the lower water will meet that above a considerable distance above the drop. The backwater being thus ample, it does not appear necessary to construct a fall here. Provision is made under the arches of the present bridge to prevent undue acceleration of velocity in the channel below.

The alterations in channel down to the Puttree Works need no remark.

From the flooring of the Roorkee Bridge upwards, the new bed is projected on a declivity of 1 in 9000, which produces a drop at the Rutmoo Works of 3.64 feet; at the Puttree, 6.06 feet below the floorings. The level of the floorings of the Rutmco Works, it is evident, cannot be altered. The surface of the water at full supply would therefore stand here ( $12 \cdot 8-3 \cdot 64=$ ) $9 \cdot 16$ feet above those floorings. Now, as the width between the revetments above the Regulating Bridge is 236 feet, the velocity of current here cannot exceed
 Tabular Statement) with the calculated velocity in the channel below.

It has been stated before (para. 13) that, wherever an abnormal widening of the Canal channel has taken place, the bed is at a higher level there than either above or below. In these places the current has formed its own channel, the declivity of the surface of the water remaining uniform. This being the observed result of natural operations, it seems to me clear that, in projecting the levels of an artificial channel for a uniform velocity of current, wherever abnormal widening is necessary, there the bed should not be formed on the usual level, but be raised to such a height that the areas of water section in the ordinary channel and the widened portions should nearly correspond, or rather that the velocity of current through either should remain uniform. The bed between the Puttree and the Rutmoo, it will be seen from the section, has already been channelled out to nearly the fall extent required for this alteration; and as the declivity of the surface of the water through the Regulating Bridge will remain unaltered, the action there will be inconsiderable; a talus of boulder work will answer every purpose of protection. Two other incidental advantages of no slight importance may be mentioned here:-The water at full supply will stand below the tops of the piers of the Rutmoo Dams, now ten feet above the flooring; and the headway under the lock passage at the Puttree at full supply will be 8.4 feet instead of 2 -34, as it would be with 13 feet of water on the flooring there.

Though the channel through the stiff soil of the Peeran Kulliar Bridge is apparently proof against the effects of a higher velocity than that calculated in the remodelling; and though the bed of the Solanee embankment might be protected, as Sir P. T. Cautley at first proposed, by a layer of boulders,-the velocity which would obtain at full supply, 4.091 feet per second, with the present declivity of bed, appears to me hazardous. I have therefore preferred to reduce it to the normal limit. Protective measures to the bed and banks will thus also be unnecessary.

The estimate provides for the widening and deepening of the present channel, and the repair of the towpath on the left, or tracking bank, as well as cutting it down where necessary to a constant level of 2 feet or thereabouts above the calculated full supply level.
46. The navigation is provided for, in addition to Lockage from Khunkul alterations at some of the bridges, by a new lock shannel

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the talus of boulders. The waterwings and ghats must be raised slightly to suit the increased height of water.

A separate lock channel (Sheet 3), for the reasons given before, has been projected on the left bank ; the lock chamber situated at the junction with the Canal below the bridge to allow of a scour throughout; a bridge will be required in prolongation of the present one over the Canal ; the channel, to save excavation, is designed with boulder masonry revetments on both sides.
49. The alterations at the Puttree and Ranipoor Works may be described together, being to a great extent similar ;

Puttree and Ranipoor Locks. Sheets 5 and 6 shew the proposed additions, \&c., at the former: the masonry of the fall ogees to be removed, thus making the drop vertical; new flooring laid in below and covered with stone at the point of greatest action. Piers to be built in the centre of each opening supported on flying arches abutting on the drop walls and faces of the super-passage arches ; between these gratings, of sal wood, to be fixed as shewn in plan.

Similar gratings to be attached at the extremities of the arches on the down stream side, in order to diminish the velocity under the super-passage, and prevent acceleration of velcicity in the channel below.

The boulder talus to be lowered 6.06 feet and relaid; the lowest layers in mortar ; the upper, of the largest stones carefully packed together; and to secure this from all risk of being undermined, a row of wells to be sunk 50 feet below the head wall of the super-passage, right across the channel. Round wells have been projected for all such work in preference to the "blocks" of the original design from the very superior facility with which they can be undersunk. With these protections and the amount of backwater here, no apprehensions need, I think, be entertained for the safety of the work.

Piers and waterwings up stream must be raised to suit the increased height of water in the channel.

The alterations for the navigation are as follow. A portion of the outer revetment of the masonry channel above the lock to be removed to give direct access from the Canal, the old navigable channel being embanked across higher up, and a culvert constructed under the embankment for the escape of drainage water ; lock chainber walls and gates to be raised slightly above full supply mark. As with a less supply in the Canal than 9 feet the depth of water on the lower flooring will not be sufficient to float a laden boat, the formation of a second lock chamber below the arches of the super-passage becomes a necessity; the proposed design of this will be seen in the plan; with a higher supply in the Canal than 9 feet the use of these gates will be unnecessary.

At the Ranipoor Works similar alterations are required, with the exception of the down streain gratings, additional lock chamber, and new boalder talus and curtain; the present talus to be lowered one foot. The headway under these arrangements at full supply will be but 4 feet; if the bed from this point to the upper Bahadoorabad Fall be depressed 4 feet (it is at present channelled out to a greater depth), and
additions be made down stream as at the Puttree, a similar extent of headway will be available. This would add about Rs. 15,000 to the aggregate of the present estimates: I have not therefore included it in the alterations, not being necessary to the water carrying capability of the Canal. If, however, a free passage for boats be considered a necessity, it is the most economical way of effecting it. The only other course open is to carry a separate lock channel round the works, crossing under the torrent at some distance higher up on the right bank of the Canal, the cost of which, I need hardly say, would be very great.
Bahadoorabad Falls and
Locks.
50. The Upper Bahadoorabad Falls will be altered as follows (vide Sheet 4): masonry weirs to be built on the ogees to convert the fall into a vertical one. The bridge piers to be lengthened and short piers built in the centre of each bay to support gratings similar to those designed by Captain Dyas. These will consist of wooden bars resting on wrought iron girders, as described hereafter for the Muhmoodpoor Falls: the bridge arches to be raised $5 \cdot 80$ feet to suit the increased height of water up stream; water wings raised 7 feet down stream; revetments 2 feet.

Similar alterations generally to the lower Bahadoorabad Fall, but no raising of revetments down stream necessary, boulder talus to be lowered 1.4 feet.
If the bed, as suggested in last para., be lowered 4 feet above the Upper Bahadoorabad Falls, the raising of the arches there will not be necessary.

At the upper lock on the separate channel new gates will be required to suit the increased depth of water, and the chamber walls must be raised 3.5 feet. Alteration at the lower lock will be confined to raising the bridge 3.5 feet.
51. At the Rutmoo Regulating Bridge the headway at full sapply under the present arches being only 5.84 feet, it is proposed to raise the side arches, or rather to remove them, and build new ones of greater height and span ; the increased width will allow of a narrow passage under the arch to admit of the towing lines being carried through by hand. This involves raising the entire roadway of the bridge, but it appeared, after careful consideration, to be the most economical mode of effecting the object in view.
The sluice apparatus will be altered similarly to that at Myapoor.

The boulder talus to be removed and relaid at a level of 3.64 feet lower, and a row of piles driven across the bed below the bridge as shown (vide Sheet 7).
A new entry to the drainage culvert under the bridge flooring has been projected from the down stream approach on the right bank to carry off drainage which collects there from the low land near ; the inlet on the torrent side having been, as before stated, closed for some time past. I have not estimated for removing the piers of the Inlet Dam ; if decided on, the cost of demolition would be trifling.
Solanee Aqueduct.
52. At the earthen embankment across the Solanee valley, the only additions which appear necessary are rows of stakes driven across the bed between the ghat revetments at intervals of 200 feet level with the remodelled bed; this, with
the reduced velocity of current, will gradually silt up the intervening spaces, and so protect the foot of the masonry ghats.

In the Aqueduct itself, the water at full supply will stand at the level of the roadways; a low parapet will therefore be necessary along the edges on either side ; for this a height of one foot will suffice, the entire flooring to be covered with a layer of well puddled clay covered with brick on edge; the action of the current here will be so slight that little beyond a water tight covering, is necessary. Seyssel asphalte would doubtless be the best material for the purpose, but the cost would be greater perhaps than the circumstances call for. According to data brought from England by Captain C. S. Thomason, R. E., the cost of a half inch layer of the material could not be less than one rupee per superficial foot, or Rs. $1,55,380$ for the whole Aqueduct.
53. The alterations to the bridges along this section consist of removals of floorings where necessary, and relaying at the level of the remodelled bed, as well as constructing towpaths, as shewn in Sheet 15, under the left arches for which iron uprights supporting girders, and a wooden pathway have been preferred to any other construction, from the slight obstruction they offer to the passage of the water.
54. The headway available for navigation under the remodelled works on this section is not, it will be observed, uniformly up to the prescribed limit of 10 feet :-

| At Myapoor it is | ... | $7 \cdot 5$ | foet. |
| :---: | :---: | :---: | :---: |
| Jowalapoor do. | ... | $8 \cdot 13$ | " |
| ", Ranipoor do. | ... | $4 \cdot 00$ | " |
| " Puttree do. | ... | $8 \cdot 40$ | " |
| " Dhunowri do. | ... | $10 \cdot 00$ |  |
| " Muhewur do. | ... | 11.00 | " |
| Roorkee do. | ... | $9 \cdot 20$ |  |

As it has been shewn above that the Ranipoor Works can be altered, the height available under the Puttree super-passage, 8.40 feet, must be taken as the absolute limit for through traffic along this portion of the line. Heavily laden boats seldom pass under the Myapoor Regulator, so that the headway there may be considered sufficient; and considering the nature and extent of the traffic likely at any time to pass above Roorkee, the Puttree limit will, I believe, be ample.

## REMODELLING FROM ROORKEE TO CAWNPORE BRANCH.

55. The mean velocities of current which the soil will stand without erosion in the channel below Roorkee have been determined at 2.7 feet per second for the portion from Roorkee to the new falls below Sirdhanna at the 76th mile, excepting the very sandy reach between the Muhmoodpoor and Nirgajnee Falls, where it does nut appear safe to allow a higher velocity than 2.5 feet per second.

Below the Sirdhanna Falls, the mean velocity is uniformly assumed at 3 feet per second.

## Bridges.

Heqdway on remodelled line not uniform.

Between Roorkee and the Futtehgurh Branch Head, if the waterway of the present bridges be taken as the limit of the mean width of section in the earthen channel, the full supply depth of water, with mean velocities of 2.7 and 2.5 feet per second, would be $15 \cdot 15$ and 16.36 feet respectively, which would necessitate lowering the bed above the falls from 7 to 8 feet if the bridge arches there be not raised. If the bed be retained at the level of the crests of the falls, the surface of the stream at full supply would stand much above the level of the country, and from 5 to 6 feet above the springings of the bridge arches higher up, causing an afllux (though slight); the headway for navigation would then also be insufficient. By depressing the bed 7 or 8 feet, the floorings of the bridges would be lowered to a dangerous extent, and much excavation would be necessary, and that too, in all probability, below the level of the surface of the springs. It becomes, then, an obvious necessity to increase the width of waterway at the bridges along these 30 miles. Another arch of 55 feet span (there are now three arches of 55 feet span) gives a mean width of 220 feet, which has been taken as the basis of calculation.

The consequent declivities, (vide Tabular Statement) vary from 1 in 11,000 to 1 in 13,000, with depths of water from 11.3 to 12 feet. The side slopes of the channel are projected at 1 in 2 ; the berms are estimated at 15 feet in width on the left or towing bank, and 5 feet on the other; tops to be two feet or thereabouts above surface of full supply in the Canal. In very light sandy soil the side slopes will be cut to 1 in 3 ; and where erosion is likely to take place, spite of the reduced velocity, a protection of grass ropes laid closely together and pegged into the slopes, such as is in use on the Dutch Canals and elsewhere, has been included in the estimate. When the traffic becomes extensive, as I have no doubt it eventually will, some such protection as this will be necessary in all sandy soil to prevent erosion from the wash along the sides. Wherever excessive widening of the channel has taken place, the estimate provides for narrowing and forming banks to the proper section.

Below the Futtehgurh Branch Head, the waterways of the present bridges have been taken as the limiting mean width of cross section. Into the detail of the new projectiou of bed it does not appear necessary to enter here. Above most of the old falls the bed has been depressed to admit of the water at full supply passing through clear of the soffit of the arches, though in some cases above the springings. At the Bolundshuhur and Koel Branch Heads, the bed is lowered up stream to an extent equal to the difference between the depth of high water up and down stream ; elsewhere the levels of the bridge floorings have been retained wherever possible, or if altered, only to such an extent as was considered safe; a boulder or kunkur talus will, it is believed, be a sufficient protection in any case.
New Falls and Locks 56. The number of new falls on this line will be 15, projected. height of descent varying from 3.53 to 8.54 feet, making a total of 29 from Myapoor to Nanoon.

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The first will be situated at the extremity of the curved revetment wall below the Roorkee bridge where the straight reach to Hafiznuggur commences; that near Sirdhanna, at the point where the sandy merges into stiff soil: near the escapes, they have been projected immediately below and in connection with the Escape heads, to give the means of damming the Canal across when it is desirable to lay it dry below. At Moonda Khera and Kasimpoor it would have been more economical to have placed the new falls between the ghat revetments above the bridges; the restricted width there, however, necessitates their transfer below the bridge in each case; elsewhere the new works are projected above and in connection with existing bridges.

The waterway has been fixed generally by making the width between the waterwings up stream equal, or as nearly so as possible, to the mean width of water section in the channel above, so that there should be no tendency to acceleration of velocity until quite close to the crest of the fall. The navigation has been provided for by locks on either side (videSheet 8 ), the entrance to which is upwards of 105 feet above the drop at the fall, a distance quite sufficient, I think, to obviate any risk of boats or rafts being carried.within the influence of the accelerated velocity. Should, however, further protection be found desirable hereafter, a floating boom, in the form of an arrow head, with the apex up stream, and fastened to the bed in the centre and the extremities of the lock chamber walls on either side, or a boat bridge, as projected by Sir Proby Cautley, will answer every purpose. Double lockage has, in every case, excepting the old falls, been estimated for ; a single lock at each fall will not, in my opinion, be adequate to the wants of the traffic which may ultimately be expected on the Canal. The overfall is constructed on the general design of those on the Baree Doab Canal, of which a plan is annexed, except that the piers are of the shortest possible length, to give the fullest effect to the backwater in the cistern and basin below; the bridge, wherever there is one, being situated at the extremity of the tail basin. A short talus of boulders or kunkur blocks is attached to the down stream wall of the cistern; with such a protection there, and considering the reduced action of the water with this form of fall, a depth of eight to ten feet may be considered ample for the deepest foundations. The floorings of existing bridges in connection with these falls will, I believe, be sufficiently protected by relaying the talus down stream. The width of the fall bays has been projected generally at fifteen feet,and the total number estimated for (vide abstracts of estimates) in 5 sets, varying in width of waterway. For the purpose of an estimate this may be taken as sufficiently accurate ; but in practice it will, of course, be most economical to adjust the waterways to the mean widths of channel immediately above.

Iron lock gates are estimated for, on account of their superior durability and lightness of construction.

Thevarious forms of these works, according to locality, from which the estimates have been framed, will be found in Sheets 8, 12, 14, 17 and 19 of Plan $X$, and their several waterways in Appendix B.

## Alterations to present

57. The existing falls, ter in number, it is proposed to alter or add to more or less extensively. It would extend this Paper to an inconvenient length to enter into the detail of alterations at each ; suffice it to state, that each has been carefully considered and estimated for separately. Sheets 10 and 11,16 and 18, and the following description of one, will sufficiently explain the nature of the proposed remodelling generally.

A plan and sections of the Muhmoodpoor Falls will be found in Sheet 10. The waterway is two hundred feet, divided into 8 bays of 25 feet each; the entire height under the arches being only $11 \cdot 35$ feet, and the depth of water at full supply with the remodelled bed being calculated at 11.3 feet. The bed up stream has been projected at a level 2 feet below the present crest of the fall; the water will thus stand $9 \cdot 3$ above the flooring of the bridge. The lower flooring must be depressed to the level of the new bed, or 2.56 feet; the depression being carried above the foot of the ogee as shewn; immediately above this a vertical drop wall to be built across each of the bays, the top flush with the present bridge flooring; the bridge piers to be produced, and new piers in the centre of each bay to be constructed for the support of the grating girders; (the width, 25 feet, of the original bays would require too costly a section of girders) ; sections, \&c., of the latter are shewn separately on an enlarged scale. The new piers will be hollowed out underneath to allow the utmost possible freedom to the backwater below the fall; and as it is doubtful whether a cistern could be formed under the fall with the present height of spring level, a covering of stone for a length of 50 feet below this drop wall has been estimated for. With the extent of backwater shewn here, well executed brick on end will be amply strong enough to face the rest of the work with. The boulder talus to be removed to the level of the new bed, and relaid, if the remaining thickness be found to be less than 3 feet. The curved extremities of the tail revetments to be cut away to the width and the shape of the sides of the channel below, and the earthen slopes pitched with boulders or kunkur for a length of 100 feet below the fall. At the entrance to the lock channels it will be seen, from the annexed sketch, that the stream is confined between masonry revetments to a width which in no case is equal to the increased mean waterway of the earthen channel, and above the Futtehgurh Branch Head very much less. The estimate provides for removing one side and the attached rajbuha head, as well as replacing the latter and removing the left revetment of the masonry passage at the entrance to the lock channel, where the contracted width has been found so great an inconvenience. A new exit channel has been projected below the lock, rejoining the Canal in the shortest possible distance (vide annexed sketch).

Though the bed immediately in advance of the lock must be depressed 2.56 feet below the level of the flooring, the backwater, except with a very low supply, will be sufficient to float a laden boat in the lock chamber. The lock bridge must be raised 1.44 feet to give a ten feet headway at full supply.

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The fall gratings estimated for throughout these projects are, as I have stated elsewhere, on a general design which has worked successfully elsewhere; constructions for which we have not a precedent, I have thought it best under the circumstances to avoid. It appears to me, however, possible to improve on the form and material of these gratings, substituting masonry in place of iron and wood.

A design for such a grating is shewn on Sheet 11, adapted for one entire bay of the Muhmoodpoor Falls (for, with masonry, it would not be necessary to build intermediate piers).

The arch shewn might be built of brickwork, overlaid on the top with stone. It would abut on the edge of the drop wall, supported at the crown by the head wall of a light bridge thrown across the extremity of the piers. Vertical slits in this arch, of suitable form and dimensions, would give the necessary waterway, and though the exact form of the curve to be given to the sides of the slits or notches and their width is as yet undetermined, the co-efficient of discharge not being accurately known, regulation of the height of water surface above can at any time be effected without difficulty. The strength of the abutting bridge between the piers need not be great, as the super-incumbent weight on the arch decreases rapidly towards the crown. The absence of the intermediate pier will obviously increase the effective backwater.

With the high rate of stone now procurable, the difference of cost (in favour of the new design) cannot be estimated higher than one-ninth ; but its superior durability will not, I think, be questioned.

I have, from a desire to utilize to the fullest extent all existing works, retained in this estimate the present lock channels and locks; but considering the inconveniences and obstructions caused by them, I believe it would be preferable to construct new lock chambers in the side bays of the existing falls. A design for this is shewn in the plan of the Muhmoodpoor Falls in dotted lines. It would be necessary to raise the side arches some eight feet to obtain the full amount of headway; to effect this a drawbridge has been projected and estimated for, the thrust of the other arches thus left without sufficient support being counteracted by the lock chamber walls built up against the side piers. Protection to craft above the fall might be afforded at a small expense by a line of piling extending up stream in the prolongation of the lock wall, the top raised two or three feet above full supply level. This arrangement for the lockage would raise the present estimate for each fall by about Rs . seven thousand (7,000).
58. The proposed alterations to the Bridges are as follow :An arch of fifty-five feet span to be added to each of the bridges (eight in number) between Roorkee and the Futtehgurh Branch Head. The general design is shewn in Sheet 9, which gives plans and sections of the Munglour Bridge. Towpaths will be added under the left arch, as shewn in Sheet 15 ; floors lowered and relaid, and waterways, \&c., raised where necessary. From the Futtehgurh Branch Head to the Sirdhanna Bridge, alteration will be confined to removing and
relaying floorings, raising waterwings where necessary, and attaching towpaths; the latter under all the arched bridges along this portion of the line are projected at a width of twelve feet, the curve of the arches not allowing of sufficient headway closer to the abutment.

Along the entire line from the bridge below Sirdhanna to Nanoon, the headway, I have before stated, is deficient. It is proposed to remove the arches, substituting iron girders and roadways of kunkur over sheet iron plates, the general design of which is shewn in Sheet 15. Another method of getting over the difficulty was by the excavation of a navigable channel bridged at a sufficient elevation, round each bridge on the Canal, and this would undoubtedly be the leasticostly; but the constant petty expenses and inconvenience such channels would inevitably cause make the former method preferable as a permanency. The towpatbs under these girder bridges need not project more than seven feet. The other alterations are similar to those at the bridges higher up.

The flooring of the Sirdhanna bridge, which is of kunkur masonry, has remained in first-rate order, spite of a constant rush of water through the arches. I have therefore estimated for new floorings of similar material, wherever it is procurable at a moderate cost.

New masonry arches mightbe constructed at half the expense of girders, and would be more durable; but the time occupied in the manufacture of bricks and of erection would be much longer, and it would be necessary to raise the approaches, which with girders would generally be retained at their present levels.
59. At the Branch Heads, similar changes are required as at the Rutmoo Regulating Bridge, with the addition of gratings and lockage into the branch lines.

Sheet 13 shews the proposed alterations at the Futtehgurh Branch Head. The two side arches of the bridge over the main channel to be raised sufficiently to give the normal headway of ten feet; iron drop gates to be fitted to these openings; each of the remaining arches to be divided into three parts by iron uprights grooved for the reception of wooden sluice gates. The left arch of the Futtehgurh Branch Bridge to be similarly raised, and the remaining superstructure of both bridges, and the curved connecting wall with a roadway, to be made up to the same height. A weir two feet in height to be built across the floorings of the other arches on the Futtehgurh Branch, immediately under the upstream head wall ; iron girders to be attached to these arches as on the main regulator; the necessary heading up of the water to be effected by vertical wooden gratings let into the grooves, similarly to sluice gates. A lock chamber to be constructed as shewn, formed partially by the left bay of the bridge, the upper gates being placed close to the up stream headwall.

The floor of the lock chamber to be level with the top of the weir.

The side slopes of the channels down stream to be pitched for a length of 50 feet with kunkur blocks or boul-
ders. Ghat revetments and waterwings on Main Channel to be raised 2 feet. Alterations and additions at the Bolundshuhur and Koel Branch Heads to be similar, except in height of raising. The lock chambers here are restricted to 15 feet in width by the spans of the existing side arches.

Single lockage only, it will be observed, has been provided for the Branch Canals ; the traffic there will be small compared with that on the trunk line.

At the Cawnpore and Etawah Branch Regulator, the surface of full supply down stream in the Etawah branch will be eight feet below the soffit of the arches; in the Cawnpore Branch, ten feet, as the bed there will be depressed two feet. The bridges may, therefore, I think, remain as at present. If the headway under the Etawah Branch Bridge be hereafter found too little, a navigable communication may be made between the branches lower down, or a channel cut round the bridge, unless it be found practicable to depress the bed for some distance below the head.

Double lockage provided for (vide Sheet 20) on the main line regulator ; single on the Etawah branch; the locks in both cases to be constructed in the main chaunel above the bridges, to admit of boats passing under the bridges at the level of the lower water, and also to keep them out of the influence of the accelerated velocity close to the bridges.

As the difference of level up and down stream at full supply will be, in the Etawah Branch 3 $3 \frac{1}{2}$ feet, in the Cawnpore Branch $5 \frac{1}{2}$ feet, gratings similar to those for the Puttree Works are designed in all except the lock bridge arches; to support these, piers to be built in the centre of each arch up to the level of the top of the up stream waterwings. The edges of ghat walls up stream to be raised two feet.
The inverts of the bridge flooring on the Cawnpore Branch to be removed; and flat flooring, covered with brick on edge, substituted. A line of piling to be driven across the bed of both branches at the extremities of the waterwings, and the bed between them and the bridge curtains to be covered with a four feet layer of kunkur blocks.
60. At the escape heads no alteration appears necessary, except lowering the floorings in four bays of the Khutowlee, Aboo Nulla and Jannee Khoord works, to admit of laying the Canal below them dry when necessary. From the silted-up condition of these escape heads, the lower districts of the Canal are frequently overloaded, when irrigation is not required above, by a five feet supply on the Roorkee gauge (the present full supply being seven feet). Store rooms, as described hereafter for the alternative line, are estimated for each new overfall ( 15 in number).

## REMODELLING.

## Camnpore Brance.

61. It is proposed here to depress the bed two feet from the regulating bridge to the Keylunpoor Bridge at the 3rd

Alteration in Channel
to Keylunpoor Bridge.
mile, removing the ogee fall in the flooring of the bridge, and thus obviating the necessity of gratings and lockage. The irrigation will be little if at all affected by this depression, as the depth of water in the channel will be greater with the reduced velocity. All the other modifications to this branch are solely due to the necessities of navigation.

## Alterations to Bridges generally.

62. To give a clear headway of ten feet, all the bridges, 56 in number, between the heads and the Dubowlee escape at the 165th mile, must be more or less raised. This it is proposed to effect by substituting girders, as on the main line. At the Aima bridge, 122 d mile, the substitution as before mentioned, has been already made. The railway girder bridge at the 163 d mile must be raised two feet, and the gradient altered on both sides to correspond. A towpath seven feet in width, close to the left abutment of each bridge, to be fixed similarly to those on the main line.
Retention of present channel below Dubowlee Escape as navigable line recommended, and why.
63. The lower railway bridge being close to the station at Cawnpore, and the surface of the roadways of the bridges in the town, above the terminal lockage, being on the level of the adjoining streets, any raising there is out of the question ; there is nothing for it but to depress the bed along this portion of the line. Now, from the Duknapoor Bridge, that situated immediately below the lower railway bridge, there were three methods of completing the navigable courmunication with the upper portion of the Canal :-1st. By taking off a navigable channel above the upper railway bridge, and rejoining the Canal above Duknapoor; the length of this would be about five miles, requiring one lock and four bridges.

2nd. If deficiency of supply be feared from the wants of the irrigation in a dry season, this channel should be led off from above Runjeetpoor Bridge (the 150th mile), where the last set but oue of the rajbuhas leave the Canal ; this would add 12 miles to the length, 2 locks and 9 bridges would be required, but it would diminish the number of bridges to be raised on the Canal by six. Both these methods would avoid the railway altogether.

The 3rd was to depress the bed from below Dubowlee bridge to the upper lock at Cawnpore sufficiently to afford the necessary headway under the bridges in the town.
The balance appears to me to be in favor of the latter, and it is the least expensive. The alterations are projected accordingly. The water in the channel below Dubowlee being only for navigation purposes, the declivity and area of section have been determined for a velocity of current just sufficient to prevent the growth of weeds, viz., $1 \frac{1}{2}$ feet per second; the width at the water line with a depth of four feet being 36 feet, ample for the passage to and fro of two fullyladen boats. The new section nust be carried up close to the Dubowlee bridge, above which the last irrigation channel is taken ; as, with only 4 feet of water in the Canal there (the least, I think, which can be allowed for efficient navigation) the discharge will be 225 cubic feet per second, of which the rajbuba will take off fully 80 cubic feet, leaving only 145 cubic feet, the minimum absolutely necessary for the navigation below.
64. The drop below the Dubowlee Bridge will thus be 5.61 , to overcome which, a double lock, similar to those hereafter described for the separate navigable line, is projected; the head of this will be placed about 150 feet from the bridge on the down stream side.

The Dubowlee Bridge itself will be raised to give the full headway; and here it seems preferable to build an arch of larger span so as to give room for towpaths on either side, formed by cutting down the present abutments to a suitable level above the bed, the waterway at this point being reduced to 20 feet, almost too narrow of itself for the purposes of the traffic. The new abutments can be formed without difficulty by filling in the present flank arches, thus giving a clear span of 30 feet, with 5 feet towpaths on either side.
65. The bottom of the foundations of the bridges below Dubowlee being either level with or slightly higher than the remodelled bed, it is proposed to underpin them, as shown in the plan of Duknapoor Bridge (Sheet 21), relaying the floorings at the new level. The span of these bridges, 20 feet, will not allow of the construction of towpaths for cattle under the arches; 18 feet is, I think, the very smallest width which can be allowed for the passage of a laden boat moving with the usual velocity; foutpaths 2 feet wide only have therefore been projected close to each left abutment, which will allow of uninterrupted tracking by human labour along these two last miles.

The upper floor of No. 1 lock, the highest of the terminal series, to be lowered $3 \frac{1}{2}$ feet, and the gates adjusted to the lower level.
The upper mills will have to be altered to suit the depressed level of the water in the Canal. The cost has not been included in the estimate, as mills in such localities, it is well known, always pay their own expenses, whether of original construction or repairs.
The arch of the "General Gunj" Bridge to be removed, and girders substituted at a higher elevation of $4 \frac{1}{2}$ feet. Two feet towpaths added below.

## The arches over lock chambers No. 8 to be raised 2 feet.

The quay revetments to be removed wherever the width of channel is thereby narrowed to 20 feet; the ghatts, on account of their great length, aggregating 695 feet on both sides, it is proposed to leave for the present, underpinning their foundations as shown in Sheet 22. If found to interfere materially with the traffic hereafter, they can be removed and rebuilt at a trifling cost ; the section on the remaining portion of the earthen channel to be altered as shewn in Sheet 22.
66. This completes the detail of the proposed remodelling on the trunk line. The restricted waterway, 20 feet, through the bridges at and near Cawnpore may hereafter be found inadequate to the wants of a fully developed traffic ; this may, however, be obviated at any time by removing the arches and substituting girder bridges of greater span, suitable for an enlarged waterway and cattle towing tracks.

The surface of the springs, even with a dry Canal, being everywhere either at or close below the level of the bed, deep

New lock and alteration at Dubowlee Bridge.

Alterations to works in Cawnpore.

Waterway through Cawnpore may be found insufficient, and remedy possible.
foundations wherever necessary have been projected and estimated for on undersunk wells, except in localities where, as at the Roorkee Fall, the water can be carried off by a cut into adjoining low ground.

Towing, it will be observed, has been provided for only on one bank; with a downward current all along the channel, haulage upwards only will be necessary.
Alterations on Branches.
67. On the Futtehgurh Branch, the only alteration which appears necessary to enable it to carry the full supply safely, is reduction of the declivity of bed in the first 17 miles from the head.

This may be effected by raising the crests of the Churialee and Akbarpoor Falls 2.6 and 1.68 feet respectively, on which might be built masonry weirs (or the bays planked up) to a beight sufficient to maintain the surface of the water at its normal level.
The Bolundshuhur Branch seems to act sufficiently well as an irrigating channel; the Koel Branch, it has been stated before, is not yet in existence; and the Etawah Branch is in fair order.

If the Branch Canals also are to be efficient navigable communications, alterations similar to those described for the trunk line will be required to the bridges generally and lockage at the falls; the latter was included in the original designs for the Futtehgurh Branch, but has hitherto been held in abeyance. The detail of such additions and alterations I have not had time to go into; they are therefore not included in the present estimates.

## THE ALTERNATIVE LINE.

New line into Futtehgurh Branch not recommended, and why.
68. The maximum supply which can be safely carried by the existing line at various points between Roorkee and Nanoon Regulator, is shewn in the annexed statement in the column of "Actual Capacity."

This is immediately below the-

|  | Mile. | Cubic feet per second. |
| :---: | :---: | :---: |
| Hafiznuggur Falls ... . . ... ... | $23+d$ | 1870 |
| Futtehgurh Branch Head ... ... ... | 50th | 1790 |
| Jutpoora Bridge ... ... ... ... ... | 80th | 2533 |
| Mussoorie Falls ... ... ... ... ... | 106th | 2240 |
| Pulra Falls ... .. ... ... ... ... | 149th | 1967 |

It has been stated before, in para. 37, that two suggestions have been made regarding the course of an alternative line, one of which is to cross the present channel above Meerut, and take up the line of the F'uttehgurh Branch somewhere
lower down. If this be adopted, and the maximum supply ( 2,533 cubic feet) admitted into the old line there (say near Sirdhanna), after taking off the volume of the Bolundshuhur and Koel Branches ( 1,040 cubic ft.) and the expenditure en route ( 493 cubic feet) only 1,033 cubic feet will pass the head of the $\mathbf{K}_{\text {oel }}$ Branch, and the Channel there can carry 1,967 with perfect safety. The discharge below the Mussoorie Falls would, on this supposition, be 2,368 cubic ft ., while the maximum capacity there is but 2,240 cubic feet; some protection would therefore be necessary to the banks there, and below the Koel Branch head the irrigation would necessarily suffer from the depression of the water surface in the channel. On economical grounds, then, I think this proposition may be considered out of the question, and we may take Sir P. T. Cautley's original suggestion as the only course feasible.
69. It is obvious from the statement given above, and that at para. 44, that the existing channel cannot at any point carry the entire supply of water allotted to it in the original projection. It is therefore clear that the alternative line must run the whole distance from Roorkee to the Cawnpore and Etawah Branch heads.

Now, the maximum supply which the channel immediately below Roorkee can carry is about 2,000 cubic feet per second; but between Hafiznuggur and Muhmoodpoor it is only ... ... .. ... ... ... 1,789 cubic feet.

If we add to this the expenditure above
Hafiznuggur Falls ... ... ... ... .. 82.5 "
The maximum admissible at Roorkee is 1871.5 " Of this 1,240 cubic feet will be abstracted by the Futtehgurh Branch, leaving 300 cubic feet for the irrigation wants below. The old channel should therefore be supplemented here by a feeder from the alternative line ; this, however, is not advisable on economical grounds, as the channel in this sandy tract cannot carry the entire supply required down to Nanoon, and a second feeder would consequently be a necessity.

The first junction has therefore been projected at the highest point, where the soil will admit of a 3 feet velocity of current. A bove this, the irrigation must be provided for by holding up the water at each fall.

All irrigation above the junction at Jutpoora Bridge must be supplied from the existing line; below that, the proportion allotted to it (8 cubic feet per mile being taken as the entire expenditure) is 6 cubic feet per mile, the remaining 2 cubic feet being distributed from the alternative line. The volume to be admitted into the old line at the junction is limited by the capacity at the Pulra Falls (with the addition of the expenditure en route) to 2,375 cubic feet per second ; and as the discharge of the alternative line above, vide Tabular



Statement, Appendix B) is $(6870-1870=) 5,000$ cubic feet; and the quantity remaining in the old channel over and above the expenditure below Roorkee is 165 cubic feet; the junction must deliver 2,240 cubic feet into the latter, leaving 2,760 to be carried on by the alternative line. As the abstraction of the supply for the Bolundshuhur and Koel Branch Heads would reduce the volume of water in the existing channel below its actual capacity, besides affecting the irrigation, it will be evident that they must be supplied from the alternative line.

The new depth of water below Jutpoora will be about the same as at present, so that the existing irrigation there will be unaffected by the changes.
70. Of the proposed course of the line little explanation is necessary, beyond that afforded by the map (Plan II). Suffice it to say, that it crosses the West Kalee Nuddee above Deobund, passing thence along the watershed (vide cross sections, Plan V.) between that river and the Hindun, and recrossing close to its junction with the latter. At the 61 st mile the first bifurcation is situated, the easterly branch being the feeder to the old channel. The heads for the Bolundshuhur and Koel Branches have been projected at the actual crossing point of the former, and on the line proposed for the latter.
71. The sections of channel and declivities of bed have been determined for mean velocities of current, similarly to those in the remodelling of the old line, viz., $2 \cdot 7$ for the upper portion down to the junction; 3 feet per second for all below. The soil generally on the upper section is very similar to that on the present line opposite, with the exception of the sand hills at Toghulpoor and elsewhere.

The cross sections of the channels are shown in Plan VII. In natural channels there is, doubtless, a proportion of depth to breadth with which any tendency to abrasion would be equal on bed and banks; there are few observations as yet, however, by which the proportion for different velocities of current could be accurately determined. On the Western Jumna Canal, the proportion, where the stream appeared to have formed a channel for itself, was 1 to 13 . On the Ganges Canal in the sandy tracts it varies from $\frac{1}{18}$ th to $\frac{1}{20}$ th and even less. I have adopted the last proporion for the largest section, as a greater depth than 10 feet is inconvenient in the working of the sluices, lock machinery, \&c., \&c.

The towpaths are maintained at a uniform height, generally 2 feet above the calculated surface of full supply. A surf berm, 4 feet in width at the level of full supply, as on some Canals in America and clsewhere, is projected for the section where navigation will be carried on, on which the growth of grass, \&c., should be encouraged, to form a protection from the wash caused by the passage of boats.

The navigable communication is proposed to be carried along the alternative line from Roorkee to the Jutpoora junction, whence it may pass along the old line, the depth of water being sufficient, and the headway under the bridges ample, except at the branch heads, and the bridges at the Pulra and

Course of Alternative Line.

Sections of channel and declivities.

Simra locks ; the headway at the former should be but $\$_{\frac{1}{2}}$ feet with a full supply; 7 feet at the latter.

No lockage or towing paths in the alternative line are therefore projected below the Junction Branch head. The communication with the Futtehgurh Branch will be kept up along the old line from Roorkee downwards. Escapes are projected from two points :-The first, 1 mile 920 feet in length, from the 51 st mile, into the West Kalee Nuddee above the lower crossing; the second 8 miles long, from the 10 th mile, immediately above the bifurcation for the Bolundshuhur Branch, into a nullah leading into the Jumna.

No levels have been taken over the actual course of the latter; the estimate is based on a parallel line of levels some 2 miles to the Nurthward.
72. The overfalls, 25 in number, are designed similarly to those described for the remodelling (vide Plan X., Sheets 26 and 27) ; no locks attached below the Junction Branch head.
73. Two aqueducts are required across the Kalee Nuddee, which form a very heavy item in the Estimate. At the lower crossing especially, the width and depth of valley rvide longitudinal section, Plan IV.) is very great ; and it is necessary to keep the bed of the Canal at a higb elevation to avoid heavy cutting on the down stream side. The designs for these works are given in Sheets 30 and 31. The waterwrys for the river floods have been determined from that of a bridge on the Shamlee Road, opposite Moozuffurnuggur, which has now been standing for many years, I believe, without injury from the highest known flood.

The width of the Kalee Nuddee catchment basin is tolerably uniform throughout, and the distances from the head of it are as follow:-
$\begin{array}{lllc}\text { Upper crossing, Alternative Line.......... } & 17 & \text { miles. } \\ \text { Mozuffurnuggur Bridge.................... } & 36 & \text { " } \\ \text { Lower crossing, Alternative Line ......... } & 56 & \text { " }\end{array}$

$$
\text { Lower crossing, Alternative Line ......... } 56 \text { „ }
$$

The aggregate area of waterway section at the bridge in the highest known flood was $165 \times 10=1,650 \mathrm{~S}$. feet

This gives for the upper crossing a waterway of $1_{30}^{1650}=779 \mathrm{~S}$. feet nearly.

For the lower $\frac{\operatorname{sen} 1650}{30}=2567$ S. feet nearly.
And as the depth of the highest known floods are 13 feet at the former, 12 feet at the latter, the widths of waterwey should be 60 feet and 214 feet respectively.

The declivity of the bed of the river, however, at the upper crossing is 1 in 2291, while at the lower, where there is also an ample backwater from the Hindun in time of flood it is 1 in 3748; a somewhat greater width has therefore been given to the former; the waterway (vide Tabular Statement) being 2 spans of 40 feet each for the upper, 4 of 50 feet each for the lower crossing. The difference of level between the bed of the river and the Canal channel $i$ s, in the former case 22.4 feet, in the latter $34 \cdot 17$ feer.

The width of waterway above for the Canal in both is identical, viz, 190 feet (equal to the mean width of the carthen
channel) divided into two channels each 95 feet wide hy a wall 5 feet thick; roadway on the right bank 12 feet wide, on the left 5 feet.

Abutments, piers and waterwings on well foundations sunk 12 feet below the bed of the river; a flooring of kunkur blocks 3 feet thick under the arches, and on the bed for a width of 20 feet up and down stream ; the abutments, backed with puddled clay, and side slopes of the Canal channel, adjoining the wing walls, pitched with kunkur blocks; clay well puddled to be laid over the entire flooring under the brick on edge.

Along the centre of the embankments leading to the aqueducts on both sides, walls to be built of rubble masonry, (as shewn in Plan VII), 2 feet in thickness, level with the surface of full supply in the Canal at top, their foundations being laid one foot below the ground line. The Canal bed here not to be made up uniformly to the true level, a hollow to be left in the centre all the way along 20 feet narrower than the bottom width of the bed, divided at intervals of 100 feet by cross bars 10 feet wide at top; these spaces are left to be silted up after the admission of the water. The walls in the centre of the side banks are intended to prevent all risk of injury from the encruachments of rats or other vermin. A long embankment on the Baree Doab Canal, constructed and protected in this way, has had the water passing over it for some years, without giving the slightest trouble beyond the few repairs necessitated by the settlement of the earth.
74. The Regulator at Roorkee is $\mathrm{a}_{\mathrm{l}} \mathrm{m}$ wn in Sheet 23. The angle of divergence is projected at 45 .

The bridge across the alternative line to have an aggregate waterway of 184 feet divided into 8 bays of 23 feet each by piers 3 feet in thickness; on the old channel, 7 similarbays giving a waterway of 161 feet. For regulating purposes each bay to be divided into 3 parts by short pipes 2.5 feet thick, entending up stream to a length of 5 feet from the bridge head wall, across which will be thrown light arches to form a pathway on which the lifting machinery may be worked. The width of the sluice gates will thus be 6 feet. The object of placing the regulating apparatus thus clear of the bridge is to allow the boiling and eddying, which always is found more or less under such circumstances, to be partially deadened in the wide bays under the bridge arches before issuing into the earthen channel below.

The difference of level between the water surface at full $\frac{\varepsilon}{4}$ supply, up and down stream being, in the old line 8.8 feet, in the alternative 2.8 feet, masonry weirs are projected across the flooring of both Regulators close to the sluice grooves of a height sufficient to maintain the surface of the water up stream at its normal level. Lock chambers are designed, similar to those at the overfalls, on the left and right bank in the old and alternative lines respectively.

The flooring under the bridges to be depressed 2 feet beluw the bed level, and extended to a length of 100 feet below the down stream head wall, terminating in a curtain ten feet in depth ; a talus of boulders 10 feet in width, 2 feet thick, to be added for further protection.

A drainage cut leading into the adjoining valley of the Solanee has been estimated for to carry off the spring water when laying the foundations.
At all the bifurcations the Regulating Bridges have been placed as close as possible to the points of divergence, the actual position being determined by the width necessary for the bridge approaches between the branches, and the curve in the channel leacing to the entrances of the locks.
The aggregate waterway under the bridge arches, is, in every case, equal, or nearly so, to the mean width of water section at full supply in the channel below.

The sluice arrangements are identical in all.
At the Junction Branch Head, 61st mile, a drop occurs in the bed. Overfalls are designed at the heads of both Branches in lieu of one descent immediately above the point of divergence, for regulating purposes; in the latter case, dam bridges, with numerous piers and narrow openings, would have been necessary in place of the wide arched bridges below the falls as now projected (vide Sheet 24.)

The depth of fall would also be restricted to that required for the junction line $3 \cdot 26$ feet, necessitating an additional fall lower down on the alternative line.

Lockage into the junction branch is provided for on the left bank of the regulating bridge.

The bridges at the extremities of the tail basins are of exaotly similar design to those constructed elsewhere on the line for purposes of cross communication only; towpaths 7 feet in width and a clear headway of 10 feet are provided for on the junction. On the other side, the aggregate span of the bridge is equal to the mean width of the water section in the channel below, the arches being sprung from the level of the full supply in the Canal.

At the Bolundshuhur and Koel Branch Heads the fall is placed above the bifurcation, no navigable communication being necessary, and consequently the height of the arches of the dam bridges less. In the former case an escape head is projected on the right bank, between the fall and the regulators, the effects of the rush of the water through which will be in great measure neutralised by the wide basin forming the Canal channel above.

Navigable communication with these branches has not been provided for ; if found desirable hereafter, cross channels may be cut at a small expense to connect them with the old trunk line.

Escape lleads.
75. At the 51 st mile, the upper escape into the Kalee Nuddee diverges. Here the works (vide Sheet 28) consist of a fall, $\mathbf{6}$ feet in depth, with a dam bridge below, the escape head being placed between the two on the left bank; the position of the overfall in this case being determined by the expediency of diminishing the height of drop at the tail of the escape as much as possible. The escape head itself is designed similarly to the regulator at Roorkee ; arches of bridge 23 feet span, sluice openings 6 feet each in width, the aggregate waterway through the latter being 90 feet.

The second escape head from above the Bolundshuhur Branch bifurcation has a waterway of 60 feet in openings of similar width.

Overfalls at the extremity of each escape are designed, as shewn in Sheet 29 ; the drop there on the upper escape being 27 feet, on the lower 27.83 feet; gratings and cisterns below the lower bed as for the Canal overfalls; deep foundations on wells. The height of fall may be thought hazardous; one, however, of somewhat similar design, 17 feet in depth, on one of the Baree Doab Canal Escapes, has now been in existence for some years, and has always acted very satisfactorily. In fact, where the height of drop is considerable, the water falls almost vertically, so that it is only necessary to provide against violent action immediately at the foot of the fall which, with a sufficient depth of backwater, is not a difficult matter.
The drop wall should be carefully backed with puddle, as shewn; and the cistern floor, if stone be not procurable at moderate cost, with brick on end of the very best workmanship and material.
Bridges for cross communication are provided on the lower escape ; one for the East Indian Railway.
76. The general designs for the bridges on the alternative line are given in Sheets 32, 33, and 34. In the upper section, navigation is provided for by a clear headway of 10 feet above the level of full supply, and towpaths under the arches 7 feet wide on either bank. The aggregate width of waterway is projected equal to that of the water section in the earthen channel; arches below the junction branch head sprung from calculated surface level of full supply.

The widths of roadway between the parapets have been determined as in Sir P. T. Cautley's original design, viz. :For the main lines of road ... ... ... ... 25 feet.
District roads regularly lined out and constructed ... ... ... ... ... ... ... 20
Village communications ... ... ... ... 18 ",
Invert arches, to form the floorings between arches, sprung from the level of the bed; foundations of waterwings and curtains laid from 6 to 8 feet below the bed level. With the reduced velocity a boulder or kunkur talus will hardly be necessary. Under the metalling of the bridge above, a layer of brick on edge, or kunkur masonry of similar thickness, to be laid from end to end between the kerbs.
77. For the passage of the minor country drainages, calverts under the channel are designed as shewn in Sheets 35, 36,37 , and 38.

The admission of drainage water into irrigation channels should never, I think, be allowed unless it be impossible to dispose of it otherwise. Only one instance where this is unavoidable occurs on the alternative line, at the 16th mile, for which an inlet of 10 feet waterway is designed. The waterway for the culvert to carry off the floods of the Seela Nullah, mile 8, has been determined from that of an existing bridge over it, close to the crossing point of the

Bridges.

Drainage Culverts.

Canal. The proper waterways for the others it is difficult to fix accurately, from the utter impossibility of ascertaining in so limited a time the quantity of water to be provided for at each drainage line. They have been approximated to as nearly as the data procurable wculd admit ; but this, I confess, is the most unsatisfactory part of the entire project.

Rajbuha Inlets and
Outlets. Outlets.

Chokies.

Well Foundations.
78. Inlets and outlets have been provided for all rajbuhas or main watercourses, at or near their present crossing points ; the general design is shewn in Sheet 37.
79. Accommodation for chokeedars, or watchmen, and store room for sluice gear, \&c., has been provided at each fall or dam bridge. The general design for these buildings is shewn in Sheet 38. The arched masonry roof, in lieu of a flat one supported by wooden rafters, will, it is considered, be more economical in the end, from its superior durability; and the estimated cost is little greater than that of similar structures on the old plan.

First class Chokies on the standard plan of those on the Ganges Canal have been estimated for, at intervals of 12 miles on the upper section (or first 61 miles) only. Two second class Chokies between each. Below the junction branch head the line will run so close to the present main channel that separate first class accommodation seems unvecessary; second class chokies only have accordingly been estimated for, at intervals of about 5 miles.
80. Metalling has been estimated for on all bridges, and for a certain length of the approaches on either side.

The deep foundations of all works from the 123 rd mile downwards have been projected on undersunk wells, and estimated accordingly; the longitudinal section shews that the surface of the springs all along this portion of the line stands either at, or close to, the proposed level of the bed.

Sixty-seven bridges, including those at falls and branch heads, have been estimated for, giving crossing points at arerage intervals of $2 \frac{1}{2}$ miles on the entire distance, 165 miles, from Roorkee to Nanoon.

No ghâts, either in connection with, or separated from, the works, have been estimated for.

Here the descriptive detail of the alternative line closes; with the aid of the plans and the tabular statements in appendix B., I trust it will be sufficiently intelligible.

[^0]to be thoroughly repaired; and to be cut down, wherever the height is excessive, to a uniform level above the bed.

At the overfalls, a drop wall to be built, as described for the remodelling, the entire width of the bays, on the present ogees, under the down stream head wall of the bridge; bridge piers to be lengthened, and new piers constructed similarly for the support of gratings of a length suitable to the reduced depth of water. The lower floorings to be remored to a depth of four feet for a length of 40 feet below the new drop walls, thus forming a cistern below the level of the bed; with the reduced volume of water falling into this cistern, it will be sufficient protection to cover the flooring with a layer of brick on end.

At the Futtehgurh Branch head, the bay on the eastern bank of the Futtehgurh Branch to be left free for the passage of boats; iron grooved uprights and wooden sluice gates to be attached to the other arches as designed for the remodelling, the closure when necessary of the cistern bay being effected by an iron drop gate.
To the regulating bridge on the main channel no additions appear necessary. When the Futtehgurh Branch is taking off its full supply, it will be necessary to shut up several of the openings in the main line regulator entirely, for which the present sluice apparatus will answer.

Under the left arches of all the bridges from Roorkee to the Futtehgurh Branch head and from the Jutpoora Bridge to Nanoon, 40 in all, towpaths will be required 7 feet in width. Alterations at the Cawnpore and Etawah Branch heads, the same as for remodelling.

## SEPARATE NAVIGABLE LINE.

82. The main lines of artificial irrigation channels in flat countries are, as a rule, and for obvious reasons, carried along the several watersheds, independently of the positions of towns and villages; and this is also the most economical course, both as regards cost of construction and after-maintenance, the interference with the flow of the natural drainage being the least possible. When, however, the object is solely navigable communication, a Canal would, it is manifest, lose much of its value were it not brought close to the large markets. The actual course of the proposed navigable channel from Roorkee to the head of the Cawnpore and Etawah Branches has therefore been determined by the position of the larger towns situated in the general direction, wherever the levels of the ground in their vicinity would allow of carrying the bed either horizontally or on a continually descending slope. This of course raises the cost materially, as they, in common with all towns of any magnitude, are invariably situated close to the drainage outfalls, where water either flows or collects in considerable quantity at some season of the year.
The line of levels on which the estimate is based (vide Plan IV.) is identical with that of the alternative channel from Roorkee to the watershed between the Hindun and West Kalee Nuddee at the 17 th mile, whence it diverges to the westward

Course of line, and how determined.
of Deobund, passing close to the town ; thence it runs in nearly a direct line to Mozuffurnuggur, crossing the West Kalee Nuddee about 3 miles above the town; it joins the Ganges Canal a little above the Kutowlee Bridge, whence it issues a few hundred feet lower down; and taking a slightly circuitous route parallel to the Canal for some miles, to avoid the drainage into the Aboo Nullah, reaches the city of Meerut ; from this it passes along a watershed down to Haupper, aud about 5 miles further on crosses the Choya Nullah, a large feeder of the East Kalee Nuddee ; a little in advance of this the line reaches the central watershed of the Doab, and, passing close to the westward of Bolundshuhur, rejoins the Ganges Canal close to the Grand Trunk Road Bridge; from this it turns off to the town of Khoorja, where it again meets the proposed course of the alternative line, from which it finally diverges at the 138th mile, inclining to the westward to reach the town of Koel, from which it passes in a direct line to its junction with the Canal above the Nanoon Regulator.
Section of channel and declivity.
83. The dimensions of the water section of channel, and the waterways of bridges and locks, have been fixed at the minimum which I believe to be absolutely necessary for boats propelled by cattle haulage, which, I have before stated (para. 41), seems to me best adapted to the babits of the people. It has, however, the advantage, in comparing the cost with that required to give efficient navigable communication along irrigation lines, of showing the smallest outlay at which it is possible to construct a separate channel for the navigation only. On the main lines of the Ganges Canal, the width and depith of water, it must be borne in mind, will admit of the passage of small steamers. In the annexed Statement are embodied certain details of Canals in different countries, from which may be gathered the dimensions considered necessary for various kinds of craft.
( 49 )
Tabular Statement showing certain details of various Canals.

|  | Namms. | Channrl. |  | Locks. |  | Remaris. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Country. |  |  |  |  |  |  |
| America ... | Chesapeake and Ohio | 48 |  |  |  |  |
| Ditto . | Welland | 58 | 8 | 110 | 14 | Recommended by Board of Engineers in 1826. |
| Ditto ... | Ditto $\quad$... | ... | - | 130 | 32 | Below St. Catherine's. |
| Ditto ... | Ditto | ... | ... | 130 | 24 | At Port Colborne on Lake Erie. From Report of |
| Ditto ... | Ditto ... | 70 | $8 \cdot 5$ | 110 | 24 | For Schooner Traffic. ${ }^{\text {Recommended }}$ in ${ }^{\text {a }}$ (1832, by Lieut.Col. |
| Ditto ... | Ditto | ... | .-. | 180 | 45 | \% Steamers. $\quad\left\{\begin{array}{l}\text { 1837, by C. E. } \\ \text { Section. }\end{array}\right.$ |
| Ditto ... | Ditto | ... | 9.5 | 150 | 26.5 | Notes.) <br> For propellers of from 300 to 500 tons, (from Lieut. Haig's |
| Ditto ... | St. Lawrence | $\cdots$ | 9 | 200 | 45 | For Steamers, (ditto.) |
| Ditto $\quad$ Dito | Ditto | 140 | 10 | 200 | 55 | Will pass Steamers 180 feet $\times 52$ feet. \} ieut Col Philpoter |
| Ditto | Erito Canal | 48 | 5 | 100 | 20 | Not wide enough for large Steamers. $\}$ Lieut. Col. Philpotts. |
| Ditto ... | Morris Canal, New Jersey... | 32 | 4 | $\cdots$ | $\ldots$ | Boats $8 \cdot 5$ feet broad, 60 to 80 feet long, from 25 to 30 tons |
| Ditto ... | Pennsylvania ... | $\cdots$ | .. | $\cdots$ | $\ldots$ | Boats 12 to 15 feet broad, 80 feet long; when loaded with full |
| France ... | North-Western Districts ... | ... | 5 to 7 | 100 | 18 | number of passengers draw 1 foot of water. Haulage by horses. Boats from 500 to 200 tons. |
| England ... | General Sections .. |  | ... | ... | 7 | Boats 30 to 35 tons. |
|  | Ditto ... | 40 | ... | $\ldots$ | $\left\{\begin{array}{l}14 \\ 18\end{array}\right\}$ | $\}$ Ditto 60 to 120 tons. |
| Scotland ... <br> India ... | Forth and Clyde ... <br> Madras Coast Canal ... | 40 | 9 ... | 68 | 15 20 | Screw Steamer navigates throughout. |

The section here estimated for is 20 feet wide at bottom with side slopes of 1 in 2 , and depth of water of 5 feet, giving a total width at the water line of 40 feet.
The declivity of bed, 1 in 12,000 , is calculated to give a mean velocity of current in the channel of 1.5 feet per second (or about 1 mile per bour), the least, as far as can be ascertained at present (vide Appendix D) which will prevent the growth of weeds; it appears to be also, from the experience on the Madras Canals, (vide Report by Sir A. Cotton dated 16th June 1852, and elsewhere,) the maximum for economical navigation.
The general form of banks is shown in Plan VII. The towpaths 1.5 feet above the surface of full supply, 20 feet in width, to afford space for the passage of wheeled vehicles all along the line.
The depth of water will doubtless, be diminished materially at certaiu points by absorption and evaporation; no feeders from the main Canal, however, appear necessary ; the requisite supply can always be obtained from some of the numerous rajbuhas which intersect the line from Mozuffurnuggur downwards.
84. The general design of the locks, 30 in number, is given in Sheet 39 Plan $X$. They are projected in two chambers, each 16 feet in width, with a clear length of 100 feet; a bridge carried across both chambers below the lower gates with a clear headway under the arches of 10 feet; the width of roadway determined as for ordinary bridges. The gates of iron with sluices in each leaf; opening and closing effected by chains passing under pulleys in the side walls and worked by crabs from above. Inlet and outlet passages of masonry provided round each set of gates on the land side. The detail of the masonry work is almost identical with that of the present Ganges Canal Locks.

The depth of drop is in general 6 feet, but ranges from 281 to 12 feet.

The design for locks at the departure from and junction with the Canal are shewn in Sheet 39.
85. Three aqueducts are required; two over the West Kalee Nuddee, the third over the Choya Nullah above Bolundshuhur; the general design is given in Sheet 42. The waterways for the floods of the rivers are determined as for the alternative line; the flood discharge to be provided for at the crossing of the Choya being estimated by a comparison with the actual waterway of the bridge across the East Kalee Nuddee at Bolundshuhur ; the waterway of the bridge on the Grand Trunk Road, which crosses close to the line of levels, would have given a more accurate basis for the calculation; the actual dimensions were not, however, procurable at the time.

The width for the Canal channel is 30 feet in the clear with towpaths 5 feet wide on one side, 10 feet on the other, the additional width on the latter being given to admit of the passage of wheeled vehicles.

The foundations of piers, abutments and waterwings to be on wells sunk 12 feet below the beds of the nullahs, kunkur
flooring under the arches, and the details of the masonry work similar to those of the alternative line aqueducts.

Masonry walls in the centre of the embankments leading to these aqueducts, to prevent the encroachments of vermin, are not included in the estimate : the risk of injury, with so small a depth of water, does not appear to me sufficiently great to justify the extra expense; cores of sand in a similar position filled in as the banks are made up would, I think, answer every purpose; leakage need not be apprehended except in soils saturated with "reh" or other deliquescent salts.
86. Culverts and syphons for the passage under the Canal of the country drainage will be similar to those on the alternative line, the only difference being in length of archways, which is here invariably 89 feet. The tabular statement shews the several positions and waterways of these works.

Syphons for the passage of the various rajbuhas or main irrigation water-courses under the Canal are estimated for ; the general design is shewn in Sheet 43. Cross communication is provided at 87 points along the entire distance- 164 miles giving one passage at an average interval of about 2 miles : on the alternative line the interval was about $2 \frac{1}{2}$ miles, the difference being owing to the latter passing at some distance from almost all the more crowded thoroughfares: 28 of these are provided for at locks, the remaining 59 by bridges, the general plan of which is figured on Sheet 41. The headway under the arches is 10 feet; towpaths 6 feet in width on each side, width of waterway 22 feet, thus making the span of the arch 34 feet over all. Depth of curtain and waterwing foundations 6 feet in general. The details of floorings, roadways, \&c., identical with those on the alternative line.
87. Store-rooms or chokies as before described are estimated for each lock and point of junction with, or departure from, the Main Canal.
88. From the 118th mile downwards the deep foundations of all works in the channel are estimated on wells sunk 8 feet below the bed level, water lying close to the surface everywhere.

Drainage Culverts, \&c.

Chokies.

Well Foundations.

## RATES POR RSTMMATES, GATGES CAKAL

E. B- Hatos in Riryen and deeimale.

Dimentinas in Eagitibl faes and deximale.

| Dascrarmos. |  | (3) | Fer |  |
| :---: | :---: | :---: | :---: | :---: |
| Broximuor- |  |  | Cabic fort |  |
| Ather line, 1at Section | 2 | 275 | 1000 |  |
| \% Endbaskuent at Kalse Nudise | $\cdots$ | 800 | * |  |
| \% End Sertion | - | 230 | $=$ |  |
| \% 36d \# | - | 250 | $=$ |  |
| Hatres exearation | $\ldots$ | 22 | , |  |
| Por formilatione | - | 325 | $=$ |  |
| Manriguble line | - | 225 | $=$ |  |
| Widening prasent Caugss Canal | * | 200 | $=$ |  |
| Approsehes | - | 30 | \% |  |
| Barcxorax- |  |  |  |  |
| Phain, heary | $\cdots$ | 20 | 100 |  |
| Tine | - | 24 | $=$ |  |
| Brick on elge | - | 25 | $=$ |  |
| Arching, heary (ineluling castrings) | $\sim$ | 32 | \% |  |
| Da light ( ${ }^{\text {(iftto) }}$ | - | 28 | $=$ |  |
| Inverts | $\cdots$ | 25 | $=$ |  |
| Kenmes Masoner - |  |  |  |  |
| - Plaín, heary |  | 17 | $=$ |  |
| Arching heary (inclating ceutrings) | - | 20 | = |  |
| Arching hesry (imelaling coutring) | . | 28 | \% |  |
| Ditto, light (Ditto) | --- | 24 | , |  |
| Boulder Masonry May | - | 14 | \% |  |
| Robble (Kunksr) Mawary | ... | 14 |  |  |
| Mooring (dry) lowkur | $\ldots$ | 8 |  |  |
| Metalling Piosdvays | - | 6 |  |  |
| Puilding | -.. | 10 | 1000 |  |
| Agrestome flooring | -- | 3 | Cubie ft |  |
| Delhi ${ }^{\text {D }}$ Do. | *- | 3 |  |  |
| Tlooring boulders (dry) | $\ldots$ | 7 | 100 | Carried by boats |
| Pitching kmour or boulders | ... | 10 |  |  |
| Undersinking wells, " large" | ... | 70 | Rg. ft |  |
| Watcoun "mmall" | $\ldots$ | 5-0 |  |  |
| Well Carbs, " "large" (17) Do. | $\ldots$ | 90 20 | each. |  |
| Thateling slopes | $\ldots$ | 05 | $100 \stackrel{3}{\mathrm{~S}} \mathrm{q}$. ft |  |
| Planking Z $^{\prime \prime}$ over green work | $\ldots$ | 02 | $\mathrm{Sq} . \mathrm{ft}$ |  |
| Robble, filling in wells | $\ldots$ | 12 | 100 cub. ft |  |
|  | $\ldots$ | 25 30 | Rg. ft each. |  |
| Pilas, 18 | $\ldots$ | 30 | each. <br> Cubic ft. | Includingdriving. |
| Poulder crib work | ..- | 12 | 100 |  |
| Boulder flooring dry (on alternative line) | $\ldots$ | 10 | » |  |


| Description. | @ | Per |  |
| :---: | :---: | :---: | :---: |
| Stone Grooves | 10 | L. ft. |  |
| 1st Class Chokies, Ganges Canal pattern | 3200 | each. |  |
| 2nd do. do. ${ }^{\text {d }}$ d | 800 | " |  |
| Sal Kurries, 10 feet long | 1.5 |  |  |
| Puddled clay backing | 3.0 | Cubic ft. 100 |  |
| Removing old boulders or kunkur flooring | 7.0 | 1000 |  |
| Demolition of brickwork | 10 | 100 |  |
| Piling, 12 feet long, including driving | $5 \cdot 0$ | L feet. |  |
| Requlating Bridgr Gear |  |  |  |
| Cast iron uprights, grooved do. Grooves ( 0.5 feet wide) | 9.4 5.0 | L. ft. |  |
| Windlasses, 6.5 feet long | 22.0 | each. |  |
| Do. 20 do. | 66.0 |  |  |
| Iron drop gates | 5.0 | Sq. ${ }^{\text {n }}$. |  |
| Wooden do. (width, 6 to 7 feet) | $1 \cdot 25$ |  |  |
| Lifting Chains, small | 0.9 | L. ft. |  |
| Do. large | 10 | " |  |
| Axle Blocks, large | 12.0 | each. |  |
| Do. small | 50 |  |  |
| Sleepers | 3.0 | Cubic ft. |  |
| Hooks for lifting sleepers | $7 \cdot 0$ | each. |  |
| Lock Grar |  |  |  |
| Lock Gates, Iron | 6.5 | Sq. ft. |  |
| Sluices in gates | $125^{\circ}$ | each. |  |
| Crabs | $135 \cdot$ | " |  |
| Pulleys | 5. |  |  |
| Chains .. | 1.0 | L. ft . |  |
| Fall Gratinga. |  |  |  |
| Iron girders ( 11.5 to 15 ft . long, 7 to 9 feet apart) | 4.25 | L. ft. |  |
| Iron grating bars $0.0 \times 0.5 \times 0.5$ | 0.875 |  |  |
| Wooden do. do., $29.0 \times 0.5 \times 0.5$ | 21.75 | each. |  |
| Wall plates, iron, complete ... | 6.25 | L. ft. |  |

## RATES, MATERIALS, \&c.

89. The annexed table shews the rates on which the estimates generally have been calculated. They have been fixed by a comparison of the actual rates of work now obtaining in the several Canal divisions, or, where the descricition of work had not been previously executed there, from the most reliable sources.
90. Bricks must still be our mainstay in the construction of masonry work, for, notwithstanding all that has been said and written about the sandstone of the Sewalik Hills, in the neighbourhood of Hurdwar, all the information we have hitherto been able to glean both as regards quantity and quality as well as cost, is very unsatisfactory. The officer appointed to search for it, Mr. J. Kelly, was unable to commence operations before the beginning of last A pril when but three months remained till the setting in of the rains, during which, from the deadlg nature of the climate, no European or native can, without risking his life, continue in the neighbourhood. His researches, however, prosecuted vigor usly for the time they lasted, have only c.nfirmed the general correctness of Sir P. T. Cautley's views. Sandstone of sufficiently good quality is to be found but in small seams here and there at considerable intervals; good stone close to the surface, the quality det-riorating as the quarrying penetrates into the hill side; and as to cost, it was found that blocks of sufficient size and weight for our purposes could not be turned out, hammer dressed, and loaded in boats, for less than 14 annas to one rupee per cubic foot. If to this be added the cost of carriage to the works, 8 annas (or 0.5 rupee) per 100 cubic feet per mile by carts, or 5 annas ( 0.31 ruper) by boat down the Canal, and setting, I do not think it can be safely rated in the work at less than Ks. 2 per cubic foot. The Delhi sandstone, latterly used for repairs to the fall flourings, does not cost less lail into the work than Rs. 3 per cubic foot; and this in cisses where the distance of carriage is 70 miles, 18 of which is by carts, the remaining 52 by boats up the Canal. These rates might certainly be improved on were quarrying operations carried out on a large scale ; but even were the rates of the Hurdwar stone to come down so low as one rupee per cubic foot laid into the work, the cost of the limited extent of stone covering estimated for the Muhmoodpoor Fall flooring would equal that of a grating only ; and with the latter protection, and a cistern below the fall, it has been sliewn above that good brick covering has been found to last well under similar circumstances elsewhere. Now, as first rate brick on edge (or on end) facing can be executed for Rs. 25 per 100 cubic fret (or 4 rupee per cubic foot), it appears to me that the substitution for it of stone, except under very peculiar circumstances, to ensure a greater degree of safety, would be an unjustifiable expense. The only positions, therefore, for which it has been estimated in these projects are the tops of cistern walls, \&c., and the floorings immediately under the gratings in the present overtalls, where the formation of a cistern below the level of the bed appears impracticable. The cost of such a covering at the Muhmoodpoor and similar falls, upwards of Rs.

16,000 for each, forms, it will be seen from the estimate, a very large proportion of the total outlay.
91. All the rates of these estimates are somewhat in excess of those at which work is now executed on the Canal. Plain brickwork costs from Rs. 16 to 18 per 100 cubic feet. The rate for this in the estimate is Rs. 20. On the alternative line, a slight advance on existing prices may ba expected on account of the large demand for materials; and if the remodelling of the present Canal be adopted, the excess will be necessary to cover the cost of pumping in laying foundations.

These rates are much in excess of thuse obtained in the original construction of the Canal (except in the works from Hafiznuggur upwards). Referring to the Ganges Canal Report, Vol. II., pages 179.182, the average rates of building were, at Muhmoodpoor Falls-


Now, although, as is stated there, old bricks, from some ruins in the vicinity were used to a certain extent in the works, these rates are less than really good brickwork can possibly be constructed at, and the consequence is manifest in the inferiority of materials and workmanship.

At Hafiznuggur Falls, the rate for masonry was Rs. 17-3-11 per 100 cubic feet, and at the Puttree Works, (page 87)-


The rate for arches over 30 feet in span, Rs. 32.0 per 100 cubic feet, including centrings, is based on that for brick on edge, Ks. 25 per 100 cubic feet, the addition for the centrings being obtained from the cost of bridges above Roorkee, where the same wooden centrings were used for several arches. The construction of the Punjab Railway, the route of which is traced on Plans I and II, will not, I think, materially, if at all, affect the rates of labour or materials; there are no masonry works of any magnitude along the line, and the earthwork is nowhere heavy. In the districts affected by the East Indian Railway opposite Delhi, I found by enquiry on the spot that the rate of coolie hire and excavating rose during the construction of the very heavy railway embankments across the vallies of the Hindun and Jumna rivers, but fell again immediately on their completion, and at the date of my enquiries it was at its usual figure. In fact, a permanent rise in the price of labor can only follow a general increase of wealth and prosperity in the country ; the construction of the largest public works can only affect it locally and temporarily.

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 where, and is buisg gracmin one of larger diureztions





 us the Mulimoodpoor Fails, 3:18 ace, ar eve farion

 distant, close to the Weat Rajee Syijuee I is if too ope a grain to be of much value for is great; but it appears wostavd iè erexive accias $f$ water well, and would answer adrimity fre raieie mont Prom this downwards the material is proctrine witi more or lem facility; but really good kankur, of onmpacs textare which will stand pressure, is not fonod mais above the Bolendshuhur Branch Head at the $110 \mathrm{~m}_{\mathrm{a}}$ miz

In the original construction of tie Ganges Canal Works below this point it was extensively ased ; a large proportion of the masonry on the line of the East Incian Raiiway is 000 structed with it; and from the present condition of all the works built of it, it appears preferable to brick, if care be taken in the selection according to the amount of pressare or wear it may have to surtain. The liahommedan engineers of bygone years used it largely in their works wherever it was to be found ; the roadway of one of their bridges, still in good pretervation close to the once extensive city of Sirhind on the old line of road between Delhi and Lahore, is paved rith kunkur, brought, it is said, from a quarry still in existence in the valley of the Sotlej, some 20 miles distant, which eren now is worn through in very few places, though till within the lact fow years a heavy traffic was, for, we may say, centuries, constantly paseing to and fro over it. The largest proprortion of the masonry in these projects, except heavy arching, has accordingly been estimated at kunkur rates wherever the unaterial in known to exist; the rates are based on thoee actually obtaining on the Canal Works, with an addition to cover the price which will probably have to be paid
to the owners of the land in which the material may be found, the demand of late years having given it a value it did not formerly possess.
94. Timber of all descriptions has risen greatly in price, owing to the heavy demand for railways and other large public works throughout the country. The rates for iron and framed wood work are those at which the Roorkee Workshops can now supply them.
95. In Appendix G. will be found a statement of the total outlay on the Ganges Canal from the commencement Wood and Iron-work. up to 30th April 1863, divided into two periods, before and after the 1st August 1854, the date of the introduction of the Central Office system of Accounts. The Canal was formally opened three months previously, in April 1854, so that the expenditure shewn for the first period may be taken without much error as that incurred before the commencement of irrigation. The percentage of establishment on the total outlay, including contingencies, during that period was 9.9 (say 10) per cent. On the Baree Doab Canal Works in the Punjab it was 10.6 per cent. In present estimates I have taken it at 7 per cent, as the new works lie either on existing lines or in the Canal districts where the services of the present establishment will be available to a certain extent. The allowance for contingencies, rated on the estimated cost of the works, 5 per cent for remodelling existing lines, is intended to cover all expenditure on such items as conservancy arrangements, Office contingencies, temporary shelter for work-people, tools, \&c., and possible excess of cost from unforesen circumstances; with the addition on the alternative and separate navigable lines, of compensation for trees, wells, \&c., taken up with the land, and "survey and lining out," where it is increased to 8 per cent. The last item has been taken from results on the Baree Doab Canal Works, (where it was about 10 per cent) allowing for difference in labor rates, as I do not know the exact nature of the charges included in the Ganges Canal Statement under the heads "Sundries" and "Contingencies."
96. I have stated above in para. 91 that the rate for masonry in remodelling the existing lines has been taken somewhat in excess to cover the cost of pumping in the construction of foundations below the bed where springs will in every case be more or less troublesome. From notes made by Lieutenant Forbes, R. E., Executive Engineer of the Northern Division, Ganges Canal, when repairing the floorings of the present overfalls, where the surface of the springs is above the bed level, the cost of baling averaged Rs. 3 per 100 cubic feet of masonry laid in. The estimated rate for all the masonry work in the remodelling project is about Rs. $1 \frac{1}{2}$ per 100 cubic feet in excess, which will, I believe, be ample to cover all expenses in this item.

Pumping or baling.
Precentage for Contingencies and Establishment.

## COMPARISON OF PROJECTS AND TOTAL COST.

Advantages and disadvantages of alternative project.
97. The advantages of a duplicate line below Roorkee are thus summarized in the Memo. of the Committee (Appendix"A.) "First, the facility of construction, and of forming a reliable estimate of cost and time of construction; and second, the very great convenience that may be anticipated from the possession of a duplicate line of main channel throughout so great a distance, which would admit of a supply of considerable amount being sent down to the lower parts of the canal, while either of the alternative main lines was closed for repairs." Sir P. T. Cautley in his Memo. (vide same Appendix) adds another, viz., "dealing with more manageable masses of water."

The disadvantages are stated to be, that the alternative line "in part, at least, must probably be regarded as not available as an irrigation channel, inasmuch as the supply of water is hardly sufficient for the works already designed and in course of completion in the lower Districts of the Doab; that it involves the passage of the drainage of large areas of the Doab"; and -" further, although the works will be less difficult, they will in absolute quantity be heavier, and require more material, the material again requiring land carriage to a greater extent than would be the case on the open canal." To these are added the waste from absorption and evaporation (which would be nearly double that in a single channel) and the largely increased expense of subsequent repair and maintenance, including establishments.

The absolute necessity of maintaining uninterruptedly the present volume of water to supply existing irrigation, unless there be very cogent reasons to the contrary, is assumed by all.

The present channel can, by careful watching and timely repair, be made to carry this supply until the completion of the duplicate line ; but the latter, it must be borne in mind, will only relieve the present line below Roorkee. It has been shewn above that extensive alterations are required in the channel above that point, where there is no possibility of making a supplementary line; a regulating bridge must also be constructed across the present channel at Roorkee, at the proposed bifurcation of the duplicate line. Now the alterations to the channel and works above Roorkee must be effected with a dry canal, as it would be economically, if not physically, impracticable to divert the canal supply even temporarily round such works as the superpassages at the Puttree and Ranipoor and the Escape Dam at the Rutmoo: even at Roorkee, this would be no easy matter, hemmed in as the canal is, by buildings, \&c., on either side, and in such close proximity to the edge of the high bank of the Ganges valley.

The new works and alterations in remodelling the line below Roorkee, at least so much of them as lie below the full supply level, may, I think, be completed, if materials, \&c., be ready on the spot, in very nearly the same time as those in the Ganges valley. In this point of view then there does not appear to be much in favor of the duplicate line. The foundations of the works in the latter however would, with the exception of the aqueducts, be laid on a dry subsoil for the first 121 miles, below which the section shews springs
close to the surface, and wells must be undersunk for all deep foundations on the last 44 miles to Nanoon.

The aqueducts and embankments across the Kalee Nuddeed valley are necessarily large and massive, and judging from the experience on other similar works, could hardly be completed under four or possibly five years. The syphons and culverts for the passage of the minor country drainages may hereafter prove insufficient, as it is impossible to foresee how much additional duty may be thrown on them when the subsoil has become fully saturated by constant irrigation.
98. The volume to be dealt with does not appear to me to affect the question much if the velocity in the earthen channel be reduced to a safe limit, and the form and depth of the overfalls be such as to prevent too violent action on the masonry; while at the same time confining the irregular action within the limits of the walls or other protective works. If the existing line be remodelled so as to effect this, (and there is no reason against it except difficulty of execution) the trifling repairs which might be necessary could be made by shutting the canal for a few days or a week or two at farthest; nor would this, I believe, be required except at long intervals.

The additional loss by absorption and evaporation is, I think, a weighty argument against any division of the supply in the trunk line. Though the total wastage will eventually, when the soil becomes fully saturated, be a fraction of what it appears to be now, absorption there must always be to some extent, and the evaporation is of course constant. Data are not in existence for the exact determination of either, but the proposed division of the supply would at least double it, whatever the amount may be.
99. For the upper 60 miles of its course the alternative line would be useless as an irrigating channel, no water being available for lands not included in the original project, the limits of which it does not reach till opposite Sirdhannah, at the lower passage of the West Kalee Nuddee.
100. The reduction of the supply in the present channel Effect on existing irrito a depth of 4 feet must, to a certain extent, affect the irri- gation. gation more or less from Roorkee to Sirdhannah. Above the Futtehgurh Branch head, the depth of full supply at present averages about 6 feet, which would be thus reduced by two feet. For this there is no remedy in the outlets above the Hafiznuggur falls ; those lower down may be removed to the fall next above, and so obtain a larger supply by a drop at the head ; from the Futtehgurh Branch head to the Sulawur falls the supply must be obtained by planking up the bays of the overfalls Below Sirdhannah, I have elsewhere shewn, the present irrigation will be unaffected by the changes in channels.
101. "The obvious advantage of remodelling the present Advantages of remodelcanal," as stated in the Memo. above referred to, "is that it ling. will admit of the general design of the works being maintained almost intact, and will interfere very little with the system of minor irrigation channels already in existence." The excavation also would be small, and the quantity of material for masonry works very much less than in an entirely new channel.

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 2 n n．taice．Tifis will inctesse the expense by the loss of a parin water res：and onmersation which mast be given
 W．．．in dot bettr．and after ail a year＇s stoppage is not much in tistexisutese of wints destined to last periapis for centaries．

Scar：thes whole of the alterations for remodelling above the prossi：sricice of fuil supply can of course be execated viti 2 rurising canal．
Crat af and
$1 \%$ AId，las：ly，the comparative expense of the two orferes．The $\in$ timated eret of construction is shewn in the arerate artided to the Report．To this must be added for
 Cata iterm in Appendix H．）From statements given in the Cet re lefort of list January 1853，the average tax on lands， aa3 Metri：Lrjuncishuhur and Allyghur Districts，appears to be aly，ut Lupees 1.2 per acre；if we take double this for the utal value，and 10 years＇purchase as the selling price，the comprination will be within the mark at Rupees 24 per acre．

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(61 )
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To arrive at the total probable outlay on the works for remodelling we must add-first, the probable amount of remission of land revenue solely due to canal irrigation ; second, compensation to enable cultivators to re-open new wells or repair old ones for the supply of water to lands in cultivation previous to the receipt of canal irrigation ; and thirdly, the loss of a year's water rent in case of the canal being closed for that period.

The increase to the land tax now realized, solely due to irrigation from the Ganges Canal, is, from rough statements, kindly furnished to me by the District officers, as follows:-

| In Seharunpore | ... | ... | Rs. | 4,630 | 14 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mozuffurnuggur | $\ldots$ | ... | , | 91,158 | 0 | 0 |
| Meerut | ... | ... | , | 20,000 | 0 | 0 |
|  |  |  |  | 15,788 | 0 | 0 |

or say Rupees $2,00,000$ in the aggregate for all the districts down to Cawnpore.

The total area irrigated by the Ganges Canal channels during the last year $1863-64$ was 449,788 acres. "Kutcha" wells, i. e. without masonry lining, will answer all needful purposes for so short a period, especially as the level of the springs in the irrigated districts is nowhere far below the ground surface. One such well, it is estimated, is sufficient for the irrigation of six acres, and would cost about Rupees 10. We shall be well over the mark, I think, if we take half the present irrigated area as requiring new wells; the amount of compensation will then be-

$$
10 \times \frac{449,788}{2 \times 6}=\text { Rupees } 3,74,823 .
$$

The aggregate canal revenue during the past year 1863-64 was Rupees 7,73,390.

We have then for the alternative line-
Cost of construction as per Estimate, Rs. 70,67,380
Compensation for land occupied-Acres 10,625 @ Rs. 24

$$
2,55,000
$$

$$
\text { Grand Total, Rs. ... } 73,22,380
$$

For the remodelling from Roorkee to the head of the Cawnpoor and Etawah Branches-

Construction as per Estimate
Rs. $30,98,372$
Compensation and losses if canal be
closed for a year-
\($$
\begin{array}{llr}\text { Remission of land revenue } & & \begin{array}{r}2,00,000 \\
\text { Compensation for wells }\end{array}
$$ <br>

Loss of water rents \& " \&\)| $3,74,823$ |
| :--- |
|  |
|  Grand Total  | <br>

\& $" & \underline{44,73,390} \\
& & 44,46,585\end{array}$
shewing an excess of Rupees $18,75,795$ against the alternative line; nor does this represent the whole of the difference, the amount of loss by closure of the canal being debitable to the entire remodelling from the head, as well as to the portion below Roorkee.

As to the time which either project would probably Time probably occupicd take to complete, if men and money be available as required, in censtruction.
three years will, I think, judging from the tine required for the brickmaking, suffice for the remodelling; the alternative line must obviously be a longer business, depending mainly on the time required for the completion of the Kalee Nuddee aqueducts, which could hardly be less than five years.
Probable cost of diver- 104. Supposing the construction of new falls, the alterasions. tion of old ones, and adding arches to the bridges below Roorkee were practicable with a running canal, the cost of diversion may be estimated as follows :-

The estimated rate for a temporary 8 feet fall is, vide Estimate 180, Rupees 59 per running foot; for earthen channels above Futtehgurh Branch head, Rupees 4,250; below that point they would average Rupees 3,350 each. If we take 150 feet as the minimum width of water-way above, and 115 feet below the same points, we have-

Cost of temporary fall above Futtegurh
Branch head $150 \times 59$... Rupees 8,850

| Channel | $\cdots$ | $"$ | 4,250 |
| :---: | :---: | :---: | :---: |
|  |  |  |  |
| Cost of fall below do. $115 \times 59$ |  |  |  |
| Channel | $\ldots$ | $"$ | 6,785 <br> 3,350 |
|  |  |  |  |
|  |  |  | 10,135 |

And total for entire line from Roorkee to Nanoon, above Futtegurh Branch Head-


Grand Total, Rs. $1,91,660$
Navigation; general con- 105. In considering the advisability of forming navigable siderations. communications in connection with these irrigation works, either along the irrigating channels themselves, or by separate channels, the question of cost appears to me quite a secondary one. Whatever may be said of Sir Arthur Cotton's under-estimates of outlay and over-estimates of profits, the principle he has so long and ably advocated is undoubtedly true; that in no country,-and especially where the distances from the producing districts to the seaboard are long-can the material resources be fully developed without the aid of water communications. Railways can supply the wants of traffic in passengers and valuable merchandise of small bulk, better than any other known mode of couveyance, but they cannot now, nor cver will be able to carry the heavy and bulky raw materials, which form the staples of commerce in a country like this, at sufficiently low rates to admit of their being sold at the seaboard at remuncrative prices. European and American experience prove it-and even in England with its net-
work of railways, and not a spot 100 miles distant from the sea-the prices at which canal shares are quoted, shows that there too the water traffic in the interior must be very extensive. It is calculated that Indian cotton, to drive American cotton out of the English market, must be delivered at the seaboard at 4 pence per pound, or Rupees $13 \frac{1}{3}$ per maund of 80 pounds. Now the cost by rail from Agra to Calcutta, a distance of about 900 miles, including transit from the interior to the Railway Station, averages from Rupees 6 to Rupees 7 per maund, which would thus leave but $6 \frac{1}{3}$ to $7 \frac{1}{3}$ Rupees per maund for production, screwing, \&c.; while a rate of Rupees 8 per maund is considered as hardly sufficient to remunerate the actual cultivator. Compare this with the carriage of a 400 pound bale for 1,000 miles down the Mississipi for $1 \frac{1}{2}$ dollars ( $£ 0-6-3$ ), or about $\frac{5}{8}$ ths Rupee per maund, and it is easy to perceive that India, unless with the present exceptionally high prices, can never compete successfully with America in this article of trade.
106. Below Allahabad the Ganges is navigable at all seasons; boats of considerable size can pass up as high as Futtehgurh, except in a very dry year, when the passage may be closed for a couple of months.

Between Futtehgurh and Gurmuhteesur, navigation is stopped for 8 months in the year, except for small boats of from 50 to 100 maunds.

On the Jumna as far up as Agra, the boat traffic is considerable all the year round. For the districts above these points then, as far as the foot of the Himaylayas, water communication is needed. Whether this should be along the lines of irrigation canals, where they lie in the general direction of the country traffic, or by separate channels constructed expressly for the purpose, is mainly a question of comparative expense. The latter, however, would have so far the advantage in being carried close to the large trading marts, while the irrigation channels, as a rule, pass at some distance. The great drawback to any separate line scheme is the wastage by absorption and evaporation which is a clear loss to the irrigation.
107. The probable expense of separate navigable lines, it will be seen from the estimates, is much in excess of what would be required to make the irrigation channels navigable throughout. The proportion of the cost of the navigable part of the alternative line, due solely to the navigation, is Rupees $5,52,494$, while the cost of the separate channel for that distance, taking the average mileage rate of the channel from Roorkee to the Nanoon bifurcation is $64 \times 25,39,131$ $=$ Rupees $9,90,848$.

The latter sum, however, is considerably less than the actual cost of the separate channel there, the estimate for the aqueducts and their embankments at the Kalee Nuddee crossings alone amounting to Rupees 2,56,615, besides the excess in the lockage, thirteen descents out of the total number of thirty occurring in this portion of the line.

If again the total estimated cost duc to navigation on the
whole distance between Roorkee and Nanoon be compared, we have-

On the alternative line project $\quad .$. Rs. $6,06,069$
On the remodelled line ... ... ", 11,44,688
while the estimate for the separate channel
amounts to ... ... ... " $25,39,131$
To the probable cost of a separate channel along the general line of the Cawnopore Branch, a close approximation may be made thus-

Total cost of separate channel as per estimate, without establishment and contingencies ... Rs. 21,97,241

## Deduct:

Aqueducts and their embankments, Rs. 3,10,154

| Lockage, | $\ldots$ | $\ldots$ | ... | $7,51,505$ |
| :--- | :--- | :--- | :--- | :--- |
| Chokies, | $\ldots$ | $\ldots$ | $"$ | 10,899 |
| Bridges, | $\ldots$ | $\ldots$ | $"$ | $3,65,865$ |

Difference, Rupees $\overline{7,58,818}$.
The total length, exclusive of the embanked channels at the crossings of the Kalee Nuddee and Choya, is 160 miles.

We have then a mileage rate for the channel, including the excavation, drainage culverts, irrigation syphons and junctions with the main canal, of $\frac{7,58,818}{160}=$ Rupees $4,742 \cdot 6$. The length of the Cawnpore Branch to the top of the first descent at the terminal lockage is 169 miles, and the total fall in this distance is 190.26 feet, of which $\left(\frac{169 \times 5280}{12000}=\right) 74.36$ feet would be due to the declivity of bed, leaving $115 \cdot 90$ to be disposed of by locks, say, 19 locks of $6 \cdot 1$ descent each.

From the detailed statement it appears that crossings are required at intervals of 2 miles. The total number of crossings required will therefore be $\left(\frac{169}{2}=\right) 84$; of these 19 will be supplied at the locks; for the remaining, 65 bridges must be constructed, the aggregate of which will amount to $\left(\frac{65}{59}\right.$ $\times 3,65,865=$ )Rupees $4,03,070$. The total cost will then be as follows:-

| miles excavation, \&c., at R per mile ... | .. Rs. | 8,01,060 |
| :---: | :---: | :---: |
| 19 Locks, at Rupees 25,200 | ... | 4,78,800 |
| 65 Bridges | ... " | 4,03,070 |
| 19 Chokies, at Rupees 364 each | ... " | 6,916 |
|  | Rs. | $\overline{16,89,846}$ |
| Contingencies @ 8 per cent. | ... " | 1,35,187 |
|  |  | 8,25,033 |
| Establishment @ 7 per cent. | ... " | 1,27,752 |

Grand Total, Rs. $19,52,785$
The estimated cost of the alterations required for navigation on the existing irrigation line being

Rs. $4,26,843$

The above estimates shew only the probable expense of the construction of the separate channels; if to these be added the compensation for land occupied, the total probable outlay will be as follows:-

From Roorkee to Nanoon.

| Construction | $\ldots$ | Rs. | $25,39,131$ |
| :--- | ---: | ---: | ---: |
| Compensation for land, 4,383 <br> @ Rs. 24 per acre | $\ldots$ | , | $1,05,192$ |
|  | acres |  |  |
|  | Grand Total, | Rs. | $26,44,323$ |

## Nanoon to Cawnpoor.

Construction ... Rs. 19,52,785
Compensation for land, 4,507 acres @
Rs. 24 per acre ... " 1,08,168
Grand Total, Rs. $\quad 20,60,953$
108. Considering then, the large additional outlay in first Adaptation of irrigating contruction and expense of future maintenance, and above all, channels for navigation the wastage of water involved in the projection of separate chan- proferable.
nels, the balance appears to me to be largely in favour of adapting all the larger irrigation channels for navigation. If it be found advisable hereafter, short channels from the trunk lines can, without difficulty, be carried into the chief trading towns, the escape water from which would pass off into some of the numerous watercourses, and thus be utilized for irrigation; and at the extremities of the canals, if the navigation be found to be obstructed at certain seasons, by a decrease in the supply of water, owing to the requirements of the irrigation, separate channels can be led off above the points where the obstructions are felt, rejoining the present lines at the junction lockage into the rivers as on the Cawnpore and Etawah Branches. The designs and estimates now submitted for separate channels, it must be recollected, are on a minimum scale; the main line and branches of the Ganges Canal for 9-10ths of their length, would give a very much wider and deeper waterway.
109. With the water communication thus completed Connection, with navialong the centre of the Doab to the Ganges ot Cawnpore, it will gable system, on West be quite possible to connect the Districts lying north of Delhie bank of Jumna, feasible. between the Sutlej and Jumna rivers. In a report on a canal projected from the Sutlej, lately submitted to Government, it was shewn that a navigable connection between one of its main channels and the Western Jumna Canal, below Kurnaul, was feasible. From this point, if existing obstructions be removed, the traffic may pass down the Delhie branch of the latter to within a few miles of the city of Delhi, whence a separate channel, provided with suitable lockage, would carry it into the Jumna, which may, I believe, (for there are no levels over the actual course) be connected at no great expense by a navigable channel with the main line of the Ganges Canal.

This would give an uninterrupted water communication from all the northern districts lying between the Ganges and the town of Sirhind on the left bank of the Sutlej, to the seaboard at Calcutta. To the west of Sirhind, the export traffic would be conveyed by the canals there into the Sutlej; thence down the Indus to Kurrachee.

Navigation returns from Ganges Canal.
110. The nett profits from navigation on the canal during the last few years were as follow :-

| 1859-60 | ... | ... | Rs. | 47,220 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1860-61 | ... | ... | " | 1,12,757 | $\left\{\begin{array}{c} \text { Including } \\ \text { some prots } \\ \text { of provious } \\ \text { yearas. } \end{array}\right.$ |
| 1861-62 | $\ldots$ | ... | " | 55,725 |  |
| 1862-63 | ... | ... | " | 46,834 |  |

Were the existing obstructions to navigation removed, and the branch lines also made navigable throughout, there is little doubt that the returns would be vastly increased, probably seven or eight times greater than the largest amount litherto realized.

Total probable cost of Ganges Canal.
111. The detailed estimates now submitted, I have elsewhere stated, refer only to the main line and the Cawnpore branch. With their aid however, and the statement of actual expenditure in Appendix G., we may arrive at an approximation to the total outlay required for the completion of the entire system of works. The total expenditure on all accounts up to 31st July 1854, before irrigation had commenced, was Rupees $1,41,83,146$. If from this the cost of "Contingencies" and of "Establishment" be deducted, the percentage on the remainder for the former item is 0.7 for establishment; 9.9 on the total of the contingencies and outlay on works. Applying these percentages to the cost of "Original Works," and "Rajbuhas" during the second period, from 1st August 1862 to 30th April 1863, we have-

| Statement ... ... ... ... ... ... |  |
| :---: | :---: |
| ontingencies at |  |
| Total | 56,25,629 |
| Establishment at 9.9 per cent... ... ... |  |
| Total cost since commencement of irrigat | 61,82,566 |
| Previous expenditure |  |
| and Total up to 30th April 1863 | 2 |
| Of this the expenditure on the Cawnpore Branch was |  |
| nd deducting cost of the terminal wo in Cawnpore.. |  |
| The cost of the channel above was. | 16,2 |

The estimated cost of remodelling the Branch amounts to nearly one-fourth of this; for the Etawah Branch it may be taken at one-fifth.


The length of the Etawah Branch completed was somewhat in excess of 169 miles, about the same as the Cawnpore Branch, the discharge at the heads of both being identical. The difference in cost is accounted for by the reduced dimensions of section in the lower half of the Etawah Branch. For arriving at the cost of the other branches, which will only be secondary navigable lines, the lower estimate will, I think, suffice. From the estimates now submitted, one-tenth additional appears to me about the amount required to make these channels navigable when there is no lockage.

Assuming the cost to be proportionate to the several discharges at the head, we have for the Futtegurh Branch-

Cost of channel without overfalls ...

| $\frac{12,09,068 \times 1240}{1640}=$ |  |  | Rs | 9,14,173 |
| :---: | :---: | :---: | :---: | :---: |
| 1-10th for navigation |  | ... | " | 91,417 |
| 4 falls already built | ... | ... | " | 1,19,560 |
| 6 do. at Ks. 25,000 each | ... | ... | " | 1,50,000 |
| 4 locks at, 20,000 | $\ldots$ | ... | " | 80,000 |
| 6 " $\quad 15,000$ | ... | ... | " | 90,000 |
| Contingencies and Establishment as |  |  |  |  |
|  |  |  |  |  |
| fore... ... | ... | ... | " | 1,54,187 |
|  | Total | ... | " | 15,99,337 | Deduct.

Expended up to date ... Rs. 6,85,006
Contingencies and Establishment ", 73,085
7,58,091
Required to complete ... Rs. 8,41,246
Bolundshumur Brance.
Cost of works due to irrigation as before

| $\frac{12,09,068 \times 520}{1,640}$ | $=$ | Rs. | $3,83,363$ |
| :---: | :---: | :---: | ---: |
| Contingencies and Establishment |  | $"$ | $\frac{40,902}{}$ |
| Add 1-10th for navigation | $\ldots$ | $\ldots$ | $"$ |
|  | $4,24,265$ |  |  |
|  | Total, Rupees. | $\frac{42,426}{4,66,691}$ |  |

Deduct.
Expended up to date
Rs. 81,854
Contingencies and Establishment
$\begin{array}{ll}" & 8,733 \\ \text { Rs. } & 90,587 \\ 3,76,104\end{array}$

> Required to complete

## For the Koel Brance.

Estimate as for Bolundshuhur Branch Rs. 4,66,691
The aggregate length of the Rajbuhas completed up to 30th April 1863 was 2,266 miles; the total expenditure on this head being Rupees $31,81,259$, the rate per mile is found to
be Rupees 1,404 , which, by the addition of the proportions as above for contingencies and establishment, is increased to Rupees 1,553-8. The lengths of irrigating channels are as follow:-

| On the Main Line | .... | 153 Miles. |
| :---: | :---: | :---: |
| Cawnpore Branch | ...... | 168 , |
| Etawah do. | - | 165 |
| Futtehghur do. | ...... | 145 " |
| Bolundshuhur do. | ..... | 55 |
| Koel do. | ...... | 55 |
| Aggregated |  | 741 Miles. |

The length of Rajbuha channel to each mile of canal on the main line and Cawnpore Branches, where the Rajbuha system may be considered complete, or very nearly so, is about 5 miles ; a like proportion is found to obtain on the Eastern Jumna Canal. This gives an aggregate length of Rajbuha channels for the whole of the irrigation from the Ganges Canals of 3,705 miles ; and deducting 2,266 miles, we find 1,439 miles as the length required to complete the system. The cost at the above rate would then be $1,439 \times 1,550=$ Rupees $22,30,450$. Collecting the several items together, we obtain the following as the total probable outlay on the works of the Ganges Canal and its branches-

| Expended up to 30th April 1863 | ... | Rs. | 2,03,65,714 |
| :---: | :---: | :---: | :---: |
| Remodelling as per estimate |  | " | 39,19,850 |
| Completing Etawah Branch |  | " | 4,47,613 |
| Do. Futtehgurh do. |  |  | 8,41,246 |
| Do. Bolundshuhur do. |  |  | 3,76,104 |
| Constructing Koel do. |  | " | 4,66,691 |
| Completing Rajbuha system |  | " | 22,30,450 |
| Permanent head-works on the roughly estimated at | Ganges, | " | 10,00,000 |
|  | Total |  | 2,96,47,668 |

Revenue lost and compensation given if
canal be closed for one year ... Rs. 13,48,213
Grand Total, Rs. 3,09,95,881
Taking the full supply of water delivered at the head of irrigation to be 6,750 cubic feet per second, the cost per cubic foot per second of discharge is thus, Rupees 4,740.
Discharge of Canal as- 112. The supply of water for irrigation and its distribusumed as in original pro- tion have been assumed throughout these projects according to the original projection, as it was based on results obtained from the older canals in Northern India, where irrigation had been long and extensively developed. Though, as I have stated elsewhere, the volume in the Ganges Canal is now much reduced by absorption, there is no reason to suppose that when the surrounding water-bearing strata are fully saturated, the wastage will be greater than in other localities.
Terminus of Futteghur 113. It was proposed, I understand, some time since to Branch. shuhur, at the 82 nd mile from the head, substituting for the remainder of the projected line two Rajbuhas which would be
taken off at that point. If this be determined on, a navigable communication can be made with the trunk line above Nanoon along the line of levels shewn in Plan II., a section of which will be found in Plan IV. The cost of such a channel would, I believe, be fully covered by the estimate for completing the branch as projected.

## SUPPLY OF WATER FROM THE JUMNA FOR AUTUMN CROPS.

114. The information I have been able to collect on this head is rather of a negative than a positive character. The most economical plan at first sight appeared to be, to take off the supply from the Bhoodhee Jumna above the Nyashuhur Dam, thence carrying it in the present channel of the Eastern Jumna Canal as far as the dam over the Muskarra at Kulseea. from this a new channel would reach the watershed between the Dumola and Hindun Rivers in a length of a few miles, and, passing to the southward, could be carried across the Hindun by an acqueduct, meeting the proposed alternative line above the town of Deobund. The levels Plan IX. show that it is physically feasible, but the outlay must, I think, be greater than the results would justify.

In consequence of the difficult nature of the country, and the small waterways of the Belka and Behut overfalls, on the Eatern Jumna Canal, it would be neeessary to construct an entirely new channel from above the Nowgong torrent, which would cross it below the present Dam and rejoin the Canal Channel just abjve the Muskurra Dam; the overfall or (retaining dam) on the Nowgong, built some years since to stop the retrogression of the bed level, might be utilized for the crosigns of the new canal. Below the Muskurra, however, the declivity of the country is very rapid; numerous overfalls would be required besides an aqueduct over the Hindun, which would involve a large expenditure of time and money.

The irrigation from the Eastern Jumna Canal supplies the country fairly on the right bank of the Hindun; the new channel would, therefore, not commence irrigation above the 30th mile or so from its head. Perhaps the objection of most weight against drawing water from this point, or any where above the present Canal heads, is that it might interfere with the wants of the existing irrigation at the most critical season of the year. To bring the autumn crop to perfection, water would be required from May to September inclusive.

What the increase in the volume of the Jumna may be in the former month, I cannot exactly say, no discharges having been yet measured; but from the experience of the autumn of 1863 , when the rains ceased on the 10th of September, and the hot westerly wind parched the lower districts for some weeks subsequently, it is evident that in certain seasons the whole visible supply is needed for the present canals. Though the river bed was then dry below the heads of the Jumna Canals, the cry for more water was incessant.
115. The second locality from which levels were taken From Reilmay crossing was the Railway crossing opposite Saharunpore, where a opposits Saharunpore. dam and head-works might be economically combined with the
bridge shortly to be erected. The levels however are unfavorable, and the channel would have to be carried across the Eastern Jumna Canal, and the numerous water-courses and drainages, which intersect the district, besides requiring an aqueduct for a passage across the Hindun. The volume of the Jumna also, at so short a distance from the present canal heads, would in all probability be very small at the latter end of the rainy season in a dry year.

From river at confluence of Hindun.
116. The highest point, in my opinion, from which a channel could be taken at a remunerative cost, and with a fair prospect of an unfailing supply throughout the irrigating season of the autumn crops, is at or close to the junction of the Hindun with the Jumna, 16 miles below Delhi. Opposite to this the main watershed of the Doab abuts on the valley of the river, and from a cross section taken from the Bolundshuhur Branch head to the Hindun, close to its junction with the river, it appears that the surface of the stream in the latter, in the dry season, is on the same level as the bed of the main line Ganges Canal at Moonda Khera. A channel threfore led off from this point at a declivity, say, of half a foot per mile (or 1 in 10,560 ) would connect with the Ganges Canal at about the 160th mile from its head. The length of non-irrigating channel would thus be about 48 miles, and the greatest depth of excavation would not, judging from cross section levels taken over the high land, exceed 32 feet. A dam across the river here could be constructed at a minimum outlay from its proximity to the Delhi quarries, whence stone could be floated down at a small expense in boats.

It would be perhaps advisable to keep this channel quite distinct from the existing Ganges Canal lines reserving all irrigation from it for the districts to the southward, where water can be but scantily supplied at present.

A collateral and very important advantage connected with such a channel is the opportunity it affords of continuing the navigable communication down the Doab, to the junction of the Ganges and Jumna at Allahabad. Though the volume of the Jumna in the dry season at such a distance from the hills cannot be relied on to supply any large extent of irrigation, there will always be sufficient for a navigable channel of considerable size, even with a current running at the velocity assigned to it in this project. In fact, were the river dammed across here, some such provision appears necessary to supply the wants of the traffic which now passes up as high as Delhi for the greater portion of the year. It is superfluous to say more on the matter in this paper, after the unanswerable testimony given to the value of such a communication in the late Colonel Baird Smith's Famine Report.
117. Below the confluence of the Hindun, the valley of the Jumna narrows rapidly; the section at Agra taken for the Railway is given in Plan VIII. A supply might be taken off at a lower point than I have indicated, but I doubt its feasibility at a remuuerative cost; the high bank is close to the edge of the stream, about 40 feet above it at Agra, and the declivity of the ground surface to the southward is small, not exceeding a foot per mile on the average.

## CONCLUDING REMARKS.

118. The question of permanent head works for the canal did not form one of the subjects to which my attention was directed in the present investigation; a few general remarks on it will not, however, be out of place. The site of the present temporary bunds is at a point on the river, where the declivity is great, rapids at short intervals which retrograde more or less every rainy season. The safest position for a permanent weir is obviously at the foot of this excessive slope, where the tendency to deposit on the bed commences; the construction of foundations here also would be a less difficult business than in the heavy boulder strata above. This reduction of declivity appears to occur at a point on the main stream at the bridge of boats about half a mile below the town of Khunkhul; from the levels given in the Ganges Canal Report Atlas, it appears to be just possible that a channel taken off from this point could be connected with the existing line above the Ranipoor superpassage by removing the overfall there, and deepening the bed for a short distance higher up. The cost of this channel and the new head works would not, I think, exceed that of the latter, if constructed at the upper site, added to the outlay on remodelling the line from the present Regulator to below Khunkul Bridge, besides having the advantage of a more effective control over the river above the dam.

The bed of the river at the point indicated is of shingle and small boulders; a few rapids occur lower down, but at long intervals, and the descents at each very gradual.

It has been suggested that the bridge for the Rohilcund Railway might be carried across the dam here, thus saving the expense of foundations.
119. One question-affecting not only the Ganges Canal System of general drainworks, but also the whole system of irrigation in Northern age required. India, which is yearly becoming of greater importance--is the necessity for thorough drainage in all the districts under the influence of constant irrigation. The general elevation of the spring level in the adjoining tracts was long since observed to follow the introduction of canal irrigation, but it was not till within the last few years, that the evils resulting from this over saturation of the soil, became so clearly apparent.

Swamps are formed over extended areas of low ground, which are further increased by the excessive proportion of the rain-fall, which must now find its way into such hollows, wherever the country drainage outfalls are not sufficient to carry it off, owing to the diminished capacity of the soil for absorption. The condition of the districts bordering the Delhi Branch and other portions of the Western Jumna Canal from this cause has long since attracted the attention of Government. I need not go over the details here; much has been already done in the way of draining these marshes on both the Jumna Canals. A general system of drainage however is required, embracing not only the tracts now constantly flooded, but also all districts where the spring level is so close to the surface as to injure the quality of the crops or generate malaria. When in charge of the Eastern Jumna Canal, for a few months last year, where drainage works have been largely carried out by the able and indefatigable Superintendent, Major Brownlow, R. E., (now
on furlough in England) my attention was directed to several extensive marshes which I was informed had only made their appearance a year or two previously; these were due mainly, I believe, to the general elevation of the spring level. It was also evident that some of the drainage works excavated in past years, had become insufficient for their purpose, owing to the increased proportion of rainfall to be carried off in the surface outfalls.

Nor is this to be attributed, or only very partially, to leakage from the canal channels. In an investigation which was carried out last year by the orders of the Punjaub Govennment into the causes of the diminution of water-supply in the cantonments and city of Umballah, the facts elicited clearly showed that the subsidence of the surface level of the water in the wells was due almost entirely to the shutting off of the floods from certain hill torrents which formerly inundated the country very extensively every rainy season. It is constant flooding over large areas which alone can cause a general elevation of the spring level.

This is only another instance of what may be called a truism, though it requires often to be brought to notice in this age of rapid progress (vide Sir John Rennie's letters to the "Times" in December 1863, on "Drainage, Sewage, Water-supply, and Irrigation") that where the course of nature is interfered with by artifioial operations, the interference does not stop at the point originally intended; other results supervene, which, if injurious, must also be dealt with artificially.

A general system of subsoil drainage, such as indicated above, would increase rather than diminish the extent of irrigation, as the water thus carried off might be utilized in lands at a lower level.

The "reh" efflorescence, so destructive to all vegetation, is undoubtedly due, in most cases, to excessive moisture at or close to the surface.
Remedial measures why 120. The question may be asked why were remedial not adopted earlier. measures not earlier adopted to remove the acknowledged defects in the Canal. In the beginning of 1857, by the permission of the Punjab Government, I traversed the Canal by boat from Hurdwar to Cawnpore, for the express purpose of seeing how the works had stood, with the view of deriving information which might be useful in the construction of the Baree Doah Canal, then in course of construction. All the evils now so largely developed were then incipient; but it was not oonsidered expedient to enter on any extensive remodelling scheme until the works had had a longer trial. The smothered murmurings of disaffection were then rife, and before two months had passed, the Mutiny with all its horrors was upon us; its echoes had scarce died out ere another, and scarcely less terrible visitation, the famine of $1859-60$ and $1860-61$, averspread the greater portion of the North West Provinces.

Human lives were then in the balance, and the question no longer was as to the volume which the canal could safely carry, or the best method of rectifying defects, but how the maximum supply of water could at any risk be conveyed to the starving population in the lower districts.

Various suggestions have been made and acted on before and since that time, but as the measures hitherto adopted were only directed to the protection of existing works, and did not touch the root of the evils, they have been only partially successful. Engineers were loath to recommend alterations involving a large outlay, until all other minor expedients had been well tried.
121. It has often been put forward as an objection to the Reply to objections that large irrigation works in these provinces, that they "do not Canals are unremunerapay."

Now the Western Jumna Canal has long since returned into the Government Treasury the whole outlay on its construction, and is now yielding a very large profit. The Eastern Jumna Canal has likewise cleared its own expenses, and for the last two years, the balance of revenue over current charges has been clear gain. In the case of the Ganges Canal, it must be borne in mind that the development of the irrigation has been retarded by several causes; the unsettled state of the country and financial pressure for a considerable period since its opening ; the defects in its construction, unwillingness of cultivators in certain localities to take water at first (the same objections to any innovation were experienced in the earlier stages of the Jumna Canal irrigation) to which is to be added the change in the first few years after the opening of the canal from the old system of assessing the water-rent by area irrigated, to that of charging by volume discharged; not that the latter is not the very best, if practicable, but it is a novelty, and that, as such, it should be looked on with disfavour will not be surprising to any one acquainted with the natives of this country.

Canal irrigation in a country previously without it can never be very rapidly developed, nor is it at all desirable that it should be; independently of the necessity for the consolidation of the works in the channels before a large supply of water can be allowed to pass over them, the change in climate from extreme drought to constant humidity, which necessarily follows the introduction of irrigation extensively, must have an injurious effect on the health of the inhabitants generally.

Their whole social economy also must undergo alteration, and that such changes should be gradual, no one, I think, will deny. The crops raised by the present canal irrigation are, in general, notoriously inferior in quality to those watered from wells ; and one chief reason for their inferiority undoubtedly is, less care in the cultivation as well as deficiency in manure; the greed of gain induces the Zemindars to extend the cultivation further than they have labor to work properly, and until the number of cattle increase, which can only be by degrees, the land is impoverished from constant cultivation without a due allowance of manure.

The credits to these canals in the public accounts, however, shew but a fraction of the real return. Besides the amount realized as water-rent, there is the large increase to the land tax in the irrigated districts which has never yet been allowed to be shewn as profit due to the canals in the balance sheet. Now what are the water-rates? Until within the last two years they were the rates determined by Colonel J. Colvin, Superintendent of Canals in the North West Provinces, and sanctioned
by Government in 1827, on which he observes, in his Report of 1831 , on the canals generally, "the often-repeated declaration of the Government to the Superintendents, as their main rule of guidance, that the object of Government in collecting a rent through them was not so much to form a productive source of revenue from the actual price paid for the water as to give them an efficient control over its expenditure, by making it of value to prevent its being wantonly wasted, and that they looked alone to the general improvement of the country as the source from which they should derive the return adequate to the outlay. This announcement completely prevented the Superintendent disposing of the water to the best advantage, and led to the settlement of a fixed rate of assessment, so low, that it is not sufficient to prevent carelessness, entailing much waste of water."

Now if the rates were not sufficiently high to shew a remunerative return on the outlay on the comparatively petty canal works executed up to that time, how can a profit be shewn on irrigating canals of the present day, which the experience of years has proved must, for permanent efficiency, be constructed much more expensively. Were the iucrease to the land revenue due to the irrigation from its commencement up to the present date added to the returns from "water-rent," the balance sheet would shew very differently. In the case of the Ganges Canal, unfinished and defective as it is, if we take the increase to the land tax at the lowest computation as equal to the present total of water-rent, which for the past official year 1863-64 was Rupees $6,93,134$, we find as follows :-

| $2 \times 6,93,134$ | Rs. 13,86,268 |  |
| :---: | :---: | :---: |
| Add miscellaneous receipts | 80,256 |  |
| Deduct Current expenses... | , | $\begin{array}{r} 1466,524 \\ 7,00,000 \end{array}$ |
| The | ett balance is, Rs. | 7,66,524 |

The total expenditure on the works up to the 30th April 1863, being, as calculated in para. 111, Rupees $2,03,65,712$, say Rupees $210,00,000$ up to the end of the last official year; this gives a return of 3.65 per cent on the outlay up to date.

The value to the country of these irrigation works is acknowledged on all hands; if it be deemed necessary also to shew their profits as commercial speculations, it will only be necessary to give the aggregate income derived from them, whether it be collected in the shape of land or water taxes.
Insofficieney of Estab- 122. This Report would be incomplete were I not to draw lismment of European Ufi- attention to a subject of vital importance to the success of the
eers. works both physically and financially. It may be gathered from the above pages, that information on many important points is much needed to admit of a satisfactory decision being arrived at on certain questions of constructive detail.

For investigations of this nature the strength of the present establishment of European Officers is utterly inadequate; hampered as they are, especially the younger officers, with accounts and the preparation of returns, they can scarcely overtake the ordinary current work. This is no exaggeration. I rrite of what I in common with all acquainted with the working detail of the Department know to be the case. Some men there are, of iron frame and large intellectual calibre,
whom no amount of toil, physical or mental, seems to affect; such are, it need hardly be said, rare exceptions, and their capabilities can form no basis for a calculation of the working powers of ordinary humanity. Though a portion of the account work has of late years been transferred to Central Offices, the relief to Executive Officers on this score has been little, if any; the office establishments have been so much reduced, that a very large proportion of their and their Assistants time is still taken up with merely mechanical paper work. In these Hydraulic works it must be borne in mind that much closer supervision of every detail is necessary than elsewhere. In a Railway or line of road, errors or defects in construction have only a local influence; here the results may extend themselves almost indefinitely and in a very short space of time.

Every petty distribution channel may be lined out and levelled by a Native subordinate, but numerous proofs are not wanting that the work cannot be relied on without verification by a European Officer. In addition, he has the proper distribution of the irrigation and the revenue details to look after; no trifling matter where irrigation has been fully developed.

It may be deemed travelling out of my province thus to criticise arrangements made by superior authority, but the interests at stake are large, and this Report has to deal, I conceive, with every fact affecting the efficiency of the works, without which the most elaborate system of accounts and best fiscal administration, necessary though they also may be, will be of little value.

While these projects were in course of preparation, another of our number, the late Colonel A. D. Turnbull, one whose loss will be long and deeply felt by all acquainted with him, (Natives as well as Europeans) has passed away, struck down in the prime of life and usefulness by jungle fever, the scourge which has so often ere this carried off our worthiest and best; in his case the deadly influence of malaria was without a doubt intensified by anxiety regarding his work and over exertion.
123. A memorandum by Major General Sir Arthur Cotton, Sir Arthur Cotton's Memo. which accompanied a communication dated July 1863, from the Secretary of the East India Irrigation Company, to the Government of India, claims a brief notice here as it contains strictures on the whole projection of the Ganges Canal works, which, if correct, would modify the measures now recommended for their rectification in several important points.

He states that " there are the greatest fundamental mistakes in its projection," which he classes under five heads:-

1 st.-The head of the canal is placed too high up, above a tract which has a very great and inconvenient fall, and in which there is a very heavy drainage from the sub-Himalayas across which the canal has to be carried.
$2 n d$. -The whole canal has been cut so as to carry the water below the level of the surface, entailing a vast unnecessary excavation, and keeping the water below the level at which it is required for irrigation.
$3 r d$.-The whole of the masonry works are of brick, while the most suitable stone for hydraulic works is procurable in the Sub-Himalayas.

4th.-The whole of the water is admitted at the head so that some of it is conveyed three hundred and fifty miles to the land it irrigates, while it might have been obtained at a sufficient level at a distance of say 50 or 100 miles.

5th.-There is no permanent dam across the river at the head of the canal, so as to secure the supply of water, but temporary works are thrown up after every monsoon, which are liable to be swept away at the very time when they are most wanted.

Permanent head works.
124. The last cannot be called a "mistake" in the projection, as the work has only been held in abeyance until the necessity for it was fully proved, nor would the addition undo any part of the project already executed. The ultimate necessity for some such work has for years past been a subject much discussed by the Canal Officers; amongst others, the late Colonel R. Baird Smith, in his Famine Report of 1860-61, drew the attention of Government to it as a matter of serious importance, but it was not until every drop of water in the river had become of value to the irrigation, that that necessity could be satisfactorily proved. After thirty years' experience of the efficiency of the cheap temporary bunds across the Jumna in supplying the irrigation on the $\dot{W}$ estern and Eastern Jumna canals, the projector would not, I maintain, have been justified in recommending this costly addition to the Ganges canal works without full proof of its absolute necessity. The obstruction of the entire volume of the Ganges for irrigation was required much earlier in the existence of the canal than in the case of the Jumna; in the famine year the total discharge of the former at the Canal heads was at one time so low as 5,600 cubic feet per second, the ordinary minimum when the river is at its lowest being upwards of 7,000 cubic feet. For some time past the necessity for permanent head works has been acknowledged by all, and designs for them are already before the Government.
125. As to the second point, the excavation of the canal so as to retain the water below the level of the ground surface, it was simply a sanitary precaution, the wisdom of which no one would doubt who had seen the effects of a contrary plan on the older canals in these Provinces. Independent of the risk from certain soils, such as those impregnated with "reh" and other salts, through which water will percolate, do what we may, the ravages of vermin have ere this caused sufficient injury to the channels and adjoining lands elsewhere to make the embankment system, on the score of economy, a very questionable mode of construction. In the sandy tracts moreover it would not effect the desired object of preventing percolation, the soil in no portion of it being absolutely watertight ; the only difference is that the saturation takes place in coarse sand in a shorter time ; and I have shown above, para. 119, that saturation of the soil is sooner or later a certain consequence of extensive irrigation. The statement that the water is thereby kept below the level required for irrigation, does not represent the state of matters correctly. It will be seen from the longitudinal section that the level of full supply in the canal for some distance above the overfalls, stands higher than the surface of the ground, and it is from these points only that irrigation cuts are led off between Roorkee and the 110th
mile : below this, the full supply level stands almost everywhere (in some places as much as several feet) above the ground line.

Drainage cuts would doubtless carry off leakage water, but could only very partially avert the evils of inundation from breaches in embankments.
126. The third objection that brick was used in the Substitation of stono masonry instead of stone is simply a question of comparative for brick.
expense. The overfalls on the Eastern Jumna Canal by a trial of several years had shewn what violent action good brickwork would stand, and the projector did not consider himself justified in substituting stone in the Ganges Canal works at an enormous additional expense merely to ensure a greater degree of safety. Sir Arthur Cotton himself allows that the " absolute impossibility of obtaining it (stone) within a practicable expense, would justify an Engineer in building weirs and sluices without it."

He further states that the brick masonry is " of the very best quality, both materials and workmanship as fair as any" he ever saw. Now I have shewn in paras. 20 and 21, that the materials and workmanship at the overfalls below Roorkee which have been seriously injured are very indifferent indeed, and the only way of accounting for Sir Arthur Cotton's assertion is that he never, as I am informed, saw these works, having only inspected minutely the line immediately at and above Roorkee, where, as I have shown above, the worst damage sustained by the works is the stripping of portions of the brick on edge covering from the ogee curves, and the floorings immediately below, at the overfalls.

Boulders were used extensively in the construction of the more massive parts of the works above Roorkee wherever they could be transported at a reasonable cost, but they were covered wherever fine work was required with brick facing, as building with hammer-dressed boulders is quite out of the question on account of the excessive expense.

The "Hurdwar Stone," I have shewn in para. 90, is only procurable in small quantity and at great cost.

The researches even up to the present date* only confirm the soundness of Sir P. T. Cautley's views, that the stone of the Sewalliks generally is unfit for building with; strata of good materials are to be found, but at wide intervals, and the quantity in any one locality is very insignificant.

It would be less expensive to construct overfalls of the form I have detailed elsewhere which we know can stand with even brick, or a very small amount of stone facing, than to adopt works of Sir Arthur Cotton's simpler design covered throughout with stone.

It is further recommended in the Memo. that the locks should be "lined with stones most carefully fitted." Brick has answered its purpose hitherto ; I see no good reason for the additional expense at present. A statement is made that "monstrous fines" are inflicted for any injury done by boats to the plastering. I have failed in discovering that fines were imposed on this account anywhere except at Cawnpore; there is no doubt that plaster is useless under such circumstances,

[^1]and the infliction of fines for injuries to it from boats or rafts is, to say the least, an oppressive exercise of authority.

Position of head of Ca nal.

NOTE TO PARA. 127. Since this para. was written, a review of Sir A. Cotton's and other pamphlets bearing on this discus. sion has appeared in the colamns of the "Times" Newspaper of November 2nd, in which the writer states his opinion that when the difference of level between the bed of the river and the top of the adjoining bank was so great, "an Engineer, who had to take a channel down to some point in the valley beyond it, would not be likely to turn into the bank at guch an angle as to involve "deep digging for many miles, unless he went mad on purpose; he would probably take it in flank for some distance with no more excavation than was just sufficient to make his embankment on each side, perhaps sometimes not above a yard deep, until he found a favourable opportunity for getting through with a short cutting of 20 or 25 feet, after which he would reach a flat country with only one trifling stream to cross." Sir A. Cotton (in a pamphlet which has only lately come to hand) suggests the possibility of utilizing an old branch of the Ganges shown in the map at the foot of the high bank between Sookertal and Gurmukhtesur for a portion at least of the proposed line, but it will be evident from the sections, that so shallow a depth of excavation at the entry into the high land as 20 to 25 feet could not be obtained without carrying the channel for a long distance at a considerable elevation above that old bed or the surface of the adjacent low land, keeping close to the face of the high bank, the slope of which all along is excessively steep (in places almost perpendicular) and cut into by innumerable deep ravines which carry the drainage from the table land above into the valley of the Ganges. The belt of land from which these are the ont-falls varies in width from 3 to $4 \frac{1}{2}$ miles, and the drainage thus brought down must either be discharged into the canal, or passed under or over it by masonry works.

Of the evils conseguent on the adoption of the former method of disposing of the drainage, we have proof in the case of the canal on the western bank of the Jumna above Kurnaul, where the old native channel taken up and improved by British
127. The 1st and 4th objections may be best considered together, as they involve one, if not the main point at issue. Sir Arthur Cotton is of opinion that the head of the canal is placed too high up on the river; now this is also a question chiefly of cost. No one doubts the practicability of building a weir across the river anywhere and leading off a channel thence for irrigation, but the cost of this will vary much with the circumstances of the locality. In Plan VIII., Sheets 1 and 2, are given sections of the Ganges at the points indicated in the General Map (Plan No. 1). If we take the section at Sookertal, below the confluence of the Solanee, the point suggested by Sir A. Cotton as the best site for the upper head, we find that the top of the high bank forming the boundary of the valley is 95 feet above the surface of the dry weather stream in the river, or 101 feet above the bed. Sir A. Cotton proposes to construct a weir to raise the surface of the water to a level 15 feet above the bed, and to carry the canal channel thence at a declivity of 1-4th foot per mile, or 1 in 21,120 . With this slope the bed would meet that of the present main line of the Ganges canal about Moradnuggur Bridge (the 100th mile from the head), and this distance on the most direct course is not less than 57 miles. If we suppose the bed of the new channel to commence at a level 3 feet above the present bed of the river at Sookertal, the depth of water at its entrance will be 12 feet. With this depth of water and declivity a channel 260 feet wide at bottom, side slopes 1 in 2, would discharge about 7,000 cubic feet per second, at a mean velocity slightly exceeding 2 feet per second. The maximum depth of excavation, supposing the edge of the high land to be reached at a distance of 2 miles from the weir, is 96 feet, the minimum at Moradnuggur bridge, 10 feet, giving a mean of 53 feet. The side slopes of this cutting, where nothing but pure sand will be met with below the upper 10 or 12 feet, cannot be made less than 1 in 2 ; the berms 15 feet wide on either side at a level of 14 feet above the bed. This gives a mean area of 26,186 superficial feet, or contents of excavation on 1 mile $=138,262,080$ cubic feet ; say that the rate of excavation for the first 27 miles of deep cutting is 4 Rupees per 1,000 cubic feet, for the last 28 , Rupees 3 ; (the work could not possibly be done for less) and we have an aggregate of Rupees $265,46,304$ for earthwork alone, without a single masonry work of any description! The total cost of the first 23 miles of the present Gauges Canal from the head to the Hafiznuggur falls ( 4 miles below Roorkee) including all the heavy works in the Ganges valley was Rupees 81,17,114; even as far as the 100th mile the aggregate was under a crore of Rupees. Sir A. Cotton has compared the cost of a channel from Sookertal with that of the entire existing line down to their junction ; this is obviously unfair ; the comparison should have been with the non-irrigating length below the present head, which terminates at Hafiznuggur Falls, with the addition only of the extra outlay for overfalls on the remainder of the distance.

At Bysoomah, 20 miles to the south of Sookertal, the difference of level between the dry weather surface of the

Ganges and the top of the adjoining high bank is reduced to 77.8 feet, and at Gurmuktesur, 30 miles further down, it is 56 feet.

The latter must have been one of the points alluded to in Sir Arthur's Memo. as being 40 feet below the bed of the canal, the bed of the Futtehgurh branch opposite being at very nearly that elevation above the river surface (vide Longitudinal Section 45th mile). A channel led off here would not connect with the main line of the Ganges Canal on the watershed under a length of 40 miles, besides having to cross all the drainage brought down by the East Kalee Nuddee and its feeders; a sketch of an existing bridge over this drainage at Bolundshuhur is given in Plan VI.

Now Gurmuktesur is about 80 miles below the debouche of the river into the plains at Hurdwar, which is stated in the Memo. to be so far an advantage that a larger supply would be obtainable, water draining out of the sands below the shingle tracts besides additions from the small affluents above ; and this is apparently confirmed by the statement of discharges, given in the Ganges Canal Report (Vol 1, page 24) observed simultaneously at Hurdwar and Gurmuktesur, the excess at the latter point being accounted for there by accessions from the drainage of the Khadir and the different smaller lines both on the right and left banks of the river. Such affluents however, it is obvious, cannot be relied on as a permanent source of supply, for in a dry season they would yield little or nothing; and facts are opposed to the theory of the volume being increased by water draining from the sands of the river below the shingle tract. Observations were taken to ascertain this in the Ravee some years since at the instance of Captain J. H. Dyas, then Superintendent of Irrigation in the Punjaub, the results of which were that the volume of water in the river at the season of minimum discharge was found to decrease steadily, as the distance from the foot of the hills, where the bed is in shingle, increased, and at a point 60 miles above Mooltan, or about 280 miles in a direct line from the hills, the reduction amounted to half the discharge measured at the highest point.

The affluents along this portion of the river are too insignificant, except during the rainy season, to affect the discharges materially. Now, as all the rivers from the Ganges to the Chenab traverse the plains under precisely analogous circumstances of soil, \&c., there is, I think, good reason for supposing that their discharges, excluding additions from affluents, are similarly affected. By abstracting then the whole visible supply in the shingle tracts, and if practicable, taking off channels from points lower down in the sandy beds, we obtain the largest supply possible from these rivers.

From all the information I have been able to gather, the maximum volume, when the rivers are at the lowest, will, I believe, be found at the junction of the shingle and sandy beds.

One advantage of no mean importance connected with the heads from the shingle beds is the fact of the water in the river there flowing for ${ }_{3}$ rds of the year clear as crystal, while in the sandy tracts lower down, silt is always more or less held in suspension. It must also be borne in mind that the navigable communication from the foot of the hills would not be complete if the Canal head be placed lower down. For the
officers some 40 years ago, is carried in this way for 45 miles along the foot of the high bank of the valley. Spite of its large capacity, about 2,500 cubic feet per second, and the relief afforded by escapes, it cannot at times carry off the drainage entering on the right bank. I have known it to run full for days when the head sluices were closed. The amount of silt thus deposited in the canal was, as might be supposed, enormous. With this example before them, no one would, I think, be justified in adopting a similar course in a permanent canal elsewhere.
If a channel be taken off as proposed, and the drainage carried under or over it, as the depth of excavation will be comparatively trifling, the width may be greater than if cut directly into the high bank. Assuming the depth of water entering at the head to be 8 feet, a channel 496 feet in mean width, on a declivity of $\&$ foot per mile would discharge 6.800 cubic feet per second, with a mear velocity of a little over 1.7 feet per second, and might enter the high land 30 miles from its head with a depth of excavation of about 23 feet; $6 \frac{1}{2}$ miles below this the water would reach the surface of the country. The outlay on these 36 miles for excavation, culverts, bridges, sce., could not possibly be less than 50 lakhs, and at its entry into the high land the bed would have reached an elevation above the surface of the valley immediately adjoining of not less than 25 feet; this for a permanent supply channel, and with so large a volume of water can. not, in my opinion, be consi dered safe.
If a greater depth of water be admitted into the channel, or the declivity of bed increased, or both, the width of section would of course be less for the same dis charge, but the length of non-irrigating channel would be thus extended.
The course of a supply chaunel in fact must ob viously depend on circum. stances peculiar to the loca. lity in each case, and in this of the Ganges from Sookertal to some distance at least below Gurmukhte. sur, a line along the face of the high bank is, I think, clearly unadvisable.
J. Crofton, Capt.
traffic from the interior of the hills, it would therefore be necessary to " break bulk" twice, first-at the foot of the hills, where the mode of conveyance now changes, and always must do so, and again, into the boats at the head of the Canal.

The above appear to me to be sufficiently conclusive reasons for the determination of the heads of the canals in these Provinces from the shingle tracts. Additional quantities may be drawn from points lower down, but the main supply must be taken as at present.

With regard to deep cuttings through pure sand, I have not touched on the difficulties and consequent expense of working many feet below the level of the surface of the springs, and the consequent desiccation of the lands for perhaps miles on either side; the water would of course drain into the canal, and thus pass off to benefit the lower districts, but at the expense of those higher up. Bridges and aqueducts over excavations such as these would raise the outlay to something very large. A weir if constructed so high up as even Gurmuktesur must still be chiefly of brick; laden boats cannot ascend the river in the shingle tracts, and even if that were possible, or they could be brought within 5 or 6 miles of the works by the present canal channels, the cost of stone thus transported would be enormous. Sir Arthur Cotton apparently has little idea of what the rates for materials and carriage in these Provinces really are, for in one place he estimates the cost of pitching the slopes of the canal with loose stones, carried by boat 200 miles on an average, at Rupees 4.63 per 100 cubic feet, whereas the rate of small boulders delivered 6 miles above Roorkee is now $4 \frac{1}{2}$ Rupees per 100 cubic feet.

In the case of the Jumna a point has been indicated elsewhere where it may be possible to construct a weir and head works at a practicable cost.

Enumeration of alleged " minor" mistakes.
128. Fourteen "minor" mistakes are enumerated as follow :-

1st. All the weirs are made of a length corresponding with the full breadth of the canal, \&c.
$2 n d$. These weirs are placed in the direct line of the canal, \&c, thus compelling the whole of the traffic to go round instead of the irrigation water.
$3 r d$. The whole canal has too great a fall in its bed, \&c.
4th. The canal has been terminated at Cawnpore instead of being carried on to Allahabad, \&c.

5th. The slope of the canal is continued to the end at Cawnpore, so that to keep the navigation open there must be a large body of water constantly flowing to waste into the river.

6th. The bridges are so low as to prevent a fully loaded boat passing under them.

7th. The towing paths are not carried through the arches of the bridges so that the lines have to be thrown off at every bridge, that is, at every three miles.

8th. The lock channels have such sharp curves that boats of the length of the locks cannot pass through them.

9th. No arrangement has been made for the disposal of the silt.

10th. There are no connecting navigation lines between the different branches so that boats can only get across the tract by going all the way up to the point where the branch and main line divide.

11th. The Solanee aqueduct is made of the full breadth of the canal above, and of the full length of the breadth of the river below, \&c.

12th. The breadth of the canal at the lower end is much too small for a large traffic, such as there would be if the navigation were in an effective state.

13th. The slopes of the sides of the canal are much too steep.

14th. There is no communication between the canal and the river at Cawnpore; for although there are double locks the gates of the lower one are not in repair.
129. Regarding the first of these minor defects, Sir Weirs and velocity of Arthur subsequently explains his design for preventing undue current. depression of the water surface at the falls, by contracting the width of the passage over the creek ; this passage being, as I gather from the description, in one or more large bays. In para. 39 I have given my reasons for preferring a different form of overfall.

The second alteration suggested, that the falls should be placed out of the direct line of canal, is unnecessary with the form of fall designed in these projects. It would besides lead to greater expense in construction of fall and locks and protection to the sides of the curved channel below the fall, as well as in clearance of silt hereafter, which would undoubtedly be lodged in large quantities at the head of the locks, unless the gates or sluices were kept constantly open so as to cause a scour.

The third objection, that the declivity of the canal bed is too great, should have been placed at the head of the list of defects. To any unprejudiced mind, acquainted with the real facts of the case, it will be evident, I think, that this has been the main, if not the sole cause of the evils which now impair the efficiency of the canal as an irrigating machine.
130. The ninth objection is, that there is no arrangement for the disposal of the silt. From his subsequent explanations, and design for a large basin near the canal head, which would act as a silt trap, Sir Arthur evidently supposed that the silt in the canal water is mainly brought in from the river ; while the fact is, as I have shewn in para. 36, that by far the largest proportion is due to the erosion from the bed and banks along its course ; were it otherwise, we should find silt deposits to a considerable extent in the upper reaches of the canal where the velocity of the current is much below that of the river in floods.
131. The eleventh objection is that the waterway of the Silt. Solanee aqueduct, both for the passage of the torrent below and the canal above, is much too great. Now it is an instructive fact, that of the four torrents crossed by the canal in the Ganges valley, only one, the Rutmoo, has suffered from dan-
gerous erosive action below the masonry works at the crossing, and in this instance alone the waterway for the passage of floods is contracted. From the diagram given at para. 6 it will be seen that the bed of the torrent below the Rutmoo Dam has been channelled out to an average depth of about 9 feet, the bottom of the hole scooped out some 700 feet from the masonry having been at one time upwards of 15 feet below the level of the dam flooring. In August last year, there was a hole 12 feet in depth below the flooring at a distance of only 300 feet from the dam. The boulder talus is here 53 feet in length; but undermining has been hitherto only prevented by the construction of expensive temporary weirs of boulders and piling carried right across the earthen channel. The width of the natural channel is here about 800 feet; the highest known flood has never risen over the top of the piers, or 10 feet above the flooring of the escape dam, through which the aggregate width of waterway is 470 feet up to a height of 6 feet, or 570 feet above that level.

The declivities of the beds of these torrents are as follow, vide Ganges Canal Report :-

| Ranipoor | $\ldots$ | $\ldots$ | $14 \cdot 11$ | foot per mile. |
| :--- | :---: | :---: | :---: | :---: |
| Puttree | $\ldots$ | $\ldots$ | $24 \cdot 43$ | do. |
| Rutmoo | $\ldots$ | $\ldots$ | 8.23 | do. |
| Solanee | $\ldots$ | $\ldots$ | 5.08 | do. |

The length of the temporary weir nearest to the dam was 500 feet, which was in the spring of the present year increased to 680 feet ; the result of this enlargement of the passage for floods is now seen in extensive silting up of the bed for a considerable distance below.

Sir Arthur Cotton alludes to the Gunnarum aqueduct, constructed across one of the minor branches of the Godavery, as an instance of the possibility of passing river floods with perfect safety through a very contracted waterway ; this work I had heard of years ago and was at a loss to account for the difference of resulte there, and in the torrents with which we have to deal in Northern India, until I was informed by an Officer of the Irrigation Department in Madras that the Gunnarum aqueduct was constructed over a tidal river at a point where, as well as he could recollect, the rise of the tide was from 3 to 4 feet (the distance in a direct line from the sea is, by the plan given in the late Colonel Baird Smith's " Madras Irrigation," about 18 miles) ; there must therefore be a certain depth of backwater constantly standing on, or close up to the flooring which would fully account for the absence of the dangerous channelling out in the bed lower down, which is invariably found to be the case under similar circumstances with us.

That the present waterway under the Solanee aqueduct is just sufficient and no more is evident from the absence of silting under the arches, and the perfect condition of the channel below.
As to the proposed contraction of the width of the channel for the canal above, supposing the velocity of current through it to be increased to three times that in the earthen channel, or 9 feet per second, the up traffic would be entirely stopped, as Sir A. Cotton himself subsequently observes, without the addi-
tion of a separate lock chamber at the side and machinery for heaving the boats, \&c., up the rapid. To work either of these an establishment must be permanently maintained. Besides this a large basin, protected at the sides by masonry revetments or otherwise, would be necessary in the heavy embankment at the lower end of the aqueduct, similar to those below the overfalls, to counteract the effects of the accelerated velocity and eddying action there. The expense of all this would be scarcely less than that of the work as it stands, and the wear and tear on the masonry would be infinitely greater than at present.
132. The remaining nine objections refer to points connec- Hindrances to navigated with the navigable capabilities of the canal and water tion. communication in general. One or two points only require notice here.

With reference to No. 5, it is stated that to combine navigation satisfactorily with irrigation, the lines of canal should be reduced below where the irrigation ceases, to levels by locks, and an opinion is given that the last 30 or 40 miles of the Cawnpore Branch should have been thus dealt with. This would restrict the extent of the present irrigation materially, as I have shewn elsewhere that the lowest outlet is at the Dubowlee Bridge within three miles of the terminus. It is further stated (in No. 14) "that there is no communication between the canal and river at Cawnpore." I can only say that when I saw the works in March last, the communication was perfectly free; not a hindrance existed to the passage of boats or rafts through the terminal lockage. Great difficulty had been experienced more than a year previously in working hinged gates at the lowest lock, owing to the quantity of silt continually deposited by the river water. Since the substitution of iron drop-gates, noticed elsewhere in this report, no interruption to the traffic has occurred.
133. None of the defects enumerated above (some of Irrigation returns affocthem have been long known, and acknowledged by the pro- ted by only two of the aljector and the canal officers to be so) except the original projection of the bed on too great a declivity and the absence of means for contracting the waterway at the falls, have, I maintain, affected the irrigation revenue to any appreciable degree; however large the navigation returns may ultimately be, they can never amount to more than a fraction of the former.

The temporary nature of the works on the Ganges at the heads has not hitherto, as far as I have been able to discover, been the cause of any failure of irrigation from short supply except in one single season.
134. In conclusion, I protest in the name of my brother Claim set up by Secreofficers of the Canal Department in these Provinces against tary to Irrigation Compathe claim set up by the Secretary to the East Indian Irrigation ny. and Canal Company in a letter to Sir A. Cotton, published at the end of his pamphlet, that the professional opinions and suggestions regarding the Ganges Canal works put forth by him "belong absolutely to the East India Irrigation and Canal Company." Sir Arthur Cotton, it is true, came here in the interest of the Company; he was afforded every facility for obtaining information by the canal officers; in fact, the very defects of the works, were, as he himself states, in some instances pointed out to him. On this he wrote the Memorandum above
alluded to, solely as he states for the information of the Company, and without any view to its publication. The paper, however, somehow or other, found its way into print, and has given rise to a public discussion of the professional opinions contained therein. The subjects treated of have been discussed by those concerned in and out of India for years past, and reports and suggestions have been at different times made on every point mooted in the above pages; especially a Memo. drawn out in considerable detail last year at the request of the Superintendent General of Irrigation, long before Sir Arthur Cotton's pamphlet was heard of in this country, on the rectification of the works from the heads of the Ganges Canal to Cawnpore.

Some of these views and suggestions are necessarily identical with those put forth by Sir Arthur; in others, opinions disagree ; but that in the instances, and there are several in these projects, where our views do coincide with his, they are therefore to be considered the private property of the Company is about as preposterous a claim as could be put forward.

With the personal feelings which have unfortunately become linked with this discussion I have no concern. It is a matter of great regret to us all, that two such men as Sir Proby Cautley and Sir Arthur Cotton, who have done more in their respective spheres of action for the material development of this country than any others either in the past or the present generation, should have thus come into unpleasant collision.

My task is done, as far as time and the information at hand would permit. Unpleasant and invidious as it ever must be to be obliged to criticise and point out defects in a brother Engineer's work, it has been specially so in the present instance, where the projector of those works was for many years our loved and honored chief. If the views here advocated appear to his and other minds to be legitimate conclusions from the facts, and otherwise reasonable, though they differ in some important points from the ideas which guided the original projection, let it be borne in mind that they are those of men trained in his school, who have had the advantage of the accumulation of experience in subsequent years. To detect errors is an easier task than to produce faultless designs.
J. CROFTON, Capt., r. e., On Special Duty, Ganges Canal.

## Roorkee,

 23rd November 1864.
## APPENDIX A.

## Memorandum of the Measures recommended to be taken for Remodelling the Ganges Canal.

It is admitted by every one that the Ganges canal was originally designed with too strong a slope for its bed. The primary result of this error is the crosion of the earthen channel. From this follow two secondary results. First, the formation of rapids on the masonry works which occur along the canal, by which the foundations are undermined. Second, a direct dangerous action takes place on the masonry of the falls, at which the level of the tail water has been in many cases very greatly depressed by the erosion of the earthen channel; and a condition of things is brought about which these works were not designed to meet, and which, in fact, must eventually almost certainly destroy them if some complete restoration of the original water levels is not accomplished.

It will plainly be impossible to deal with the secondary results of the erosion of the bed until this erosion is arrested; and the only way of effecting this is to reduce the velocity of the water, which can be effected either by diminishing the slope of the bed, or the volume of water in the canal, or by a combination of these two things.

Obviously, the reduction of the volume of water alone in the existing canal cannot be entertained as a remedy, as the first condition of the problem to be solved is, that the whole supply of the Ganges, capable of being passed over the Solani aqueduct, shall be made available for irrigation. Hence the slope must be reduced by the introduction of additional falls, or a new channel to carry some of the water must be formed, or these two remedies might be applied in conjunction.

In proceeding to consider further which plan shall be adopted, it is at once apparent that irrigation from the canal having been already largely developed, it becomes almost essential that the present supply of water shall be maintained in the canal, while the measures requisite for the permanent restoration of the works are in progress. To stop the canal for a lengthened period would cause most serious complications with the cultivators now using the water, independently of the actual injury done to the country by the withdrawal of the means of irrigation. Such a step as the closure of the canal, for longer periods than are now customary, that is, a month or two in the year, should not be taken unless as a last resort, after all other means have failed.

It is calculated that the canal, as now constructed, could safely carry a depth of about four feet of water and no more. To reduce the discharge from its present amount, the depth being about seven feet, to a depth of four feet would, Colonel Turnbull considers, lead to a reduction of about half of the existing irrigation, and a money loss of about four lakhs of rupees a year. It would, therefore, seem to follow that any expenditure short of four lakhs a year that was necessary to maintain the 7 -foot supply in the canal till the restoration is completed, will not be attended with greater loss to the Government than would be involved by the reduction of the supply, and the consequent falling off of revenue. At the same time, there can be no question that the community will be much better off if the supply is maintained; and on the whole, so far as it is found practicable to do this, it should be done.

Immediate steps, therefore, should be taken to secure the works that have been injuriously affected by the action of the water. The most practicable means of protecting the foundations of the falls and bridges will probably be found in theuse of loose stone to be brought down the canal from Hurdwar by boats, and the necessary steps should be taken at once for providing an ample fleet for this purpose. The canal bed should be very carefully watched henceforth, and a periodical review of the state of the bottom should be made, say every three months, so as to place beyond doubt what is the result of the remedial measures adopted. Of course the Canal Officers will use their discretion in the use of piles, or any other appliances, besides throwing in loose stones below the masonry works, that may, from time to time, be necessaay to secure the end in view.

It may here be remarked that the history of the Ganges canal seems to indicate the importance of the systematic watching of the bed of all irrigation canals of any magnitude, with a view to the Engineers being kept thoroughly acquainted with this most essential element of their works. The Irrigation Officers should be desired to consider this point,
and to frame rules under which yearly or half-yearly seports on the canal bed should be brought under the review of the heads of that department.

In proceeding further to consider the more precise form that should be given to any remedial measures, it may first be remarked that the bed of the canal above the Solani aqueduct must plainly be dealt with as it stands, and any excess of slope must be got rid of by additional falls in the present channel. There is here no possibility of a supplementary channel.

Below the Solani aqueduct greater freedom is possible, and the proposal of Sir Proby Cautley to form a supplementary or alternative channel from Roorkee, passing round by Deobund, and re-uniting with the original line at the suitable point lower down its course, calls for careful attention. If this plan were adopted, the old canal would either be re-modelled so as to carry with security an enlarged body of water, or would be made use of without material change to carry the largest possible volume that it could do with safety, which, as before observed, would give a depth of about four feet. This depth, it may be added, would be equivalent to about one-fourth of the full canal supply of 6,750 cubic feet per second.

Captain Crofton has satisfied us that it is not likely to be worth while to undertake any operations on the existing canal bed, in the way of re-modelling the slope, short of fitting it to carry the whole supply. It seems probable that at least one additional fall between each pair of existing falls would be requisite to admit of half the full supply being carried on the present line. Such falls would, of course, be less in height than falls suitable for the full supply, but the difficulty of construction would be very nearly as great in the one case as in the other, the important thing in both being the foundations, and the work to be done below the water level of the country.

In making comparison between the plan of a simple re-modelling of the present canal, and the plan of adopting a supplementary canal, the question is simplified, therefore, by the consideration that such a supplementary line would have to carry about ${ }_{3}$ ths of the whole supply, while the old canal would carry, without important change, the remaining fourth. We have in fact to compare a properly re-modelled system of works on the existing canal, and a supplementary line to carry $\frac{3}{3}$ ths of the full supply.

The two primary considerations that should influence a decision in favour of one or other of these plans will be their expense, and the time that they will require for completion. An important, but still a secondary point, will be the relative convenience of the two arrangements hereafter.

The comparative cost of these two plans it is quite impossible to conjecture, without real designs and estimates, but their probable difficulties, advantages, and disadvantages may usefully be touched upon, with a view to forming a judgment whether there may be any a priori ground for giving a preference to either at once-immediate action being most important.

The obvious advantage of the first plan of re-modelling the present canal, (which for brevity will be referred to in the sequel as Plan A, while the alternative will be termed Plan $B$ ) is that it will admit of the original general design of the works being maintained almost intact, and will interfere very little with the system of minor irrigation channels already in existence. It is supposed that all, or at all events most of the existing bridges will be utilized, and possibly the existing falls might be used with safety with certain precautions and modifications, when the excessive slope has been removed by the erection of the new intermediate falls. On this plan the excavation to be undertaken would be small, and the quantity of materials for masonry works very much less than in Plan B. The water being all in one channel, the waste from absorption and evaporation would be less than if it were in two, and also the ultimate expense for repairs and maintenance, including establishment, would be sensibly less in Plan A.

On the other hand, the disadvantages of Plan A. would be the difficulties of constructing the necessary falls on the existing channel, while a constant supply of water was maintained in it. These difficulties will be of such a nature as to make it very difficult to form a trustworthy estimate, either of the true cost of the works, or of the time that they will require for their completion. They will also add to the risk of the works themselves being constructed in a less complete and thoroughly durable manner than might be anticipated if Plan B. Were adopted. These objections have very great weight with some of us.

The advantages of Plan B. will be-first, the facility of construction, and of forming a reliable estimate of cost and time of construction,-and second, the very great convenience that may be anticipated from the possession of a duplicate line of main channel throughout so great a distance, which would admit of a supply of considerable amount being sent down to the lower parts of the canal, while either of the alternative main lines was closed for repairs.

Although the existing canal cannot, with safety, carry more than one-fourth of the supply for a permanence, it might on an occasion of pressure, without serious risk, be made to take down probably one-half of the full supply for a limited period. Should it appear on the preparation of proper estimates that Plan B is likely to be more costly than Plan A, it will be a matter for consideration, whether these advantages are fairly commensurate with the increased cost.

The advantages of Plan B. will be that in part, at least, it must probably be regarded as not available as an irrigation channel, inasmuch as the supply of water is hardly sufficient for the works already designed and in course of completion in the lower districts of the Doab. It will involve the passage of the canal over the drainage of large areas of the Doab, requiring aqueducts in probably three parts of its course, thus introducing a fresh element of cost in addition to the requirements for bridges and falls. Further, although the works will be less difficult, they will, in absolute quantity, be heavier, and require more material ; the material again requiring land carriage to a greater extent than will be the case on the open canal.

On the whole, we do not feel ourselves to be in a position to give an absolute preference to either plan with the information now before us. But this difficulty is of less importance than might at first sight seem likely. Whichever plan were adopted-supposing that we could absolutely have preferred one to the other-a complete design and estimate would be quite essential before attempting to put any work in hand. The time that will suffice to prepare the designs and estimates for one set of works will, if suitable arrangements be made, suffice for the preparation of the like documents for the other set. Therefore we advise that immediate steps should be taken to draw out projects for both plans of operations, on the completion of which, the Government might probably think fit to cause them to be put before us, and to call for our further opinions as to the final decision to be come to.

We therefore advise that Captain Crofton, R. e, should at once be entrusted with the duty of drawing out projects and estimates under the two suppositions above referred to, and that he should have placed under him as Assistants, Mr. Hair, and another officer, to be selected by the Government, North Western Provinces. Mr. Mossop (Punjab Irrigation Department, should also be placed under Captain Crofton's orders as soon as his present season's work is over.

It may best be left to Captain Crofton to consider, in his own manner, in communication with the Government of the North Western Provinces, through Colonel Turnbull, the precise form to give to both projects. The question whether navigation should be provided for in the new supplementary canal, or whether a separate navigation canal would not be preferable, is one that may be left for Captain Crofton's consideration. Also he might be asked to bear in mind the possibility of reinforcing the Ganges canal during the rainy months with the waters of the Jumna, which are in excess of the requirements of the Eastern and Western Jumna canals, and which now flow off to waste. The capacity of the Solani aqueduct is an absolute limit to the supply of water that can be brought into the Doab from the Ganges at Hurdwar, but this supply is notoriously insufficient for the irrigation of the whole available area that lies between the Ganges and Jumna, and the present occasion seems one on which this general question might usefully be looked into.

Having in view the length of time that would be required to provide bricks in sufficient quantity for the works that will, under all circumstances, be required for the objects that have here been discussed, and remembering that the Punjab Railway Company is just about to commence operations on the country through which the alternative line of canal will pass; also having regard to the value of stone as a material for the construction of the falls, it seems specially requisite to take measures for examining properly the hills near Hurdwar, with the intention of finding a suitable stone for building purposes, if possible. There is reason to suppose that a fair description of sand-stone may be got, and the water carriage by the canal will now admit of such a material being brought down at a reasonable rate, if a stone of sufficiently good quality can be found.

For this purpose it is proposed that Mr. Kelly (lately returned from leave to England be specially nominated to examine and work the Hurdwar quarries, and to take charge o the provision of this description of material should our anticipations be realised. In the event of good stone being found, the needful staff of quarry-men will readily be obtained by the Government, North Western Provinces.

To meet the charge that will be incurred it carrying out the proposals above made, the Government of the North Western Provinces might make a transfer of credit to the extent of one lakh of rupees from such head of service as is most convenient.

Inasmuch as it will be important to restrict irrigation from the Ganges Canal, at all events to its existing limits, until the works are placed in a thoroughly secure position, it should be at once ordered that no new engagements should be entered into for the supply of
water. So far as is practicable, the work on rajbuhas under construction may be suspended. Should the officers of the Irrigation Department see their way to any reduction of the extent of irrigation without involving breach of engagements, or pressure on the country, it might be advisable to effect it, and a strict limit should, without loss of time, be placed on the maximum quantity of water to be admitted into the canal.

R. STRACHEY, Colonel, r. E.,<br>Secy. to the Gout. of India.<br>W. E. MORTON, Lieut. Col., R. E.,<br>Secy. to Govt., N. W. P., P. W. D.<br>A. D. TURNBULL, R. E.,<br>Supdt. Genl. of Irrigation, N. W. P<br>J. H. DYAS, Captain, R. E.,<br>Director of Canals, Punjab.<br>J. CROFTON, Captain, re e,<br>Offg. Supt., E. J. Canal.



The Committee of the Engineer Officers, with the fullest data before them, have, so far as I can understand, come to the conclusion, that it is not likely to be worthwhile to undertake operations on the existing canal in the way of re-modelling slope, short of fitting it to carry the whole supply.

The canal as now constructed, it would appear, can safely carry a depth of 4 feet of water, and no more. To re-model the bed, so as to carry a greater depth, say one-half of the full supply whilst the remaining half is carried by a separate channel, would, the Committee observes, be very nearly as difficult as to re-model the existing channel, so as to carry the full supply, the important item in both being the foundations, and the work to be done below the water level of the country.

The Committee, therefore, starts with the idea, that the existing channel, which can, without injury, carry a supply of 1,700 cubic feet per second, may be required so as to carry that volume only, the separate channel being projected to carry the remainder, viz., 5,050 cubic feet per second.

The objection to this appears to be-
1st. The supply provided for the irrigation of the lands under the existing channel (or what in the new scheme would become the Eastern Branch,) viz., 1,700 cubic feet per second would be insufficient for their irrigation, their wants being as follow :-
$\begin{array}{ccccc}\text { For the line from Roorkee to the Futtehgurh Branch Head, including the } & \\ \text { supply for the Futtehgurh Branch } & \ldots & \ldots & \ldots & \ldots \\ \text { For } & \ldots & \mathbf{1 , 4 2 0} \\ \text { Brane south from the Futtehgurh Branch inead to the Bolundshuhur } & \\ \text { Botaling the supply of the Bolundshuhur Branch } & \ldots & \mathbf{1 , 0 0 0} \\ \text { Total required for Irrigation on existing channel, cubic feet per second } & \ldots & \mathbf{2 , 4 2 0}\end{array}$
$2 n d$. That to make a separate or alternative line to carry 5,050 cubic feet per second, would be a very large and a very expensive work, very nearly as large, in fact, as the existing canal, whilst the existing canal would be left with a body of water in it, so small as to bear no comparison with its capacity and the extent of its works.

Now my original views on this division of the water were based on the following :-
lst. The absolute necessity of maintaining in all its integrity the supply passing the Bolundshuhur Head, viz., 4,330 cubic feet per second, for the irrigation of the Southern Districts, as originally projected.

2nd. The absolute necessity of securing to the existing channel from Roorkee to the Bolundshuhur Branch Head (6750-4330) 2,420 cubic feet per second.

I was not, of course, aware at the time of the actual quantity that the existing channel could with safety carry, but I estimated this at the full amount required for the irrigation, viz.,

2,420 cubic feet per second. It appears that I over-estimated its value, and that it will carry 1,700 cubic feet per second only, or 720 cubic feet less than what is required for irrigation.

A channel that will carry 1,700 would, with a very little improvement I imagine, carry 2,420 ; at any rate, I consider it to be indispensable that this supply should be secured to the existing line.

In addition to this, 2,420 cubic feet per second to be carried on the existing line, I proposed an additional quantity, making up the volume to 3,750 cubic feet per second, leaving the remainder to be carried in the separate or alternative line.

> From Roorkee to the Futtehgurh Branch Head, the eastern or existing channel would have to bear ... ... ... ... ... ... 3,750
> From the Futtehgurh Head to the Bolundshuhur Head $3750-1420=2,330$

whilst the remaing 3,000 cubic feet per second would be carried in the Western Channel.
I considered that the present supply, in defiance of the difficulties which encompassed it, might be maintained, until the Western Channel was completed. I looked upon the completion of this Western Channel, in the first instance, as a necessary part of the scheme, so that 3,000 cubic feet per second might be at once securely available.

The Eastern or existing channel, which now can only carry 1,700 cubic feet per second, would then be re-modelled so as to carry 3,750. This would, of course, necessitate additional falls of masonry which, although requiring deep and well protected foundations, would not (as calculated to carry 3,750 cubic feet per second) require to be of so large a size as those now in existence for the carriage of 6,750 .

In fact, the size of the falls is an element not to overlooked. Should the whole body of water be retained in the present or existing channel, the extent, and consequently the cost of constructing additional falls, will be very much more than if built for the reduced volume of water. I leave out of the question the advantage of dealing with more manage able masses of water, and the very valuable resources of having an alternative line for the security of the irrigation of the Southern Districts.

My object in writing this memorandum is, simply to point out the grounds upon which I originally suggested, the division of the water. I had much rather see the whole matter left in the hands of Captain Crofton (an Officer in whose skill and judgment I have great faith) and the Committee of Engineer Officers, who, as being on the spot, are provided with details, with which I am imperfectly acquainted, than that my views, as here submitted, should be officially dealt with.

## (Signed) <br> P. T. CAUTLEY.

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APPENDIXB.
TABOLAR STATEMENT showing positions of Masonry Works, changes in declivity and dimensions of Channel, on the present line of the Ganges Canal, with the


| APPENDIX B.-(Continued.) <br> N. B.-Proposed alterations in Antique Type. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} \text { Distance from } \\ \text { Head. } \end{gathered}$ |  | NAME OF WORK. |  |  |  |  |  |  |  |  |  | Bridass. |  |  | Redicrd iktirs op ard. |  | Projected alterations and additions. |
|  |  |  |  |  |  |  |  |  |  |  |  |  | 寅 | At prosent. | If remodel- |  |
| ${ }^{6}$ | ${ }_{\substack{\text { Feet. } \\ \text { 5111 }}}$ |  | Bahadoorabad Fall 8.93 lock on separate channel | 170 | $\cdots$ |  | $\because 0024$ | $\cdots$ | $\xlongequal[6692]{ }$ | ${ }^{6} 7750$ |  | $\left\lvert\, \begin{array}{\|c\|c\|} \hline \text { Lk. br. } \\ 19 \cdot 5 \end{array}\right.$ |  | 25 | 28 | $\begin{aligned} & 219 \cdot 70 \\ & 22 \cdot 63 \end{aligned}$ | $\begin{aligned} & 219 \cdot 70 \\ & 228 \cdot 63 \end{aligned}$ | Bridge to be raised. Grating to be attached; now entrance to lock channel; walls of lock to be raised. |
| 7 | 2527 | Bahadoorabad Fall 8.89 look on seperate channel | $\cdots$ 160 145 | $\cdots$ $\cdots$ 9.75 13 | 1. <br> 1.5 <br> 2 | $\begin{gathered} \cdots \cdots . . \\ 9382 \\ 9000 \end{gathered}$ | $\begin{aligned} & 4 \cdot 001 \\ & 3 \cdot 138 \end{aligned}$ | ${ }_{69760}^{6750}$ | $\begin{gathered} \cdots . . . . \\ \hline 7750 \\ 7100 \end{gathered}$ | 20 | $\left\lvert\, \begin{gathered} \text { Lk. br. } \\ 19 \cdot 5 \end{gathered}\right.$ | 8 | 25 | 18 | ${ }_{2238 \cdot 19}^{22 \cdot 30}$ | $229 \cdot 30$ 239 | Bridge to be raised. Grating; now exit from lock channel; Bridge of lock to be raised; Bed lowered 1'4 below Fall. |
|  |  | Sulempoor Drainage Inlet R. B. $150^{\prime}$ wide | 140 | 13 | 2 | ...... | ${ }^{\text {...... }}$ | ...... | $\begin{aligned} & 700 \\ & \ldots . . . . . \end{aligned}$ | 6.3 |  |  |  |  |  |  |  |
| 9 | 3930 | Puttree Superpassafe and lock attached, Fall 9.22. | ‥ 133 145 | … 1.05 18 | 1.5 2 |  | -..... 4.188 3.138 | 6541 6976 | $67 .$. 6900 | 20 | 15.34 | 8 | 25 | 300 | $\begin{aligned} & 240 \cdot 92 \\ & 250 \cdot 14 \end{aligned}$ | 240'92 | Grating: direct entrance made into lock. Bed lowered 6'06 below Fall. New look down stream. |
| 11 | 4173 | Badshapoor Drainage Inlet R. B. 50 wide | ... |  |  | ...... | ${ }^{\text {...... }}$ | ...... |  | 10 |  |  |  |  |  |  |  |
| 12 | 4179 | Dhunowrie Bridge <br> Rutmoo Inlet and Outlet <br> Dams, | ... | ... | ... | ...... | ...... | ...... | ..... | 20 | 20 | 3 | 55 | 18 | 253-83 | 253'83 | Towpath. |
| 13 | 328 | $\begin{aligned} & 800 \text { ' waterway above ... } \\ & \text { Ditto Regulating Bridge } \end{aligned}$ | … 150 150 | … 10 | … <br> 1.5 | $\begin{gathered} \ldots . . . . \\ \cdots \\ 4293 \end{gathered}$ | $\begin{gathered} \cdots . . . . \\ \cdots \\ 409 \end{gathered}$ | …... $\cdots 6750$ 0. | ….. <br> $\cdots$ <br> 6750 <br> 100 | $\dddot{23}$ | is | 10 | 20 | 18 | ${ }_{254}^{253.93}$ | ${ }_{257}^{25 \%} 93$ | Regulating apparatus; Side arches to be raised. Bed below lowered 3'64. |
|  |  |  | 150 | 12.8 | 1.6 | 9000 | ${ }^{3} 152$ | 6826'7 | ${ }^{6750}$ | 10 | 33 | 3 | 55 |  | 255.53 |  | Floor lowered 314.-Towpath. |
|  | 5070 | Muhemar ditto at North end of aqueduot | ... | ... | ... | ...... | ...... | ...... | ...... | 10 | 22 | 3 |  | 18 | 237.84 | 2580 289 281.42 | Ditto 179. ditto. |
| 18 | 404 | Solanee Aqueduot (Centre). |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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APPENDIX B.-(Continued.)
N. B.-Proposed alterations in Antique Type.

| $\begin{gathered} \text { Distance } \\ \text { frome } \\ \text { Head. } \end{gathered}$ |  | NAME OF WORK. |  |  |  |  |  |  |  |  |  | Briders. |  |  | Redoczd Levels of Bed. |  | Projected alterations and additions. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | At present. | $\begin{array}{\|} \text { If remodel- } \\ \text { led. } \end{array}$ |  |
| iles. | Feet. |  | Peepulheree Bridge Raolee Ditto <br> Boolundshuhar Branoh Regulating Head Main Channel <br> Boolundshnhar Branch ... |  | 9.512$\cdots$$\cdots$$\ldots$$\ldots$$\ldots$$\ldots$9$10 \cdot 6$ | 1.5 2 | ${ }_{9000}^{4275}$ | 3.938 2.985 | ${ }_{4787}^{4648}$ | 4850 |  |  |  |  |  |  |  |  |
| 106 108 | ${ }_{3889}^{4094}$ | ... |  |  |  | $\stackrel{\text {-..... }}{ }$ | $\cdots$ | $\stackrel{\text { …... }}{ }$ | ….... | 16 17 | 16 16 | ${ }_{3}^{3}$ | 45 | 20 18 | $\begin{aligned} & 433 \cdot 68 \\ & 436.04 \end{aligned}$ | 437.05 438.20 | Floor lowered 3.37. Towpath. Arch raiged. |
| 110 | 1746 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | $\cdots$ |  |  |  | $\cdots$ | $\ldots$ | ....... | …... | 5\% 15 | $\cdots$ | $\because$ | 20 | 18 | 437\% 94 | 489764 |  |
|  |  | ... |  |  |  | $\ldots$ | ...... |  | $\ldots$ | ... | 14 | 2 | 15 | 18 | $437 \cdot 64$ | 437.64 |  |
|  |  | 1.5 2 |  |  |  | $\begin{aligned} & 4275 \\ & 7500 \end{aligned}$ | $\begin{aligned} & 3: 845 \\ & 3: 084 \end{aligned}$ | $\begin{aligned} & 4274 \\ & 4872 \end{aligned}$ | 4330 |  |  |  |  |  |  |  |  |
| 113 | 3801 |  | ... |  | $\ldots$ | $\stackrel{\text {....... }}{ }$ | ....... | ........ | $\stackrel{\text {....... }}{ }$ | $\begin{gathered} 5 \cdot 6.5 \\ \text { to } 7.6 \\ 5 \cdot 3 \\ \text { to } \end{gathered}$ | 16 |  | 15 | 18 | $\stackrel{441.87}{ }$ | 440.02 422.08 | Towpath. Arch raired. |
| 116 | 3338 |  |  |  |  |  |  |  |  |  | 16 |  |  |  |  | $447 \cdot 28$ |  |
| 119 | 4335 |  | $\cdots$ | ... |  | ...... | ...... | ............ | ...... |  |  | 3 | 45 | 18 | 445.55 |  | Floor lowered 177. Towpath. Arch raised. |
|  |  |  |  | ... |  |  |  |  |  | to 6.38 | 16 | 3 | 458 |  | $\begin{aligned} & 449 \cdot 52 \\ & 453 \cdot 16 \end{aligned}$ | 449.52 | Towpath. Arch raised. <br> Ditto Ditto. |
| 122 | 3290 | Sunowta ditto ditto <br> Pukhana Fall 4.67 | … | $\ldots$ | $\begin{aligned} & \ldots \\ & \ldots \end{aligned}$ | $\ldots$ | $\cdots$ | $\ldots$ | …... | to 9 | ${ }^{16}$ | ${ }_{3}^{3}$ | 45 | 18 |  | $\begin{aligned} & 451 \cdot 49 \\ & 453 \cdot 69 \end{aligned}$ |  |
|  |  | Pukhana Bridge ditto … Dumkoura Bridge Rajbuha Outlets Urowlee Bridge and Raj-buha Outlets, Sills 2 ft. above bed | $\ldots$ | ... | ... | ...... | ...... | $\cdots$ | ...... | $\left\lvert\, \begin{gathered} \begin{array}{c} 5 \cdot 5 \\ \text { to } \\ 6.3 \\ \text { to } 6.3 \end{array} \end{gathered}\right.$ | 16 | 3 |  | 18 | $\begin{aligned} & 457 \cdot 09 \\ & 460 \cdot 33 \end{aligned}$ | $458 \cdot 26$ | - |
| 128 | 2909 |  | ... | ... | ... |  |  |  |  |  |  | 3 |  |  |  |  | Floor lowered 1.17. Towpath. Arch raised. Towpath. Aroh raised. |
| 131 | 1175 |  |  |  |  | ...... | ... | $\cdots$ | ...... |  |  |  |  |  |  |  |  |
|  |  |  |  |  | ... |  | ...... | ..... |  | $t_{0}{ }_{8}^{5}$ | 16 | 3 |  | 25 | $463 \cdot 81$ | 462.22 | Ditto. Ditto. |


APPENDIX B.-(Continued.)


APPENDIX B.-(Continued.)




|  | $\stackrel{\square}{2}$ | ํㅗㅁ | $\stackrel{\square}{-}$ | $\pm \pm$ |  | $\stackrel{8}{4}$ | $\stackrel{\square}{\sim}$ |  |  |  | $\pm \infty$－ | $\because$ |  |  |  |
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| ตงッ凶 | $\stackrel{ }{*}$ | ＊＊ | $\sim$ | जッハ | $\bigcirc$ | $\sim$ | $\sim$ | － | $\cdots$ | ーい | －－1 | － | － | － | $\infty$ |
|  | 号 | 号冎 | \％ | 哯品呙 |  | ¢ | 吕 | ¢ |  | ๓®ワ | ェッロ | $\stackrel{\sim}{\sim}$ | $\stackrel{\square}{\sim}$ | 遃 |  |
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| ： | ¢ | ！ | $\vdots$ | ！！！ | $\vdots$ | $\vdots$ | $\vdots$ |  |  | ！ | ： |  |  |  |  |



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|  | $\begin{aligned} & \stackrel{8}{+} \\ & \underset{\sim}{2} \end{aligned}$ |  |  | 禺管下 |  | ..... | $\begin{aligned} & \text { Wo } \\ & \text { in } \end{aligned}$ | $\begin{aligned} & \text { 융 } \\ & \text { 1 } \end{aligned}$ |  |  | $\stackrel{\oplus}{\stackrel{\oplus}{\dot{\theta}}}$ | $\begin{aligned} & \text { O} \\ & \text { 啇 } \end{aligned}$ |  |
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| ©i ioisio | $\underset{\boxed{E}}{\square}$ | +i | $\dot{\otimes}$ | ※苋： | ઠઠઠઠ犬 | ஹ் | :ö | 犬 | \％だっ | O울 | $\stackrel{\oplus}{\mathrm{E}}$ |  |  |


| ส－$\infty$ | $\stackrel{\square}{-}$ | ロッ | $\stackrel{\square}{-}$ | $\pm \infty$ |  | $\stackrel{1}{*}$ | $\stackrel{\text { ® }}{ }$ | 3 | $\stackrel{\square}{\infty}$ | 禺が | ロのロー | $\stackrel{\square}{-}$ | － |  |  |
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APPENDIX B.-(Continued.)


|  |  | $\begin{aligned} & \text { O. } \\ & \text { íd } \end{aligned}$ | $\begin{aligned} & \text { ฐู } \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \hline \stackrel{\oplus}{\circ} \\ & \dot{\mathbf{O}} \end{aligned}$ | $\begin{aligned} & \infty \\ & \stackrel{\infty}{\oplus} \\ & \stackrel{N}{\infty} \end{aligned}$ |  |  | $\begin{gathered} \text { ت゙ } \\ \text { む్ర゙ } \end{gathered}$ | $\begin{aligned} & \text { O. } \\ & \stackrel{\oplus}{8} \end{aligned}$ |  | $\underset{\underset{\sim}{\mathrm{i}}}{\underset{\sim}{*}}$ |  | ¢ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
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| \％ | ั๐ | $\stackrel{\sim}{\circ}$ | \％ | \％ | ® | ¢ $\%$ | めが気 | ${ }^{\circ}$ か | สั | สิ่쳐ํ | $\cdots$ | สラミタ | $\bigcirc$ |
| $\checkmark$ | $\cdots$ | $\checkmark$ | － | $\cdots$ | $\cdots$ |  | － | カ－ | $\cdots$ | いい | $\cdots$ | いの－の | － |
| $\stackrel{\ominus}{\rightrightarrows}$ | $\stackrel{\circ}{=}$ | $\stackrel{\bullet}{\ddot{\sim}}$ | $\stackrel{\bullet}{=}$ | $\stackrel{\bullet}{=}$ | $\stackrel{R}{\dot{g}}$ | $\stackrel{\circ}{\oplus}$ | Nợ io | $: \stackrel{\circ}{\ddot{2}}$ | $\stackrel{\circ}{\square}$ |  | $\stackrel{\bullet}{=}$ |  |  |


APPENDIX B.-(Continued.)



| $\stackrel{\infty}{\sim}$ | $\stackrel{\infty}{\sim}$ | $\underset{\sim}{\infty}$ | $\underset{\sim}{\infty}$ | $\underset{\sim}{\infty}$ | $\infty$ | $\infty$ | ッロッツ | ล | $\underset{\sim}{\infty}$ | －${ }_{\sim}^{\infty}$ | $\underset{\sim}{\infty}$ | －${ }_{\sim}^{\infty} \times \infty$ |  |
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| － | － | － | N | ＊ | $\cdots$ | －r | の－ヵの | か－ | $\cdots$ | がい | $\cdots$ | いいいの | $\cdots$ |
| $\stackrel{0}{\square}$ | $\stackrel{0}{\square}$ | $\stackrel{\infty}{\underset{7}{7}}$ | $\stackrel{\infty}{\underset{\sim}{\square}}$ | $\stackrel{\bullet}{Ð}$ | $\underset{\sim}{9}$ | 只 |  | ： | $\stackrel{\bullet}{\square}$ |  | $\stackrel{9}{\square}$ |  |  |

APP BENDIX B.-(Continued.)

| $\begin{gathered} \text { Distnnice } \\ \text { from } \\ \text { Head. } \end{gathered}$ |  | NAME OF TORE. |  |  |  |  |  |  |  |  |  | Buidars. |  |  | Redoced Lervels of Bed. |  | Projected alterations and additions. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  | 容 |  | At present. | $\begin{array}{\|c} \text { If remodel- } \\ \text { led. } \end{array}$ |  |
|  |  |  | Jaolee, Megalating Head | ${ }_{70}^{70}$ | TEHGURII BRANCHI. |  |  |  |  |  |  |  |  |  |  |  | $\begin{aligned} & 344 \cdot 81 \\ & 354: 35 \end{aligned}$ |  |
|  | 0 | \%\% |  |  |  |  |  |  |  |  |  |  | 20 | 18 | $333 \cdot 59$ |  |  |
|  | 4884 | as <br> $\ldots .$. <br> $\ldots .$. <br> $\ldots .$. <br> $\ldots$. <br> $\ldots 9.5$ <br> 69 |  | 6 | 2 | 5500 | 2721 | 1225 | 1240 |  |  |  |  |  |  |  |  |
| 2 | 4884 |  | let LB | $\cdots$ |  | $\cdots$ | $\cdots$ | $\cdots$ | $\cdots$ | $\cdots$ | is | 1 3 | ${ }_{25}^{10}$ | 18 | $337 \cdot 48$ |  |  |
| ${ }_{6}$ | ${ }_{3206}^{660}$ |  | Juturara, llajbuha Outiet | ... | $\ldots$ | ...... | ...... | ...... | ..... | ... |  |  |  |  | 35748 |  |  |
|  | 3300 |  | Lith 1 Bridge $\ldots$ | $\ldots$ | $\cdots$ | $\cdots$ | $\cdots$ | $\cdots$ | $\cdots$ | $\ldots$ | 13 | ${ }_{3}^{1}$ | 10 20 |  |  |  |  |
| 8 | 边3960 |  | Sumbu hera Dito ... | $\ldots$ | $\cdots$ | $\cdots$ | ...... | $\cdots$ | ..... | $\ldots$ | 13 | 3 | 25 | 20 | 34449 |  |  |
| 10 | 3300 |  | Churima fill 7.94 | ... | $\cdots$ | ...... | ...... | .... | ...... | ... | $\cdots$ | , | 20 |  | 346.89 35483 |  |  |
|  |  |  | $\begin{gathered} 69.5 \\ 63 \end{gathered}$ | $\cdots$ | $\begin{array}{r} 1.5 \\ 2 \end{array}$ | $\begin{array}{r} 4190 \\ 5500 \end{array}$ |  |  |  |  |  |  |  |  |  |  |  |
| 12 | 3300 | Bhoomnh Bridge | \%8:5 | .... | 715 | $\stackrel{4333}{ }$ | ...... | $\cdots$ | ...... | $\cdots$ | 13 | 3 | 25 | 18 | 36733 | $\begin{aligned} & 360.59 \\ & 370.21 \end{aligned}$ |  |
| 14 | 4620 | Meerpoor Bridge ... | 67\% | … | 1.5 | 4333 | ...... | $\cdots$ | ..... | $\cdots$ | 13 | 3 | 25 | 18 | 360.14 |  |  |
| 16 | 720 | $\underset{\text { Akbarpoor, Rejbuha }}{\text { letat- }}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 17 | 660 |  | ... | $\ldots$ | $\cdots$ | $\ldots$ | $\ldots$ | $\ldots$ | $\cdots$ | $\ldots$ | … |  | 20 | is | $363.02$ $370 \cdot 96$ |  |  |
| 18 | 4620 | Kol Dridge ... | ${ }^{66}$ | $\ldots$ | 1.5 $\cdots$ | ${ }^{4175}$ | ..... | ...... | ..... | ... | $\ldots$ | 3 | 25 | 20 | 373.15 |  |  |
| 21 | 4620 | Nowana Bridgo | 65.5 | $\cdots$ | 1.5 | 4310 | $\ldots$ | ...... | ...... | ... | ... | 3 | 20 | 18 | 376.90 |  |  |
| 24 | 1023 | Dhakuulee, Rajbaha Out- | 64 | ... |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | 1320 | Het, R13, | $\cdots$ | $\cdots$ | … | $\cdots$ | ....... | $\stackrel{\text {....... }}{ }$ | $\stackrel{\text {...... }}{ }$ | $\cdots$ | ... |  | ${ }_{20}^{6}$ | 18 | 379.97 <br> 787 |  |  |
|  |  |  | 62.5 | ... |  |  |  |  |  |  |  |  |  |  |  |  |  |


APPENDIX B．－（Continued．）

| Distance from Head． |  | NAME OF WORK． |  |  |  | 『 <br> \％ <br> 总 |  |  |  |  |  | Briders． |  |  | Rediced levels Q pred． |  | Projected alterations and additions． |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | At present． | If remodel－ |  |
| $\begin{gathered} \text { Miles. } \\ 80 \end{gathered}$ | $\begin{array}{r} \text { Feet. } \\ \mathbf{6 6 0} \end{array}$ |  | Anchoura Bridge $\quad . .$. | 37.5 | $\cdots$ | $\because \mathrm{i} \cdot 5$ | $\stackrel{4222}{ }$ | $\cdots$ | ．．．．．． | ．．．．．． | $\cdots$ | $\cdots$ |  |  | 20 |  |  |  |
| 82. | 2970 | $\left\{\begin{array}{cc}\text { Ganges River } \\ \text { Ditto Bridge }\end{array}\right.$ |  |  | ．．． | ．．．．．． | ．．．．．． | ．．．．．． | ．．．．．． | ．．． | ．．． | 2 |  | 24 |  |  |  |

AITHRNATIVE LINE, GANGES CANAL:

| Distanc | m Head. | Bottom width | Depth of Water. | $\begin{aligned} & \text { Side slopes } \\ & 1 \text { in } \end{aligned}$ | Deoivity of bed 1 in | $\begin{gathered} \text { Mean } \\ \text { velooity } \mathrm{ft} \text {. } \\ \text { per sea. } \end{gathered}$ | Discharge calculated o ft. per sec. |  | Discharge required c . <br> ft. per seo. | Reduced level bed. | Remaris. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\underset{0}{\text { Miiles. }}$ | $\underset{0}{\text { Feet. }}$ | Branch Head $170$ | Old line, 21 New line, 24 | penings of 6 do. of 6 <br> 2 | eet earh, Road eet each, do. $10,000$ | 5 feet -•• <br> 2.676 | $5,086 \cdot 7$ |  |  | 261.25 | Inck on right bank into new line. <br> Ditto from left bank old line into old ohannel. |
| 2 6 7 8 | $\begin{aligned} & 2500 \\ & 3000 \\ & 4000 \end{aligned}$ |  | of 51 feet itto <br> ela N. 2 ar | each, towpat <br> of 20 feet | $\text { is } 7 \text { feet, Road }$ | 18 feet ... | $\begin{aligned} & \cdots \\ & \cdots \\ & \cdots \end{aligned}$ | $\ldots$ | ... $\cdots$ $\cdots$ | $262 \cdot 55$ $264 \cdot 71$ 265.34 $\{265 \cdot 47$ |  |
| 8 | 0 | Fall 6 feet, | bays of 15 | et, Bridge a | efore, Road |  | ... | . | ... | $\left\{\begin{array}{l}265147 \\ 271\end{array}\right.$ | Fall No. 1. |
| 10 | 2640 | Do. | do. | do. | do. | .... | ... | . | $\ldots$ | $\left\{\begin{array}{l}272 \cdot 79 \\ 278 \cdot 79\end{array}\right.$ | " No. 2. |
| 11 | 2000 | Culvert for D | ainage one | ch of 10 feet | ... |  | $\cdots$ | $\ldots$ | $\ldots$ | 279.2.3 |  |
| 13 | 0 | Fall 6 feet, 1 | bays of 15 | t, Bridge as | before, Road | 8 feet | ... | .. | ... | $\left\{\begin{array}{l}2 \times 0 \cdot 11 \\ 286 \cdot 11\end{array}\right.$ | " No. 3. |
| $\begin{aligned} & 14 \\ & 15 \end{aligned}$ | 2640 | Aqueduct, 2 Bridge, 4 ar | ches of 40 es of 31 feet | t, Canal wat ach, towpath | rway 190 feet 7 feet, lioad | ray 18 feet | $\ldots$ | $\ldots$ | $\ldots$ | $\begin{array}{r}286.89 \\ 287 \\ \hline 280\end{array}$ | Orer Kalee Nuddee. |
| 16 | 0 | Inlet for drai | age, 10 feet | reh | . ... | ... | ... | .. | $\ldots$ | $\left\{\begin{array}{l}280 \cdot 22 \\ 287 \cdot 69\end{array}\right.$ |  |
|  |  | Bridge as be Do. | re, Roadway do. | 25 feet 20 feet | $\begin{aligned} & \ldots \\ & \ldots \end{aligned}$ |  |  | $\ldots$ |  | 2888.42 289.47 | Road from Mozuffernuggur to Seharunporo. " " Deobund to Bahlah. |
| 19 20 | $\begin{array}{r} 2000 \\ \theta \end{array}$ | Culvert for | doinage, one | 20 feet arch of 10 fe | $\ldots$ | $\cdots$ | $\cdots$ | $\ldots$ | $\ldots$ | $\begin{array}{r}2899 \\ \hline 2980\end{array}$ | " " Deobund to Bahlah. |
| 22 | 0 | Fall 6 feet, 1 | bays of 15 | et, Bridges a | before, Road | ay 18 foet | ... | ... | ... | $\left\{\begin{array}{l}290 \cdot 86 \\ 296 \cdot 86\end{array}\right.$ | No. 4. |
| 24 | 0 | Do. | do. | do. | ... | ... | ... | ... | ... | $\left\{\begin{array}{l}297 \cdot 91 \\ 303 \cdot 91\end{array}\right.$ | No. 5. |
| 26 | 0 | Da. | d. | do. | ... | ... | ... | ... | ... | $\left\{\begin{array}{l}304.97 \\ 310.97\end{array}\right.$ | No. 6. |
| 29 | 0 | Bridge as be | re, Roadwa | 18 feet |  | ... | ... |  | ... | 31255 |  |
| 31 | 0 | Fall 6 feet, 1 | bays of 15 | t, Bridge as | before, Roadw | 520 feet | ... | ... | ... | $\left\{\begin{array}{l}31361 \\ 319.61\end{array}\right.$ | No. 7 Road from Mosuffernuggur to Thanah Bhowun. |
| 35 | 0 | Do. | do | do. | ... | ... | ... | ... | ... | $\left\{\begin{array}{l}3: 1773 \\ 327 \cdot 73\end{array}\right.$ | No. 8 Road from Mozuffurnuggur to Shamloe. |

APPENDIX B.-(Continued.)

|  |  |  |  |  | T. B.-Datum | line 200 fe | above floor | of Myap | Regulating | e. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Distance from Head. |  | $\begin{aligned} & \text { Bottom } \\ & \text { Width. } \end{aligned}$ | Depth of Water. | $\begin{aligned} & \text { Side slopes } \\ & 1 \text { in } \end{aligned}$ | Deolivity of bed 1 in | Mean velocity. <br> ft. per seo. | Discharge calculated c . ft. per see. | Discharge required c. ft. per sec. | Reduced level bed. | Remaris, |
|  |  | Fall 6 feet, 11 bays of 15 feet, Bridge as before, Roadway 18 feet |  |  |  |  |  |  |  |  |
| Miles. | Feet. |  |  |  |  |  |  | ... | $\left\{\begin{array}{l}329.31 \\ 335.31\end{array}\right.$ | No. 9. |
| 38 | 0 |  |  |  |  |  |  | . | \{336.89 | No. 10. |
| 41 | 0 | Do. Bridge as |  |  | do. | ..- | $\cdots$ | - ... | $\left\{\begin{array}{l}342 \cdot 89 \\ 344 \cdot 14\end{array}\right.$ |  |
| 43 | 2000 |  | Fall 6 feet, 11 bays of 15 feet, Bridge as before, Roadway 20 feet |  |  |  |  |  | - | $\{345.63$ | No. 11 Road from Mosufturnuggur to Boorhaneh. |
| 46 | 0 |  |  |  |  |  |  | ... | - ... | $\left\{\begin{array}{l}351.53 \\ 353.12\end{array}\right.$ | No. 12. " " |
| 49 | 0 | D |  | do. do. |  | 18 feot | ... | - ... | $\left\{\begin{array}{l}359 \cdot 12 \\ 3512\end{array}\right.$ |  |
|  |  |  | Do. Dam brid | ge 11 arches |  |  |  |  | $\left\{\begin{array}{l}360 \cdot 17 \\ 366 \cdot 17\end{array}\right.$ | No. 13. |
| 61 | 0 |  | Dam bri | ge 11 arches | of 23 feet |  | ... |  | 366.17 |  |
| 53 | 0 ) | Escape Head, 15 arches of 6 feet Aqueduct, 4 arches of 50 feet, Canal $\dddot{W}$ aterway, $\dddot{1} 90$ feet |  |  |  | $\text { t } \cdots$ | $\cdots$ |  | 367.23 368.65 3 | Over Kalee Nuddee. |
| 55 | 2640 | Aridge as before, Roadway 18 feet. |  |  | ... | $\ldots$ | $\ldots$ | ... | 368.50 $\mathbf{3 7 0} 0$ |  |
| 68 59 | 2000 0 | Raju. |  |  | et … | .... | $\ldots$ | ... | $370 \cdot 40$ |  |
|  |  | Branch Head, Main, Fall 6 feet, 10 bays of 10 feet, Bridge 3 arohes of 40 feet, Roadway 18 feet |  |  |  |  |  |  | $\left\{\begin{array}{l}371.45 \\ 377.45\end{array}\right.$ | No. 14. |
| 61 | 0 |  |  |  |  |  |  |  | $\left\{\begin{array}{l}371 \cdot 45 \\ 374 \cdot 71\end{array}\right.$ |  |
|  |  | 90 | 7 | 2 | 5400 | 3.000 | 2184 | 2240 |  |  |
| 1 | 2000 | Bridge 3 arches of 40 feet, Towpath 7 feet, Roadway 18 feet |  |  |  |  |  | ... | 376.05 378.63 |  |
| 4 | 0 | Fall 6 feet, 6 bays of 14 feet, Light 10 feet Bridge |  |  |  |  | ... | ... | $\left\{\begin{array}{l}384 \cdot 63 \\ 385.28\end{array}\right.$ |  |
| 4 | 3510 | Junction above Jutpoora Bridge ... |  |  |  | ... | ... | ... |  |  |
| 61 | 0 | 104 | 8 | 2 | $6500$ | $2 \cdot 925$ | 2808 | 2760 | 377.45 | Below Branoh Head. |
| 63 | 0 | Bridge 3 arobes of 40 feet, Roadway 18 feet ... ... ... ... ... |  |  |  |  |  |  | 379.07 |  |



APPENDIX B.-(Continued.)
N. B.-Datum line 200 feet above flooring of M yapore Regulating Bridge.


APPENDIX B.-(Continued.)
SHPARATH NAVIGABLE IINE.


| 3000 | Ditto ditto. |
| :---: | :---: |
| 4000 | Syphon for Rajbuha, 6 feet. |
| 1000 | Bridge as before, Roadway 18 feet. |
| 6000 | Culvert, 2 arches of $\mathbf{2 0}$ feet |
| 4500 | Double lock, 6 feet Bridge, Roadway 18 feet |
| 0 | Ditto Ditto |
| $\begin{array}{r} 1000 \\ 0 \end{array}$ | 2 Syphons for Rajbuha, 6 feet each. Bridge as before, Roadway 18 feet. |
| 0 | Double lock, 9.73, Bridge, Roadway 10 feet ... |
| 2000 | Double lock chamber, bed level, Roadway 10 feet |
| 4000 0 | Bridge as before, Roadway 20 feet. |
| 1000 | Ditto Ditto 18 feet. |
| 2000 | Bridge as before, Roadway 18 feet. |
| 0 | Syphon for Rajbuha 6 feet. |
| 0 | Double iock, 6 feet, with Bridge, Roadway 18 feet |
| 300 | Syphon, 2 arches of 6 feet |
| 3000 | Bridge as before, Koadway 18 feet. |
| 5000 | Syphon for Kajbuha 6 feet. |
| 0 | Double lock, 6 feet Bridge, Roadway 18 feet |
| 4000 | syphon for Rajbuha 6 feet. |
| 2000 | Bridge as before, Roadway 18 feet. |
| 0 | Double lock, 8 feet, Bridge, Roadway 20 feet ... |
| 4500 | Syphon for drainage, 10 feet wide. |
| 0 | Bridge as before, hoadway 20 feet. |
| 4000 | Syphon for Kajbuha, 6 feet wide. |
| " | Bridge as before, Koadway 18 feet. |
| 0 | Double lock, 6 feet Bridge, Roadway 18 feet ... |
| $\begin{aligned} & 4500 \\ & 2000 \end{aligned}$ | Syphon for drainage, 1 arch of 6 feet. bridge as before, loadway 18 feet. |
| 0 | Double lock, 6 foet Bridge, Roadway 20 feet |
| 2000 | Culvert for drainage, 1 arch of 10 feet. |
| 3000 | Bridge as before, Koadway 25 feet |
| 0 | Ditto Ditto 18 feet. |
| 4500 | Ditto Ditto |
| 4500 | Syphon for Rajbuha 6 feet. |
| 0 | Double lock, 6 feet with Bridge, Roadway 18 feet |
| 4500 | Bridge as before, Roadway 18 feet. |



APPENDIX B.-(Continuel.)


## APPENDIX C.

## Formole for Discharges, \&ce.

The formulæ employed for calculating velocities and discharges throughout these projects are as follows :-

1. Velocities in open channels "in train."
$\mathrm{V}=90 \sqrt{\frac{\mathrm{R}}{\mathrm{S}}}$ for mean velocities up to 3 feet per second.
$V=93 \sqrt{\frac{R}{\mathrm{~S}}}$ for mean velocities above 3 feet per second.
where $V=$ mean velocity in feet per second.
$\mathrm{R}=$ Hydraulic mean depth

$$
=\frac{\text { Area of section }}{\text { Length of wetted perimeter. }}
$$

$S=$ Length in which the fall is 1 foot.
The co-efficients 90 and 93 are somewhat larger than those given in treatises on hydraulics; they are those, however, most nearly conformable with observation on the streams with which we have to deal in this country; where silt is as a rule held in suspension, by which the velocity of current is sensibly diminished. The nature of the soil in bed and banks also affects the mean velocity.
N. B.-All measures, whether lineal, superficial, or cabic, are in English feet, and decimals of feet.
2. Mean velocity $(\mathrm{V})=81 \times$ surface velocity.
$=81 \mathrm{v}$
3. Discharge through trapezoidal notch, longest side uppermost.

$$
\mathrm{D}=\frac{2}{15} d \sqrt{2} g h\left(3 b_{b}+2 b_{t}\right) \times \mathrm{C}_{d}
$$

where $\mathrm{D}=$ discharge
$2 g=64 \cdot 403$
$h=$ depth of water from surface to sill of notch.
$d=$ length of notch (or height, if vertical)
$b_{b}=$ width of notch at sill

$$
b_{t}=\quad \text { ditto } \quad \text { at top }
$$

$\mathrm{C}_{d}$ is a varying co-efficient, depending on proportion of width to height of notch, size and form of channel of approach, \&c.; it has been taken at 62 in the cases occurring in these calculations.
4. Approximate equation for proportion of lower to upper width of notches, or between grating bars.

$$
\begin{aligned}
\mathbf{A} & =\text { upper width } \\
\mathrm{MA} & =\text { lower } \\
\mathrm{M} & =0.62 \sqrt{\frac{.5}{h}}
\end{aligned}
$$

where $h=$ depth of water on sill of notch.
5. Discharge over weirs with vertical sides (fall "complete")

$$
\mathrm{D}=\frac{2}{3} \mathrm{C}_{d} l \sqrt{2 g}\left(h+h_{a}\right)^{\frac{3}{4}}
$$

where $l=$ width of weir.
$h=$ depth from normal surface of water above weir to top or sill of weir.
$h_{a}=$ height due to velocity of approaching water.
Remainder as before.

The co-efficient $C_{d}$ in the cases of the Canal overfalls has been taken at 628 , and if $v_{a}$ represent mean velocity of approaching stream, $h_{a}=\frac{v_{a}{ }^{2}}{C_{d}^{2} \times \sqrt{2 g}}$ and the above equation becomes by substitution and reduction.

$$
\mathrm{D}=3.36 l\left(h+0.03937 v_{a}^{9}\right)^{\frac{3}{2}}
$$

6. Discharge over weirs as before (fall "incomplete")
$\mathrm{D}=O_{d} l \sqrt{2 g\left(d_{1}+h_{a}\right)}\left\{d_{2}+{ }_{3}^{2}\left(d_{1}+h_{a}\right)\right\}$
$d_{1}=$ height of fall or difference between normal level of water above and below fall.
$d_{9}=$ depth of top of weir below normal surface level of water below fall.
Remainder as in last equation.
Substituting for $C_{d}$ and $h_{a}$ the same values as in last equation we obtain
$\mathrm{D}=5.0897 l \sqrt{d_{1}+0.03937 v_{a}{ }^{2}}\left\{d_{2}+\frac{2}{3}\left(d+0.03937 v_{a}^{2}\right)\right\}$
The observations for discharge of the Ganges Canal channels taken for the parposes of this report were obtained as follows :-

Two cross sections of the stream were taken at a uniform distance apart of 200 feet, the depths of water being measured at every ten feet or less along the width of each section.

The velocities were obtained by noting the times of transit at several points in the width of the stream of floats from the upper to the lower section; these floats were made of painted deal rods about an inch square, loaded at one end so as to float nearly vertically and pass as close to the bed of the channel as possible without touching, their upper ends projecting a few inches above the water's surface. They were found in every case to float in a line closely parallel to the thread of the current. A very near approximation, it is evident, was thas obtained to the mean velocity in the vertical plane traversed by each. A correction for the small height of the end of the float above the bed was applied to each velocity before using them for the calculations of discharge, viz.-
$C($ or multiplier of velocity $)=1-0.116\left\{\left(\frac{D-D_{1}}{D}\right)^{\frac{1}{2}}-0.1\right\}$
where
$\mathrm{D}=$ depth of water
$D_{1}=$ length of rod immersed.
This was given in the report on the Mississippi by Captain Humphreys and Lientenant Abbot as obtained by Mr. Francis in his experiments at the Lowell Water-works, where velocities of current were observed in a similar manner to the above.

Velocities were observed on the Ganges Canal and elsewhere also by a current meter of similar construction to that known as Woltmann's hydrometer as well as by sarface floats, but no method I have yet tried seems to me so satisfactory as that of the floating rods. The declivity of the water's surface was also in most cases observed for the purpose of comparison and obtaining reliable co-efficients for calculation.

## APPENDIX D. <br> Velocities of Corbent.

In a portion of the channel of the Eastern Jumna Canal lying in the old bed of the Muskurra torrent, where the current seemed perfectly adjusted to a light sandy soil, Major Brownlow, the Superintendent of the Canal, found the mean velocities of the surface to be from 2.38 to 2.28 feet per second, or mean velocities (multiplying by 0.81 ), 1.928 to 1.847 per second.

In the lower district of the same canal, near Barote and Deola, the maximum surface velocities with a fair supply were found to be 2.817 and 2.507 feet per second, or mean velocity of 2.282 and 2.03 feet per second. Silt is constantly being deposited here; the soil is similar to that below Sirdhanna on the Ganges Canal.

About 1000 feet below the Ghoona Falls on the same canal in very sandy soil, with nearly a full supply of water, I found the maximnm surface velocity to be 3.077 feet per second, or mean velocity 2.492 feet per second : no erosion from bed or banks, except when a supply much in excess of the maximum allowed is passing down.

Below the Nyashahur bridge on the same canal, where the soil is very similar to that between the Myapoor and Kunkhul bridges on the Ganges Canal, clay, shingle and small boulders, Lieutenant Moncrieff, R. E., the Officiating Superintendent, found the mean surface velocity, to be 6.751 feet per second, or the meau velocity about 5.468 per second.

The same officer observed the surface velocity at some distance below the Yarpoor Falls in the new centre division channel of the Eastern Jumna Canal, and obtained a mean of 3.957 feet per second, or about $3 \cdot 205$ feet per second mean velocity through entire section. The soil here is light and sandy, and the channel has been both widened and deepened by the current.

In one of the rajbuhas (or main water-courses) of the same canal I found weeds growing in the bed and on the sides with a maximum surface velocity of $2 \cdot 12$ feet per second, or mean velocity (V) of about 1.717 feet per second; the soil is sandy with a fair admixture of clay; silt accumulates to a troublesome extent.

In another rajbuha in the same neighbourhood I found a surface velocity of 2.38 feet per second, or mean about 1.927 feet. Silt deposits here, but no weeds appear to grow.

In the Muhmoodpoor left bank rajbuba on the Ganges Canal I found grass and weeds growing in the channel with a maximum surface velocity of 1.724 feet per second, or mean of 1.396 feet.

In the Buhadoorabad Lock Channel, Ganges Canal, weeds appear to grow wherever the maximum surface velocity is 2.38 feet, (or mean velocity 1.928 feet per second, or under); soil generally light and sandy.

On the Ganges Canal I found velocities as follow :-Below the Roorkee bridge on the main canal, where the deepened bed is covered with silt, and erosion from the sides has ceased, the mean velocity in the entire section with a supply less than the present maximum on the Roorkee gauge by two inches, was 2.92 feet per second, the soil sandy with a tolerable admixture of clay.

In the widened channel at the Toghulpoor sand hills, Mile 36 , the mean velocity, with full supply now allowed to pass down, obtained by calculation from the area of the water section and the discharge observed below Roorkee (deducting expenditure en route) was 2.532 feet per second.

In the embanked channel across the Solanee Valley, with a supply two inches under the present maximum on the Roorkee gauge, the mean velocity, obtained by calculation from the area of water section there and the observed discharge through the masonry aqueduct, was 3.04 feet per second. The deepest portions of the channelling out here, I have stated elsewhere, have been silted up.

At the 50 th mile main line, below the Jaolee Falls, with (present) full supply in the canal, the observed mean velocity was 3.059 feet per second. Erosion from the banks has ceased here; silt on the deepened bed, soil sandy.

Above Newarree bridge, 94 th mile, in a stiff clay soil, with full supply in the observed mean velocity, was 4.117 feet per second : erosion trifling here; no silt deposit.

Observations communicated by Captain Dyas, R. E., Director of Canals, Punjab.
On the Hansi branch of the Western Jumna Canals, silt was deposited with mean velocities of from 2 to 2.25 feet per second. The deposition of silt, however, obviously depends on the quantity and specific gravity of the matter held in suspension'by the water coming from above, and the ratio of the current velocities at different points along the channel.

He states, from observations on the channels of the Baree Doab Canal, that in sandy soil " 2.7 feet per second appears to be the highest mean velocity for non-cutting as a general rule, for there are soft places where the bed will go with almost any velocity ; but those sorts of places can be protected." Again, "Bad places might be scoured out with a mean velocity of 2.5 feet per second, but better soil would be deposited in place of the bad with a slightly smaller velocity than 2.5 feet; and as the supply is not always full, there would be no fear of not getting that slightly smaller velocity very frequently. The good stuff thus deposited would not be moved again by any velocity which did not exceed 2.5 feet per second."

In "Neville's Hydraulics," 0.83 or 1.17 feet per second are mentioned as the lowest mean velocities which will prevent the growth of weeds. This, however, will vary with the nature of the soil; vegetation also is much more rapid and vigorous in a tropical climate than where Mr. Neville made his observations.

In Captain Humphrey's and Lieutenant Abbott's report on the Mississippi, 1860, it is mentioned that the alluvial soil near the mouth of the river cannot resist a mean velocity of three feet per second ; and that in the Bayou La Fourche, the last of its outlets, which resembles an artificial channel in the regularity of its section and general direction, and the absence of eddies, \&c., in the stream, the mean velocity does not exceed three feet per second, and the banks are not abraded to any perceptible extent.

From the foregoing and other observations, which it would encumber this paper too much to place on record here, and taking into consideration that the higher the velocity the less the works will cost, I think the following may be taken as safe mean velocities with maximum supply in the Ganges Canal channels.

1. In the Ganges valley above Roorkee three feet per second.
2. In the sandy tract generally between Roorkee and Sirdhanna, 2.7 feet per second.
3. In very light sand, such as that met with at the Toghulpoor sand hills, not higher than 2.5 feet per second.
4. And for the channels south of Sirdhanna, 3.0 feet per second.

On the branches the same data to be assumed according to similarity of the soil.
There are soils, as Captain Dyas has noted, such as light quicksand, which will not stand velocities of even 1 foot or $1 \frac{1}{\frac{1}{2}}$ foot per second, but these are never found to any great extent in one place : erosion there can only have a local influence, and such places can be protected at a trifling expense. It is channelling out on long lines which is to be feared.

## APPENDIX E. <br> Vertical Falls with Grating.

The grating consists of a number of wooden bars resting on an iron shoe built into the crest of the fall, and on one or more cross beams according to the length of the bars. These bars are laid at a slope of 1 in 3, and are of such length that the full supply level of the water in the canal tops their upper ends by half a foot. The scantling of the bars as well as that of the beams should of course be proportioned to the weight they have to bear, plus the extra accidental strains to which they are liable, from floating timber for instance, which may possibly pass between the piers and so come in contact with the grating. In consideration of strains and shocks of this nature the supporting beams are set with their line of depth at right angles to the bars instead of vertically.
2. The dimensions of the bars used on falls of the Baree Doab Canal; where the depth of water is 6.6 feet, are as follows :-

## Diyar wood.

Lower end of bars $0.50^{\prime}$ broad $\times 0.75$ deep
Upper end of bars $0.25^{\prime}$ broad $\times 0.75$ deep
and they are supported on two Diyar beams each measuring 1 foot in breadth $\times 1.5$ foot in depth; the first beam being placed at a distance of 7.5 foot (horizontal measurement) from the crest of the fall, and the second 7.5 foot beyond the first beam. The bars of the gratings on these falls were originally placed touching each other (side by side) at their lower ends, as there was not then a full supply of water in the canal. There were thus 20 bars in each 10 foot bay. Since then the number of bars has been successively reduced to 19 and to 18 , the present number. The reduction of the number of bars and the equal spacing of the remaining bars is done with ease, as they can be pushed sideways in the iron shoe and along the beams, to which latter they are held with spike-nails. Once the correct spacing is arrived at, cleats and blocks (as shown in the drawing) are preferable to spikenails.
3. The end elevation of the bars, scale $\frac{1}{5}$ th full size, shows the way in which the bars are undercut from the point where they leave the shoe, $i$.e., from the crest of the fall. This undercutting has the effect of making each space as it were "an orifice in a thin plate," and it facilitates the escape of small matters which may be brought down with the current. Large rubbish which accumulates on the grating is daily raked off and piled on one side of the fall. This is done by the establishment kept up for the neighbouring lock. There is considerable advantage in thus clearing the canal of rubbish which would otherwise stick in rajbuha heads or piers of bridges, \&c., or eventually ground on the bed of the canal and become nuclei of large lumps and silt banks.
4. As one main object of the grating is to prevent the stream above the fall to which it is fixed from knowing that there is such a thing as a fall anywhere below it, the principle to go on in spacing the bars is to arrange them so that the velocity of no one thread of the stream shall be either accelerated or retarded by the proximity of the fall. This effected, it is evident that the surface of the water must remain at its normal slope, parallel to the bed of the canal, until it arrives at the grating. The half foot by which the water tops the bars of the grating above described causes a sudden drop there, but the acceleration to the current resulting from so small a fall as this is not practically felt to any distance back from the fall.
5. To take an example, let us assume that V (mean velocity) $=0.81 \mathrm{v}$. (surface velocity) and $U$ (bottom velocity) $=0.62 v$. (surface velocity) in every vertical line of the current flowing naturally. Then, if we make $V=2.5$ feet per second, we shall have the following velocities at the given depths below surface in a stream 6 feet deep.

| Depths below surface. | Velocities : feet per second. | Remarks. |
| :---: | :---: | :---: |
| Surface ................. 0 | 3.0864 |  |
| $\mathbf{1}^{\prime}$ | 2.8909 | , |
| - 2 | 2.6955 |  |
| Centre ................ $\mathbf{3}^{\mathbf{3}}$ | 2.5000 | Common difference 0.1955 nearly. |
| $\mathbf{4}^{\prime \prime}$ | 2.3046 | $11$ |
| - $\mathbf{5}^{\prime}$ | 2.1091 | $1$ |
| Bottom .................. 6' | 1.9136 | $1 J$ |

6. What is required, then, is to shape the sides of a given number of bars placed in a given width of bay so that the above velocities may be maintained till the water touches the grating, when in consequence of the clear fall the velocity becomes considerably accelerated. This accelerated velocity multiplied by the reduced area (of space between the bars) should
give the same discharge, with the canal running full, as the products of the original normal velocity and the original undiminished space, the width of which is of course the distance between the centre of two contiguous bars.
7. Thus, taking the lowest film (along the bed of the canal) whose normal velocity is 1.9136 feet per second, and supposing 20 to be the number of bars in each 10 feet bay, then the undiminished space for each portion of the stream will be half a foot, which multiplied by the above velocity gives a product of 0.9568 . Again, taking the same lowest film as it passes through the grating, with a clear fall, and under a head of pressure of six feet, we find its velocity to be 19.654 feet per second. Now, if we call the required width of space between the bars at this point $x_{b}$ and assume the co-efficient of contraction to be 0.6 , we
shall have $x_{b}=\frac{0.9568}{19.654 \times 0.6}=0,08$.
8. Similarly taking the film on the level of the tops of the bars, or 0.5 below the surface of the water, the normal velocity of which is 2.9887, the undiminished space being as before $0^{\prime} .5$, we get a product of 1.4944 ; and as the velocity of the film falling through the bars is 5.673 feet per second, we get

$$
x_{t}=\frac{1.4944}{5.673 \times 0.6}=0^{\prime} .44
$$

9. And lastly, taking the centre film, the normal velocity of which is 2.5 feet per second we have a product of 1.25 , and as the velocity of the same film passing through the grating is 13.89 feet per second, we get

$$
x_{a}=\frac{1.25}{13.89 \times 0.6}=0^{\prime} .15
$$

10. Hence it is seen that the sides of the bars should be cut to a curve convex towards the open space, but in practice this nicety is scarcely requisite.


The effect of catting the bars straight is of course to increase the discharge through the centre of the grating, and to diminish it at the surface.


But this is not found objectionable in practice, for, as mentioned in para. 4, the surface velocity has already been somewhat accelerated by the half foot drop at the top of the grating, and in consequence of the tendency of the lower part of the grating to clog with matter brought down by the current, there is no risk of undue acceleration to the bottom velocity.
11. Niceties of detail have not been gone into in the foregoing calculations. For instance, the natural diminution of the velocity from the centre towards the side of the H 2
earthen channel has not been taken into account, nor have the obstructions caused by the piers, the slope of the grating, and the accelerating effect (on the velocity through the grating), of the velocity of approach. The object of this paper is merely to indicate the general principle of the arrangement. Those who may have to fix gratings to falls would of course work out all needful details for themselves, according to the peculiar circumstances of each case; and should practice afterwards show that the theoretical spacing requires correction, the requisite re-adjustment of the bars is, as explained in para. 2, a very simple matter.
12. The above remarks have been limited to a consideration of the effect cansed by the grating on the channel above the fall. Its effect on the channel below the fall is equally important, but this may be gone into separately. For the present it may suffice to remark that the formula in use on the Baree Doab Canal for the lower bed of the channel is

$$
x=\sqrt{h} \sqrt[3]{d}
$$

in which equation
$x$ is the required depth of cistern
$h$, the height of fall, or the difference of level between the surface of the water above the fall and the surface of the water below it,
and $d$, the full supply depth of water in the channel. All the cisterns with depths thus obtained have answered admirably, having never required the slightest repair since they were built.
13. Another point may be mentioned as worthy of particular attention. The diminution of the waterway immediately under the grating and below the fall by the numerous piers of these falls as built on the Baree Doab Canal holds up the water, and causes it to rush out from the bays with considerable velocity. It might be found advantageous to turn arches through these piers, so as to give the water free side-play, or perhaps even to support the gratings and pathway on iron piles instead of on brickwork piers. In this case the road bridge if founded on piers of brickwork should be moved lower down to the point where the widening of the protected portion of the channel is greatest. The cisterns also, instead of ending abruptly with a vertical wall, might have their bottoms connected with the bridge flooring by a long counterslope so as to give the fallen water more room and time to get into the true normal velocity of the current in the open channel.
J. H. DYAS, Captain, R. E.

Dalhousie, 30th July, 1864.


APPENDIX F ,-(continued).
Levels of the Surface of Springs along the line of the Ganges Canal.

|  | Distance from edge of Canal. |  | Reduced Levels. |  | Springs. |  | Remarks. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Right Bank. | Left <br> Bank | Surface Springs. | Bed of Canal. | Above bed. | Below bed. |  |
|  |  |  | R. 430.29 |  | 5.01 |  |  |
| 106 |  |  | L. 430.64 | 435.30 | 4.66 | $\ldots$ |  |
|  |  |  | R. 434.47 |  | 3.58 | ... |  |
| 108 |  |  | L. 433.75 | 438.05 | 4.30 | ... |  |
| 108 ${ }^{\frac{2}{3}}$ |  | 400 | L. $\begin{array}{r}434.68 \\ \text { R. } \\ 438 \\ \hline\end{array}$ | 436.04 | 1.36 | $\ldots$ | At Raolee Bridge. |
| 110 |  |  | R. 438.74 <br> L. 435.25 | 437.90 | 2.65 | 0.84 $\ldots$ |  |
| $110 \frac{1}{3}$ |  | 300 | 434.37 | 437.64 | 3.27 | ... | At Bolundshuhur Branch Head. |
| 117 | 500 |  | 445.95 | 449.32 | 3.37 | ... | Near Jarcha Bridge. |
| 120 | 1500 |  | 450.37 | 451.34 | 0.97 | $\ldots$ | Near Gusopoor Bridge. |
| 123 |  |  | 454.06 | 455.21 | 1.15 | $\cdots$ | Near Sunowta Bridge. |
| 125 | 1200 |  | 457.08 | 457.61 | 0.53 | 1.90 |  |
| 12812 | 100 |  | 462.23 46650 | 460.33 <br> 467 |  | 1.90 | At Dumkoura Bridge. |
| 132 134 |  |  | 466.50 468.23 | 467.93 470.20 | 1.43 1.97 | ... | Near Urrowlee Bridge. <br> Near Wallipoora Bridge. |
| 138 |  |  | 474.50 | 474.78 | 0.28 | ... | Near Manun Bridge. |
| 139 | 600 |  | 476.27 | 475.79 |  | 0.48 |  |
| 143 |  |  | R. 479.23 | 479.41 | 0.18 |  | Near Moonda Khera Bridge. |
| 145 |  |  | R. 488.29 | 481.26 | $\ldots$ | 1.03 | Near Suhenda Bridge. |
| 149 |  |  | L. 492.37 | 491.59 | $\ldots$ | 0.78 | Near Pulra Falls. |
| 152 |  |  | R $\begin{array}{r}494.80 \\ \text { P0385 }\end{array}$ | 494.65 | $\ldots$ | 0.15 5.79 | Near Koel Branch Head. |
| 162 | 33 |  | R. $\begin{array}{r}\text { 503.30 }\end{array}$ | 49806 507.72 | 4.42 | 5.79 | Near Burolee Bridge. <br> Near Dubthulla Bridge. |
| 172 | 213 |  | 521.05 | 524.89 | 3.84 | ... |  |
| 175 | 97 |  | 525.09 | 529.34 | 4.25 | $\cdots$ | Near Chungeyree Bridge. |
|  |  |  | Cawnpoor | Branch. |  |  |  |
| 2 | 156 |  | 539.83 | 538.12 |  | 0.71 |  |
| 8 | 230 |  | 545.85 | 546.85 | 1.00 | ... |  |
| 12 | 121 |  | 550.82 | 551.31 | 0.49 | $\cdots$ |  |
| 17 | 344 |  | 556.12 | 559.57 | 3.45 | $\ldots$ |  |
| 21 | 228 346 |  | 562.62 564.48 | 565.08 564 504 | 2.46 0.49 | $\ldots$ |  |
| 30 | 68 |  | 572.38 | 574.65 | 2.27 | $\ldots$ |  |
| 33 | 307 |  | 576.44 | 577.07 | 0.63 | ... |  |
| 37 | 427 |  | 582.82 | 584.01 | 1.19 | $\cdots$ |  |
| 40 | 330 |  | 593.65 | 586.88 | ... | 6.77 |  |
| 43 45 | 154 89 |  | 593.38 | 590.44 | $\cdots$ | 294 3.65 |  |
| 4 | 89 |  | 59.80 | 592.15 | $\ldots$ | 3.65 |  |

APPENDIX G.
Statement of outlay on the Ganges Canal Works from commencement up to 30th April 1863.

| Division or Section in which charge were made. | Obieinat Worrs. |  |  |  |  | $\underset{\text { pairs. }}{\text { Ordinary }}$ Re- | Rajbuhas. | Plantations. | $\begin{gathered} \text { Sundries or } \\ \text { Profit and Loss. } \end{gathered}$ | *Contingencies. | Establishment. | Totals. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Main Live. | Cawnpoor Branch. | Etawah Branch. | Futtehgurb Branch. | Boolundshuhur Branch. |  |  |  |  |  |  |  |
| From commencemen up to 31st July 1854. | Rs. As. P. | Rs. As. P. | Re. As. P. | Rs. As. P. | Rs. As. P. | Rs. A. P. | Rs. $\mathbf{\Delta}$. P. | Rs. A. P. | Rs. A. P. | Rs. A. P. | Rs. A. P. | Rs. A. P. |
| Northern, including | 74,22,212 1411 |  | ..... |  |  | 1,14,476 66 |  | 8,280 150 | 33,112 2 b | 39,125 144 | 4,99,906 104 | 81,17,114 15 6 |
| Upper Central ... | 11,08,570 410 |  | ..... | 2,433 12 |  | 20,932 14 3 | ... | 14,273 $15 \quad 2$ | 1,084 18 | 2,973 225 | 71,648 15 3 | 12,21,917 010 |
| Lower " ... | 18,15,551 118 | ... | ..... | ... $\{$ | $\left.\begin{array}{ll}23,332 & 12 \\ \text { Koel } & 11\end{array}\right\}$ <br> Koel Branch. <br> 1,463 | 35,591 42 | 37,743 120 | 7,475 9 9 5 | 2,092 4 9 | 3,473 1210 | 1,66,903 211 | 20,93,627 08 |
| Cawnpoor ... ... | ...... | 14,68,647 46 |  | ...... |  | 6,271 8 8 0 | ... | 6,628 410 | 3,740 $10 \quad 2$ | 2,194 1311 | 2,01,082 311 | 16,88,564 134 |
| Etawah ... ... | ...... |  | 7,12,090 12 | ...... | ...... | 3,057 | ... | 6,342 $10 \quad 9$ | 1,505 4 | 1,535 9 | 90,067 114 | 8,13,599 910 |
| Superintendent Ge- | ... | ..... |  |  | ...... |  |  |  |  |  | 2,48,323 512 | 2,48,323 512 |
| $\begin{aligned} & \text { Total up to 31st } \\ & \text { July } 1854 \ldots \end{aligned}$ | 1,03,46,334 47 | 14,68,647 46 | 7,12,090 125 | 2,433 121 | 24,796 011 | 1,80,329 911 | 37,743 120 | 42,001 7 | 41,534 79 | 49,303 5 | 12,77,932 011 | 1,41,83,146 134 |
| From 1st Augt. 1854 to 30th April 1863 |  |  |  |  |  |  |  |  |  |  |  |  |
| Northern, including navigation naterials | 7,48,542 11 | ...... | ...... | ..... | ...... | 8,57,831 810 | 4,40,232 127 | 28,774 131 | ..... | $\left\{\begin{array}{rrrr}27,020 & 3 & 10 \\ 4,906 & 7 & 10\end{array}\right\}$ | 4,65,551 311 | 25,72,859 135 |
| Upper Central ... | 41,971211 | ..... | ..... | 6,82,572 56 |  | 1,94,851 136 | 6,15,065 887 | $\begin{array}{llll}15,267 & 3 & 9\end{array}$ | .... | 15,140 $13 \quad 5$ | 1,94,457 901 | 17,59,326 811 |
| Lower " ... | 71,976 120 | ..... | - ..... | $\ldots \ldots$ | $\left.\begin{array}{l} 81,6384 \\ \text { Koel Branch. } \end{array}\right\}$ $216 \quad 42$ | $\begin{array}{lll} 3,14,644 & 2 & 8 \end{array}$ | $11,38,745118$ | $47,238 \quad 4 \quad 3$ | ...... | $\left\{\begin{array}{rrr} 47,413 & 6 & 9 \\ 4,152 & 13 & 7 \end{array}\right\}$ | $\begin{array}{llll}3,48,668 & 2 & 7\end{array}$ | 20,61,351 124 |
| Cawnpoor ... | ...... | 3,19,114 158 |  |  |  | 2,17,534 714 | 5,55,960 910 | 28,020 13 3 | ..... | 16,356 | 2,58,465 771 | 13,95,402 126 |
| Etawah ... ... | ...... | ...... | $\left.\begin{array}{ccc}4,90,319 & 1 & 3 \\ 6,657 & 13 & 11\end{array}\right\}$ |  | ... | 1,59,105 36 | 3,93,510 80 | 26,527 2 | ..... | 15,229 46 | 2,62,154 665 | 13,46,845 1011 |
| Superintendent General's Office | ...... |  |  |  |  |  |  |  |  |  | 3,52,609 70 | 3,52,609 70 |
| Totals | 8,62,490 $10 \quad 3$ | 3,19,114 158 | 4,96,976, $15 \quad 2$ | 6,82,572 58 | 81,854 811 | 17,43,967 310 | 31,43,515 3 | 1,45,828 410 |  | 1,30,219 9 | 18,81,906 4 | 94,88,446 1 |
| Grand Totals .. | $\overline{1,12,08,8241410}$ | 17,87,762 42 | 12,09,067 117 | 6,85,006 | 1,06,650 910 | 19,24,296 13 9 ${ }^{31}$ | 31,81,258 $15 \begin{array}{ll}5 & 5\end{array}$ | 1,87,829 12 0\| | 41,584 7 7 9 | 1,79,522 14 | 31,59,838 5 5 | 2,36,71,592 14 |

( 128 )
APPENDIX H.
Statement of land required for projected lines, Ganges Canal.

| Description. |  |  |  | Length in Feet. | Width in Feet. | Total in Acres. | Grand Total, Acres. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Alternative Line. |  |  |  |  |  |  |  |
| From the head at Roorkee to the Junction Branch Head | ... | $\cdots$ | $\ldots$ | 322,080 | 600 | 4,436 |  |
| For the Junction Line | ... | ... | ... | 24,630 | 500 | 282 |  |
| Junction Branch Head to Boolundshuhur Branch Head ... | ... | ... | ... | 205,920 | 500 | 2,363 |  |
| Boolundshuhur Branch Head to Nanoon ... | ... | ... | ... | 353,760 | 400 | 3,248 |  |
| Escape No. 1 ... | ... | ... | ... | 6,000 | 300 | 41 |  |
| Escape No. 2 ... | ... | ... | ... | 53,800 | 200 | 247 |  |
| 1st Class Chokies (4 in number) | ... | ... | $\ldots$ | 300 | 300 | 8 |  |
| Separate Natigation Chansel ... ... | ... | ... | ... | 867,920 | 220 | 4,383 |  |
| Natigation Channel along Cawnpoor Branch | ... | ... | ... | 892,320 | 220 | 4,507 | 4,507 |

REMODELLING.
GENERAL ABSTRACT OF ESTIMATES.


| No. or |  | No. of еасн. | Description. |  | Cost of eace. | Total | Dee to |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Plan. | Estimate. |  |  |  |  |  | Irrigation. | Navigation. |
| $\begin{aligned} & \text { IV. } \\ & \text { X. } \end{aligned}$ |  |  | FROM ROORKEE TO NANOON. |  |  |  |  |  |
|  | 15 |  | Excavation | .. | ... | 8,00,094.75 | 7,93,994.75 | 16,100.00 |
|  | 15 | ... | Fall 4.07 at Roorkee ... |  |  | 71,753.61 | 42,860.25 | 28,893.36 |
|  | 16 | 3 | Additions, 8c., to Guneshpoor, Liburheree, and Dimat Bridges |  | 13,768•83 | 41,306•49 | 39,519.69 | 1,786.80 |
| 10 | 17 | $\cdots$ | Alterations, Hafiznugger Falls ... ... |  | ... | 40,447.77 | 37,563:53 | 2,884-24 |
| 12 | 18 | ... | Ditto, Munglour Bridge .. ... |  |  | 16,369.57 | 15,773.97 | ${ }^{5} 959.60$ |
| ${ }_{9}^{12}$ | 19 |  |  |  |  | 75,165.61 | 43,215.71 | 31,949.90 |
|  | 20 | 4 | Additions, \&c., to Mundowlee, Tooghulpoor, Bailra, and Bhopa Bridges |  | 12,295•58 | 49,182:32 | 46,799.92 | $\stackrel{2,382 \cdot 40}{384.24}$ |
| ${ }_{12}$ | ${ }_{22}^{21}$ | $\cdots$ | Fall 8.54 at at Toghulpoor Bridge | $\ldots$ | ... | 77,174:01 | 44,854•01 | 32,320.00 |
| 10 | 23 | $\ldots$ | Additions, \&c., to Nirgajnee Falls ... |  | ... | 41,755•49 | 39,989•49 | 1,766.00 |
| 10 | 24 | $\ldots$ | Ditto Jaoli ditto ... |  | ... | 39,751.09 | 37,985.09 | 1,766.00 |
| 13 | 25 | $\ldots$ | Ditto Futtygurh branch head... |  | ... | 29,308.67 | 9,872.90 | 19,435.77 |
| 14 | 26 |  | Dukheri Bridge and Fall 3.53 ... |  |  | 67,20491 | 37,715.95 | 29,488.96 |
|  | 27 | 4 | Alteratious to Nugla, Serai, Sutheree, and Aternah Bridges | .. | 2,409•63 | 9,638-52 | 7,256-12 | 2,382•40 |
|  | 28 | 2 | Ditto to Sirdhanna and Khutowlie Bridges |  | 984:00 | 1,968.00 | $422 \cdot 40$ | 1,545.60 |
| 10 | 29 30 | 3 | Ditto to Chitowra Falls Alterations to ceseapes at Khutoulie Aboo |  | $259 \cdot 20$ | $42,881 \cdot 16$ $777 \cdot 60$ | $41,453 \cdot 16$ $777 \cdot 60$ | 1,408:00 |
|  | 31 | 2 | Falls at Khutowlie and Sirdhanna ... ... |  | 49,464:38 | 98,928•66 | 39,950.74 | 58,977.92 |
| 16 | 32 | 2 | Additions, \&c., to Sulawur and Mussoorie Falls | .. | 41,484.52 | 82,969.04 | 78,431.04 | 4,538.00 |
|  | 33 | 2 | Ditto to Nanoon and Moradnugger Bridges |  | 17,728.46 | 35,456.92 | 2,242•72 | 33,214-20 |
|  | 34 | 4 | Ditto to Jutpoora, Newari, Aboopoor, and Noorpoor Bridges |  | 16,031.05 | 64,124-20 | 3,302.28 | 60,821.92 |
|  | 35 |  | Ditto to Sounda Bridge ... ... | $\ldots$ |  | 15,205.25 |  | 15,205-25 |
|  | ${ }_{37}^{36}$ | 2 | Ditto to Pooth and Nugla Bridges ... ... | . | 16,526.33 | 33,052.66 | 1,804.28 | 31,248.38 |
| 16 | 37 38 | $\ldots$ | Ditto to Bhola Falls <br> Ditto to Janni Khoord Bridge ... ... ... | $\ldots$ | ... | $\begin{aligned} & 32,070 \cdot 20 \\ & 15,623 \cdot 96 \end{aligned}$ | 28,617•88 | $3,452 \cdot 32$ 15,623 |
|  | 38 39 | 5 | Ditto to Raoli, Jarcha, Pukkana, Mamun, and Suhenda Bridges |  | 14,414•86 | $72,071 \cdot 80$ | 4,429.15 | 67,642 65 |
|  | 40 |  | Ditto to Peepulherēe Bridge $\ldots$ | . |  | 14,860.43 | ${ }^{934.77}$ | 13,925•66 |
| 13 | 41 |  | Additions, \&c., to Boolundshuhur Branch Head ... | ... |  | ${ }_{41042}^{23,39208}$ | 7,271.45 | 16,120.63 |
|  | 42 | 3 | Ditto to Nidhaoli, Geesoopoor, and Sunowta Bridges ... | ... | 13,680.72 | 41,042.16 | 4.57:20 | 40,584:96 |


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|  | $\vdots$ | $\vdots$ | $\vdots$ |
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|  | －蜀 |  |
| \％ | 滀皆 |  |
| \％ | 家 |  |

（ 133 ）

|  <br>  <br>  ポがべ |  | $\begin{aligned} & \infty \\ & \underset{\sim}{\infty} \\ & \dot{\infty} \\ & \underset{\sim}{\infty} \\ & \infty \\ & \infty \\ & \infty \end{aligned}$ |  |  |  |  |  | No $i 0$ 0 0 0 0 0 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\vdots$ | ： | ！ | ！ | 3，02，570．75 | $\dot{\prime}$ $\dot{0}$ 0 0 0 0 0 0 | $\vdots$ | ¢ |
|  <br>  <br>  ぶがい |  | $\begin{aligned} & \infty \\ & \infty \\ & \dot{\infty} \\ & \infty \\ & \infty \\ & \infty \\ & \infty \\ & \infty \\ & \infty \end{aligned}$ |  | $$ |  |  |  | $\begin{aligned} & \infty \\ & \stackrel{\infty}{\dot{\infty}} \\ & \dot{\circ} \\ & \underset{\sim}{0} \\ & \underset{\sim}{1} \end{aligned}$ |
|  | ： |  | ： | ： | ！ | ： | ！ | ： |
|  | ！ |  | ： | ！ | ！ | ： | ： | ！ |
|  |  |  |  | $\vdots$ ¢ ¢ ¢ |  |  | Ditto Cawnpoor Branch |  |
|  |  |  |  |  |  | 2 |  |  |
|  |  |  |  |  |  |  |  |  |

( 134 )
ALTERNATIVE LINE.


|  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
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|  |  |  |  |  |  | $\mathrm{B}_{\mathrm{B}}^{\dot{\circ}}=2=2=2$ $\text { L } 2$ |  |

ALTERNATIVE LINE.
General Abstract of Estimates,- cont


navigable line.
general abstract of estimates.


( 140 )
MISCELLANEOUS ESTIMATES.


## ( 141 ) <br> CLASSIFICATION OF COST.

alternative line.

| Description. | Stction No. 1. | Remainder. | Total. |
| :---: | :---: | :---: | :---: |
| Earth-work ... ... | 19,07,956.56 | 10,95,925•46 | 30,03,882.02 |
| Falls and regulating bridges ... | 13,99,275-22 | 4,26,945•67 | 18,26,220•89 |
| .Bridges ... | 2,19,662 54 | 3,82,959•87 | 6,02,622-41 |
| Drainage works, aqueducts, culverts, \&c. .. .. ... | 4,63,245•09 | 52,248•10 | 5,15,493•19 |
| Irrigation outlets, \&c., | 2,821.56 | 32,859-41 | 35,680.97 |
| Accommodation buildings | 22,249.65 | 17,523.03 | 39,772.68 |
|  | 40,15,210.62 | 20,08,661 54 | 60,23,672•16 |
| Contingencies | ... | ... | 4,81,893.75 |
| Establishment | ... | ... | 4,55,389 59 |
|  | Grand Total | upees ... | 69,60,955•50 |

sEPARATE NAVIGABLE LINE.

| Drscription. |  |  |  |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Earth-work ... ... ... ... ... |  |  |  |  |  | 7,91,859•23 |
| Lockage, including level chambers ... ... ... |  |  |  |  |  | 8,13,204•82 |
| Bridges ... ... ... ... ... |  |  |  |  | ... | 3,65,864•78 |
| Drainage works, aqueducts, culverts, syphons, \&c. |  |  |  | ... | ... | 1,83,146.08 |
| Irrigation culverts ... ... ... ... |  |  |  |  |  | 31,539•80 |
| Accommodation buildings ... |  |  | ... | ... | ... | 11,625.92 |
|  |  |  | Total |  |  | 21,97,240.63 |
| Contingencies ... ... ... |  |  |  |  |  | 1,75,779•25 |
| Establishment |  |  | ... | ... | ... | 1,66,111 39 |
|  |  |  | Grand Total Rupees |  |  | 25,39,131 $\cdot 27$ |

## ( 142 ) <br> NOTES TO THE ESTIMATES.

All measurements are in English feet and decimals.
Cost in Rupees and decimals.
The cost of overfalls at the termination of the Khutowlee, Aboo Nulla, and Jannee Khoord Escapes on the main line of the Ganges Canal, have been omitted in the estimate for remodelling. They formed part of the original projection, but I was not aware, until these estimates had been completed, that they had not yet been built. As they will ultimately be necessary to the efficiency of the Escapes, their probable cost-Rs. 47,500 each, or Rs. $1,42,500$ in the aggregate-should be added to the estimate.
J. CROFTON, Captain, R. E.

## PUBLIC WORKS DEPARTMENT.

## REPORT

on

## THE GANGES CANAL

By
CAPT. J. CROFTON, R. E.,

WITH
ESTIMATES AND PLANS.

IN THREE VOLUMES.

VOL. II.-ESTIMATES.
$\mathbb{C}$ alcutta:
PUBLIC WORKS DEPARTMENT, 1865.

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Page.Remodelling Estimates, Nos. 1 to 9795
Navigable Line, Nos. 147 to 170 ..... 115
Miscellaneous Estimates, Nos. 171 to 181 ..... 125

## GANGES CANAL.

REMODELLING ESTIMATES.

ESTIMATE No. 1.-EXCAVATION.

| No. of Mme. | L. | B. D. | Contents. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Area. |  | 13,320,787 |  |
| 1 | 5,280 | 40 | 211,200 |  |  |
| 2 | 4,045 | 40 | 161,800 |  |  |
| " | 300 | 776 | 232,800 |  |  |
| " | 638 | 1,779 | 1,126,107 |  |  |
| 3 | 5,280 | 990 | 5,227,200 |  |  |
| 4 | 4,680 | 626 | 2,929,680 |  |  |
| 5 | 5,280 | 650 | 3,432,000 |  |  |
| 6 | 248 | 650 | 161,200 |  |  |
|  | 4,232 | 441 | 1,866,312 |  |  |
| 7 | 4,711 |  | 2,077,551 |  |  |
| 8 | 1,896 | 379 | 718,584 |  |  |
| " | 2,353 | 234 | 550,602 |  |  |
| ${ }^{9}$ | 5,280 | 142 | 749,760 |  |  |
| 10 | 3,530 | 230 | 811,900 |  |  |
| " | 400 | $500$ | 200,000 |  |  |
| " | 950 | $1,210$ | 1,149,500 |  |  |
| 11 | 5,280 | 1,522 | 8,036,160 | 8,285,409 |  |
| 12 | 5,280 | 755 | 3,986,400 |  |  |
| 13 | 3,879 | 284 | 1,101,636 |  |  |
| " | 801 | . 84 | 67,284 |  |  |
| 14 | 28 | 84 | 2,352 |  |  |
| "15 | 4,952 | 375 | 1,857,000 |  |  |
| " | 300 B | 66 | 19,800 |  |  |
|  | 4,680 | 75 | . 351,000 |  |  |
| 19 | 1,072 | $849$ | -910,128 |  |  |
| " | 2,028 |  | 926,796 | 17,366,256 |  |
| 20 | 3,252 | 1,125 | 3,658,500 |  |  |
| 21 | 600 | 159 | 95,400 |  |  |
|  | 4,680 | 1,185 | 5,311,800 |  |  |
| 22 | 5,280 | 901 | 4,757,280 |  |  |
| 23 | 3,490 | 901 | 3,144,490 |  |  |
|  | 990 | 1,215 | 1,202,850 |  |  |
| 24 | 5,280 | 1,197 | 6,320,160 |  |  |
| 25 | 4,680 | 752 | 3,519,360 | 28,009,840 |  |
| 26 | 5,280 | $784\}$ | 5,008,976 |  |  |
| 26 27 | 1,109 | $784\}$ |  |  |  |
| " | 300 |  | 166,200 |  |  |
| 28 | 3,571 | 2,557 | 9,181,047 |  |  |
| 28 | 5,280 300 | 1,999 | $10,554,720$ 76,800 |  |  |
| \% | 300 | 156 | 46,800 |  |  |
| 3 | 4,680 | 1,231 | 5,761,080 | 34,916,823 |  |
| 30 | 5,280. | 790 | 4,171,200 |  |  |
| 3132 | 5,280 | 1,085 | 5,724,800 |  |  |
|  | 194 | 1,08587 | 210,490 |  |  |
| " | 400 |  | 34,800 |  |  |
|  | 4,286 | 87 | 3,047,346 |  |  |
| 33 | 5,280 | 658 | 3,474,240 |  |  |
| 34 | 4,680 | 751 | 3,514,680 |  |  |
| 35 | 5,280 | 1,596 | 8,426,880 | 24,437,236 |  |
|  |  | Carried over | ... | 126,336,351 |  |

400-88-1-65.
( 2 )
estimate no. 1--EXCAVATION,-continued.


ESTIMATE No. 1.-EXCAVATION,-continued.

(4)

ESTIMATE No. 1.-EXCAVATION,-continued.

| No. of Mile. | L. | B. D. | Contents. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Brought forward ... <br> Area. | ... | 337,809,034 |  |
| 106 | 2,712 | 230 | 623,760 |  |  |
| „ | 400 | 169 | 67,600 |  |  |
| " | 1,768 | 502 | 887,536 |  |  |
| 107 | 300 | 169 | 50,700 |  |  |
| " | 4,680 | 730 | 3,416,400 |  |  |
| \% | 300 5 | 96 | 28,800 |  |  |
| 108 | 5,280 | 440 | 2,323,200 |  |  |
| 109 | 4,680 | 306 | 1,432,080 |  |  |
| 110 | 5,280 | 404 | 2,133,120 |  |  |
| 111 | 300 | 137 | 41,100 | 10,063,196 |  |
| " | 1,446 | 404 | 584,184 |  |  |
| " | 3,234 | 21 | 67,914 |  |  |
| 112 | 5,280 | 21 | 110,880 |  |  |
| 113 | 5,280 | 21 | 110,880 |  |  |
| 114 | 4,680 | 21 | 98,280 |  |  |
| 115 | 5,280 | 21 | 110,880 |  |  |
| 116 | 5,280 | 21 | 110,880 |  |  |
| 117 | 3,038 | 21 | 63,798 |  |  |
|  | 1,642 | 214 | 351,388 |  |  |
| 118 | 5,280 | 214 | 1,129,920 |  |  |
| 119 | 5,280 | 214 | 1,129,920 |  |  |
| 120 | 4,680 | 214 | 1,001,520 |  |  |
| 121 | 5,280 | 214 | 1,129,920 | 3,787 |  |
| 122 | 5,280 | 214 | 1,129,920 |  |  |
| 123 | 4,680 | 214 | 1,001,520 |  |  |
| 124 | 5,280 | 214 | 1,29,9120 |  |  |
| 125 | 5,280 | 214 | 1,129,920 |  |  |
| 126 | 2,927 | 214 | 626,378 |  |  |
|  | 1,753 | 131 | 229,643 |  |  |
| 127 | 5,280 | 131 | 691,680 |  |  |
| 128 | 5,280 | 131 | 691,680 |  |  |
| 129 | 4,680 | 131 | 613,080 |  |  |
| 130 | 5,280 | 131 | 691,680 | 3,544,141 |  |
| 181 | 5,280 | 131 | 691,680 | 3,51,111 |  |
| 132 | 4,680 | 181 | 613,080 |  |  |
| 133 | 5,280 | 131 | 691,680 |  |  |
| 134 | 3,772 | 131 | 494,132 |  |  |
|  | 908 | 269 | 244,252 |  |  |
| 135 | 5,280 | 267 | 1,409,760 |  |  |
| 136 | 5,280 | 813 | 1,652,640 | 4,144,584 |  |
| 137 | 5,280 | 262 | 1,383,360 |  |  |
| 138 | 4,680 | 245 | 1,146,600 |  |  |
| 139 | 5,280 | 245 | 1,293,600 |  |  |
| 140 | 4,680 | 245 | 1,146,600 |  |  |
| 141 | 5,280 | 245 | 1,293,600 | 6,622,000 |  |
| 142 | 5,280 | 245 | 1,293,600 |  |  |
| 143 | 212 | 245 | 51,940 |  |  |
| " | 300 | 330 | -99,000 |  |  |
| " | 4,468 | 612 | 2,734,416 |  |  |
|  |  | Carried over ... | 5,472,556 | 373,516,499 |  |

(5)

ESTIMATE No. 1.-EXCAVATTON,-continued.

| No. or Mile. | L. | B. D. | Contents. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Brought forward ... | 5,472,556 | 373,516,499 |  |
|  |  | Area. |  |  |  |
| 144 | 5,280 | 536 | 2,830,080 |  |  |
| 145 | 300 | 260 | 78,000 |  |  |
| " | 4,680 | 805 | 3,767,400 |  |  |
| " | 300 | 290 | 87,000 |  |  |
| 146 | 5,280 | 481 | 2,539,680 | 12,205,036 |  |
| 147 | 5,280 | 415 | 2,191,200 |  |  |
| 148 | 5,280 | 398 | 2,101,440 |  |  |
| 149 | 3,461 | 398 | 1,377,478 |  |  |
| " | 400 | 106 | 42,800 |  |  |
| \% | 400 1019 | 197 | 78,800 |  |  |
| $\cdots$ | 1,019 | 535 | 54.5,165 |  |  |
| 150 | 5,280 | 463 | 2,444,640 | ,321,203 |  |
| 151 | 5,280 | 458 | 2,418,240 | ,321,20 |  |
| 152 | 4,724 | 458 | 2,163,592 |  |  |
|  | ${ }_{5}^{300}$ | 98 | 29.400 |  |  |
| 153 | 5,236 | 223 | 1,167,628 |  |  |
| 154 | 5,280 | 22.3 | 1,177,440 |  |  |
| 155 | 5,280 | 223 | 1,177,440 | 8,133,740 |  |
| 156 | 4,680 | 223 | 1,043,640 | 8,183,7 |  |
| 157 | 5,280 | 223 | 1,177,440 |  |  |
| 158 | 5,280 | 223 | 1,177,440 |  |  |
| 159 | 363 | 223 | 80,949 |  |  |
| " | 300 4,317 | 804 | 241,200 |  |  |
| $1{ }^{\prime \prime} 0$ | 4,317 5,280 | 900 | 3,885,300 |  |  |
| 160 | 5,280 | 260 | 1,372,800 | $8,978,769$ |  |
| 161 | 5,280 | 217 | 1,145,760 | 8,078,760 |  |
| 162 | 4,680 | 217 | 1,015,560 |  |  |
| 163 | 5,280 | 217 | 1,145,760 |  |  |
| 164 | 1,557 | 217 | -337,869 |  |  |
|  | 2,923 | 141 | 412,143 |  |  |
| 165 | 5,280 | 141 | 744,480 |  |  |
| 166 | 5,280 | 141 | 744,480 | 4,801,572 |  |
| 167 | 2,053 | 141 | 289,473 |  |  |
| , | 300 | 71 | 21,300 |  |  |
| " | 2,627 | 203 | 533,281 |  |  |
| 168 | 5,280 | 277 | 1,462,560 |  |  |
| 169 | 5,280 | 170 | 1,497,600 |  |  |
| 170 | 5,280 | 251 | 1,325,280 | $5,273,974$ |  |
| 171 | 300 | 120 | 36,000 | 5,273,84 |  |
|  | 4,680 | 174 | 814,320 |  |  |
| 172 | 5,280 | 176 | 929,280 |  |  |
| 173 | 5,280 | 181 | 955,680 |  |  |
| 174 | 4,680 | 186 | 870,480 |  |  |
| 175 | 5,280 | 191 | 1,008,480 |  |  |
| 176 | 2,058 | 191 |  | ,61 |  |
| " | 300 2062 | 120 | $\begin{array}{r} 393,078 \\ 36,000 \end{array}$ |  |  |
|  | 2,622 5 | 296 | 776,112 |  |  |
| 178 | 5,280 300 | 236 42 | 1,246,080 |  |  |
|  |  | 42 | 12,600 |  |  |
|  |  | Carried over ... | 2,463,870 | 428,875,033 |  |

$$
(6)
$$

ESTIMATE No. 1.-EXCAVATION,-continued.


Cawnpoor Branch.

| 1 to 3. | 14,624 | 80 .94 | 1,099,725 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 165 to 168 | 14,280 | Area. 342 | 4,883,760 |  |  |
| Through ... | 2,420 | 158 | 382,360 |  |  |
| Cawnpoor ... | 850 | 18 | 15,300 |  |  |
|  |  |  | $\cdots$ | 6,381,145 | 6,381,145 |

Slopes Protected.


ABSTRACT OF COST OF ESTIMATE No. 1.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 38,972,452 | Excavation from head to Roorkee, at Re. ${ }^{2}$ per 1,000 | $\begin{gathered} \text { Rs. } \\ 77,944 \cdot 90 \end{gathered}$ |
| " | 396,807,373 | ,, Roorkee to Cawnpoor Branch head, at Rs. 2 per 1,000 | 7,93,614.75 |
| S. feet | 1,296,000 | Protecting slopes head, at 5 annas per 100 ... | 6,480.00 |
| Cubic feet | 6,381,145 | Excavation in Carvnpoor Branch, at Re. ${ }^{2}$ per 1,000. | 12,762.29 |
| Miles | 162 | Repairing Tow-path ditto, at Rs. 70 per mile ... | 11,340 |
|  |  | Grand Total | 912,141.94 |

ESTIMATE No. 2.-ALTERATIONS TO MYAPOOR REGULATOR.


ABSTRACT OF ESTIMATE No. 2.


ESTIMATE No. 3.-AITERATIONS AT KUNKHUL BRIDGE.


## ( 9 )

ESTIMATE No. 3.-ALTERATIONS AT KUNKHUL BRIDGE,-continued.
Lock Channel, \&c.


## ( 10 )

ABSTRACT OF ESTIMATE No. 3.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 10,145 | Demolition, masonry, at Re. 1 per $100 \ldots$... ... | $101 \cdot 45$ |
| " | 13,550 | Digging out cribs and boulders, at Rs. 7 per 1,000 ... | 94.85 |
| " | 4,206 | Boulder masonry, at Rs. 14 per 100 ... ... ... | $588 \cdot 84$ |
| " | 3,866 | Brick-work, at Rs. 22 per 100 ... ... ... ... | $850 \cdot 52$ |
|  | 16,100 | Boulder flooring dry, at Rs. 7 per $100 \ldots$... ... | 1,127.70 |
| Lineal feet | 179 | $10^{\prime}$ Piles, including driving, at Rs. $2 \cdot 5$ per foot ... | $447 \cdot 50$ |
| Cubic feet | 168 | Sal wood in grating supports, at Rs. 3 " ... ... | 504.00 |
| Number | 144 | $10^{\prime}$ Kurries, at Re. 1.5 each ... ... ... ... | 216.00 |
| Cubic feet | 3,900 | Fixing and filling old cribs, at Rs. 7 per 1,000 ... | 27.30 |
|  | 2,805 | Brick-on-edge, at Rs. 25 per 1,000 ... ... ... | 701.25 |
| " | 495 | Stone-work, at Rs. 2 per foot ... ... ... ... | 990.00 |
|  |  | Total for Bridge, \&ta | 5,649-41 |
|  |  | Lock Channel, \&c. |  |
| Cubic feet | 1,288,710 | Excavation, at Rs. $2 \cdot 25$ per 1,000 ... ... ... | 2,899.60 |
|  | 132,899 | Boulder masonry, at Rs. 14 per 100 ... ... ... | 18,605•66 |
| " | 11,000 | Brick-work, at Rs. 22 per $100 \ldots$.... ... ... | 2,420.00 |
| " | 2,736 | Brick-on-edge, at Rs. 25 per 100 ... ... ... | 684.00 |
| " | 1,260 | Arch-work, at Rs. 28 per 100 ... ... ... ... | $352 \cdot 80$ |
| " | 1,200 | Metalling, at Rs. 6 per 100 ... ... ... .' ... | 72.00 |
|  |  | Lock Apparatus. |  |
| Square feet Lineal feet Number | 439 | Lock gates, at Rs. 6.5 per foot ... ... ... | 2,853.50 |
|  | 106 | Chain, at Re. 1 per foot ... ... ... | 106.00 |
|  | 4 | Pulleys, at Rs. 5 each ... ... ... ... | 20.00 |
| " |  | Crabs, „ 135 „ ... | 540.00 |
|  | 4 | Sluices, „ 125 „ ... ... ... ... ... | $500 \cdot 00$ |
|  |  | Total, Lock Cannnel, \&c. | 29,053.76 |
|  |  | Grand Total, Bridge and Lock Channel | 34,703-17 |

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\text { ( } 11 \text { ) }
$$

ESTIMATE No. 4.-ALTERATIONS AT JOWALAPOOR BRIDGE.


ABSTRACT OF ESTIMATE No. 4.

| Quantity. |  | Description. |  | Total. |
| :---: | :---: | :---: | :---: | :---: |
|  | 7862 |  |  | Rs. |
| Cabic feet |  | Demolition old masonry, at Re. 1 per 100 | ... ... | 78.62 |
| " | 4,045 | Digging out boulders, at Rs. 7 per 1,000 | ... ... | 28.31 |
| " | 5,198 | Boulder masonry, at Rs. 14 per 100 ... | ... ... | $727 \cdot 72$ |
| " | 1,733 | Brick-on-edge, at Rs. 25 per 100 ... | ... ... | $438 \cdot 25$ |
|  |  | Tow-path, vide Estimate No. 173 | ... ... | $595 \cdot 60$ |
|  |  | Grand Total |  | 1,863 50 |

ESTIMATE No. 5.-ALTERATIONS, RAMPOOR WORKS.


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\text { ( } 13 \text { ) }
$$

ABSTRACT OF ESTIMATE No. 5.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 35,819 | Demolition, old masonry, at Re. 1 per 100 ... ... | $358 \cdot 19$ |
| " | 16,240 | Digging boulders out of crib work, at Rs. 7 per 1,000... | 113.68 |
| " | 39,483 | Brick-work, at Rs. 22 per 100 ... ... ... ... | 8,686.26 |
| " | 12,704 | Boulder masonry, at Rs. 14 per 100 ... ... ... | 1,778.56 |
| " | 2,032 | Arch-work, light, at Rs 28 per 100 ... ... ... | 568.96 |
| " | 40,500 | Embankmeni, at Rs. 2 per 1,000 ... ... ... | 81.00 |
| " | 4,500 | Stone-work in flooring, at Rs. 2 per foot ... ... | 9,000.00 |
| " | 113 | Sâl wood in grating supports, at Rs. 3 per feet ... | $339 \cdot 00$ |
| Number | 128 | Kurries 14' long, at Re. 1-5 each ... ... ... | 192.00 |
| Square feet | 136.5 | Sluice gates, at Rs. 1.25 per feet ... | $170 \cdot 62$ |
| " | $667 \cdot 8$ | Lock gates, at Rs. 6.5 per foot ... . ... ... ... | 4,340.70 |
| Number | 14 | Sluices in lock gates, at Rs. 125 each ... ... ... | 500.00 |
| " | 1 | Windlasses, at Rs. 22 each ... ... ... ... | 22.00 |
| " | 2 | Axle blocks, \# 5 , ... ... ... ... | 10.00 |
| Lineal feet | 22 | Chain, at Rs. 0.9 per feet ... ... ... ... | $19 \cdot 80$ |
|  |  | Grand Total | 26,180.77 |

## ( 14 )

ESTIMATE No. 6.-ALTERATIONS, BAHADOORABAD FALL, No. 1.


## ( 15 )

ABSTRACT OF ESTIMATE No. 6.


ESTIMATE No. 7.-ALTERATIONS, BAHADOORABAD FALL, No. 2

| Description |  | No. | L. | B. | D. | Quantity | Total. | Grand Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Earth-work. |  |  |  |  |  |  |  |  |
| Approaches ... |  | 2 | $\begin{array}{r} 400 \\ 200 \end{array}$ | $\begin{array}{\|l} 30 \\ 30 \end{array}$ | 3 2 | $\begin{aligned} & 72,000 \\ & 24,000 \end{aligned}$ | 96,000 |  |
| Demolition. |  |  |  |  |  |  |  |  |
| Arches ... ... | ... | 8 | 25 | 18 | 2.93 | 10,548 |  |  |
| Parapet ... ... | ... | 12 | 25 | 1.50 | 2 | 900 |  |  |
| Pillar ... ... | $\ldots$ | 4 | 3 | 3 | 2 | 72 |  |  |
| Stairs ... ... | $\ldots$ | 3 | 2 | 4.50 | $3 \cdot 33$ | 900 |  |  |
| In Ogee... ... | ... | 4 | 54.50 | 10 | 2 | 4,360 |  |  |
| Flooring... ... | ... | 4 | 54,50 | 40 | 1 | 8,720 | 25,500 |  |
| New Masonry. |  |  |  |  |  |  |  |  |
| Lengthening piers | ... | 4 | 35.25 | 4.50 | 23 | 14,593.50 |  |  |
| " " | ... | 4 | 10 | 4.50 4.50 | ${ }_{11}^{2 \cdot 36}$ | 4,164.80 |  |  |
| " | ... | 5 | 33 10 | 4.50 $4 \cdot 50$ | $\begin{aligned} & 11 \\ & 2 \cdot 36 \end{aligned}$ | $8,167 \cdot 50$ $531 \cdot 00$ |  |  |
| Stairs" | $\ldots$ | 5 | 20 | $4 \cdot 50$ 4.50 | $2 \cdot 36$ | r $1,500 \cdot 00$ |  |  |
| Arches and spandrils | ... | 8 | 25 | 18 | 2.93 | 10,548.00 |  |  |
| Piers and abutments | ... |  | 22 | 4.50 | $5 \cdot 86$ | 5,221-26 |  |  |
| Parapet ... ... | $\cdots$ | 12 | 25 | 1.50 | 2 | $900 \cdot 00$ |  |  |
| Pillars ... ... | ... | 4 | 3 | 3 | 2 | 72.00 |  |  |
| , . ... ... | ... |  | 4 |  | 5•86 | 375.04 |  |  |
| New piers ... | ... | 8 | 34 | 3 | 23 | 18,768.00 |  |  |
| Weir ... ... | $\cdots$ | 8 | 25 | 5 | 6 | 6,000.00 |  |  |
| Raising water-wings | ... | 2 | 28 | 4.50 | 7 | 1,764.00 | 68,865 10 |  |
| $\begin{gathered} \text { Deduct- } \\ \text { Arching ... } \quad . . \end{gathered}$ | ... | ... | ... | ... | ... | '..... | 8,136 | 60 |
| Arch-work | . | 5 | 27 16 | ${ }^{18} 4.50$ | $\begin{aligned} & 2 \\ & 1 \end{aligned}$ | $\begin{array}{r} 7,776 \\ 360 \end{array}$ |  |  |
| Excavating boulders out of crib- |  |  | 227 | 80 | 1.4 | 25,424 | 25,424 |  |
| Grating and stone-work as in Estimate No. 6. |  |  |  |  |  |  |  |  |

ABSTRACT OF ESTIMATE No. 7.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 96,000 | Earth-work, at Rs. 2 per 1,000 | 192.00 |
| , | 25,500 | Demolition, old masonry, at Re. 1 per 100 ... ... | 255.00 |
| " | 60,729 | New masonry, at Rs. 22 per 100 ... ... | 13,360-38 |
| " | 8,136 | Arch-work, at Rs. 28 per 100... ... ... ... | 2,278.08 |
| ", | 25,424 | Digging boulders out of crib, at Rs. 7 per 100 ... ... | - 177.97 |
| " | - | Stone-work as Estimate No. 6 :.. ... ... | $13,080000$ |
| \% | . | Grating, \&c., as Estimate "6 ... ... ... | $8,636 \cdot 40$ |
|  |  | Grand Total | 37,979.83 |

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\text { ( } 17 \text { ) }
$$

ESTIMATE No. 8.-ALTERATIONS IN BAHADOORABAD LOCK CHANNEL.


ABSTRACT OF ESTIMATE No. 8.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet <br> " | 904,500 | Excavation, at Rs. $2 \cdot 25$ per 1,000 ... ... ... | 2,035•12 |
|  | 2,307 | Demolition, old masonry, at Re. 1 per 100 ... ... | 23.07 |
| " | 11,427 | New masonry, at Rs. 22 per 100 | 2,513.94 |
|  | 594 | Arch-work, at Rs. 28 per 100... | 166.32 |
| Square feet Number | $667 \cdot 8$ | Lock gates, at Rs. $6 \cdot 5$ per foot | 4,340.70 |
|  | 8 | Sluices, at Rs. 125 per each ... | 1,000.00 |
|  |  | Grand Total | 10,079•15 |

( 18 )
ESTIMATE No. 9.-ALTERATIONS AT PUTRI WORKS.


ESTIMATE No. 9.-ALTERATIONS AT PUTRI WORKS,—continued.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Boulder Flooring. | ... | 231 | 46 | 1-50 | 15,937 | 15,937 | 15,937 |
| In cement ... ... ... |  |  |  |  |  |  |  |
| Stone-voork ... ... ... | 8 | 30 | 25 | -75 | 4,500 |  |  |
|  |  |  | 46 |  |  | ... | 4,500 |
| Boulder flooring, dry, ... ... | ... | 231 |  | $2 \cdot 50$ | 26,565 | 26,565 |  |
| Timber woork. |  |  |  |  |  |  | 26,565 |
| Sal-wood in grating supports.. | 32 | $13 \cdot 50$ | -75 | - 75 | 225 |  | 225 |
| Sluice Gate ... ... ... | ... | 4 | 4 | ... | 16 | 16 |  |
| Lock Gates ... ... ... | 2 | 9.54 | $\left\{\begin{array}{l}13 \\ 22\end{array}\right.$ | ... | 915•84 |  |  |
| Chain ... ... ... ... | 2 | $27 \cdot 50$ | ... | ... | 55 |  |  |
|  |  |  |  |  |  | 55 | 55 |

ABSTRACT OF ESTIMATE No. 9.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 32,199 | Demolition, old masonry, at Re. 1 per 100 | 321.99 |
|  | 91,350 | Excavation boulders out of crib-work, at Rs. 7 per 1,000 | $639 \cdot 45$ |
| Number | 95 | Curbs, small, at Rs. 20 each ... ... ... ... | 1,880.00 |
| Cubic feet | 17,989 | Boulder masonry in flooring, at Rs. 14 per 100 ... | 2,518•46 |
|  | 84,104 | Brick-work, at Rs. 22 per $100 . . . \quad .$. | 18,502.88 |
| Lineal feet | 940 | Well-sinking, at Rs. 5 per foot ... ... .... ... | 4,700.00 |
| Cubic feet | 2,452 | Arch-work, at Rs. 26 per $100 \ldots$.... ... ... | 637.52 |
| , | 40,500 | Embankment, at Rs. 2 per 1,000 ... ... ... | 81.00 |
| " | 15,937 | Masonry flooring of boulders, at Rs. 12 per 100 ... | 1,912.68 |
| " | 4,500 | Stone flooring, at Rs. 2 per foot... ... | 9,000.00 |
|  | 26,565 | Boulder flooring, dry, at Rs. 7 per $100 . .$. ... | 1,859•55 |
| Number | 256 | 14' Kurries (Sâl), at Rs. $5 \cdot 25$ each ... ... $\ldots$ | 1,344.00 |
| Cubic feet | 225 | Sâl-wood, at Rs. 3 per foot ... ... ... ... | $675 \cdot 00$ |
| Sq. feet | 16 | Sluice gate, at Rs. $1 \cdot 25$ per foot ... ... ... | $20 \cdot 00$ |
|  | 916 | Lock gates, at Rs. 6.5 per foot ... | 5,954.00 |
| Number | 2 | Crabs, at Rs. 135 each ... ... ... ... ... | $270 \cdot 00$ |
| " | 2 | Pulleys, at Rs. 5 each ... ... ... .. ... | 10.00 |
|  | 5 | Sluices, at Rs. 125 each ... ... ... ... ... | 750.00 |
| Lineal feet | 55 | Chain, at Re. 1 per foot ... ... ... ... ... | $55 \cdot 00$ |
|  |  | Grand Total | 51,131•53 |

ESTIMATE No. 10-ALTERATIONS TO RUTMOO WORKS.


## ( 21 )

ESIMTATE No. 10.-ALTERATIONS TO RUTMOO WORKS,-continued.


ABSTRACT OF COST OF ESTIMATE No. 10.

| Quantity. |  | Description. | Cosr. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cabic feet | 74,400 | Excavation, at Rs. 2 per 1,000 | 148.80 |
| " | 5,643 | Demolition, old masonry, at Re. 1 per 100 ... | 56.43 |
| " | 23,680 | Boulder masonry, at Rs. 14 per 100 ... ... ... | 3,315.20 |
| " | 23,895 | Brick-work, at Re. 23 per 100 ... ... | 5,256.90 |
| " | 6,075 | Arch-work , 26 " $100 \ldots$... ... ... | 1,579.50 |
| " | 23,600 | Digging out cribs and boulders, at Rs. 7 per 1,000 ... | $165 \cdot 20$ |
| " |  | Tow path under Dhunowra Bridge ... ... ... | 595.60 |
| " ${ }^{\prime}$ | 236 | 12' Piles, including driving, at Rs. 5 per foot... ... | 1,180.00 |
| Lineal feet | 256 | Iron girders for sluice gates, at Rs. $9 \cdot 4$ per foot... ... | 2,406.40 |
| Cubic feet | 61,056 | Boulder flooring " dry" at Rs. 7 per 100 ... ... | 4,273.92 |
|  | 2,400 | Pitching on slopes, at Rs. 10 per 100 ... ... | $240 \cdot 00$ |
| Lineal feet | 108 | Wells, sinking, at Rs. 7 per foot ... | 756.00 |
| Square feet | 96 | Sluice gate, at Rs. $1 \cdot 25$ per foot ... | 120.00 |
| Number | 9 | Curbs, at Rs, 90 each ... ... ... ... | 810.00 |
| Lineal feet | 60 | Iron grooves, at Rs. 5 per foot ... ... ... ... | 300.00 |
| Number | 19 | Axle Blocks, at Rs. 5 each ... | 95.00 |
|  | 26 | Windlasses, at Rs. 22 each ... | $572 \cdot 00$ |
| Lineal feet | 612 | Chain, at Rs. 0.9 per foot ... | $550 \cdot 80$ |
| Square feet | 400 | Iron Drop gates, at Rs. 5 per foot | 2,000.00 |
| , | 1,519 | Wooden Drop gates . | 1,898.75 |
|  |  | Grand Total | 26,320.50 |

ESTIMATE No. 11.-ALTERATIONS AT PEERAN KULEEUR BRIDGE.

| Description of Work. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old Masonry. | 2$\cdots$ | $\begin{aligned} & 156 \\ & 156 \end{aligned}$ | $\begin{gathered} 2 \cdot 5 \\ 23 \end{gathered}$ | $\begin{gathered} 2.95 \\ 2 \end{gathered}$ | $\begin{aligned} & 2,301 \\ & 7,176 \\ & \hline \end{aligned}$ | 9,477 | 9,477 |
| $\begin{array}{llll} \text { Curtains ... } & \text {.. } & \text {... } & \text {... } \\ \text { Flooring ... } & \text {... } & \text {... } \end{array}$ |  |  |  |  |  |  |  |
| New Masonry. |  |  |  |  |  |  |  |
| Flooring... <br> Brick-on-edge. | ... | 156 | 23 | 1.5 | 5,382 | 5,382 | 5,382 |
| Flooring ... ... ... ... | $\cdots$ | 156 | 23 | $\cdot 5$ | 1,794 |  |  |
|  |  |  |  |  |  |  | 1,794 |

ABSTRACT OF COST OF ESTIMATE No. 11.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | $\begin{aligned} & 9,477 \\ & 5,382 \\ & 1,794 \end{aligned}$ | Tearing down old masonry, at Re. 1 per 100 <br> Boulder masonry, at Rs. 14 per 100 <br> Brick-on-edge, at Rs. 25... <br> Tow-path 7' wide; vide Estimate No. 174 | 94.77 |
|  |  |  | $753 \cdot 48$ |
|  |  |  | $448 \cdot 5$ |
|  |  |  | 377.30 |
|  |  |  | 1,674.05 |

ESTIMATE No. 12.-ALTERATIONS AT MEHEWUR BRIDGE.

| Description of Work. | No. | L. | B. | D. | Qunntity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old Masonry. |  |  |  |  |  |  |  |
| $\begin{array}{cccc} \text { Curtains ... } & \text {.. } & \ldots & \ldots \\ \text { Flooring ... } & \text {... } & \text {... } & \end{array}$ | $\cdots$ | 150 | 21 | $\begin{gathered} 1.79 \\ 2 \end{gathered}$ | $\begin{aligned} & 1,074 \\ & 6,300 \end{aligned}$ | 7,374 | 7,374 |
| New flooring, as Estimate No. 11. |  |  |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. 12.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 7,374 | Tearing down old masonry, at Re. 1 per 100 <br> Flooring as Estimate No. 11 <br> Brick-on-edge as Estimate No. 11 <br> Tow-path $12{ }^{2}$ wide | Rs. |
|  |  |  | 73.74 |
|  |  |  | $753 \cdot 48$ |
|  |  |  | $448 \cdot 50$ |
|  |  |  | 595.60 |
|  |  |  | 1,871 $\cdot 32$ |

ESTIMATE No. 13.—ALTERATIONS ON SOLANI AQUEDUCT AND EMBANKMENT.


## ABSTRACT OF COST OF ESTIMATE No. 13.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Lineal feet Cubic feet | 9,450 | Staking 80 deep, at Re. 1 per lineal feet ... ... | 9,450.00 |
|  | 77,690 | Puddled clay, at Rs. 3 per 100 ... ... ... ... | 2,330.70 |
|  | 77,690 | Brick-on-edge, at Rs. 25 per 100 ... ... | 19,422.5 |
| " | 2,742 | Brick-work, at Rs. 20 per 100 ... | $548 \cdot 40$ |
|  |  | Grand Total | 31,751 6 |
| Square feet | 155,380 | N. B.-Cost of layer of Seyssel Asphalte, $\frac{1}{2}$ inch thick, $(914 \times 170)=\ldots \quad \text {... } . . \quad \text {... } . . . \quad . .$ | 1,55,380 |

ESTIMATE No. 14.-ALTERATIONS TO ROORKEE BRIDGE.

| Description of Work. | No. | L. | B. | D. | Quantity | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Masonry. <br> Raising channel revetments downstream |  |  |  |  |  |  |  |
|  | ... | 3-500 | 2 | 1 | 7,000 | 7,000 | 7,000 |

ABSTRACT OF COST OF ESTIMATE No. 14.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Cubic feet | 7,000 |  | $\begin{array}{lll}. . & \cdots & \cdots \\ \cdots & \cdots & \ldots\end{array}$ | $\begin{gathered} \text { Rs. } \\ 595 \cdot 6 \\ 1,400 \end{gathered}$ |
|  |  |  | Grand Total | 1,995.6 |

ESTIMATE No. 15.-FALL 4.07 AT ROORKEE.


ESTIMATE No. 15.-FALL 4.07 AT ROORKEE,-continued.


## ( 26 )

ESTIMATE No. 15.-FALL 4.07 AT ROORKEE,-continued.

| Description. |  | No. | L. | B. | D. | Quantity | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Pitching slopes | ... | 2 | 50 | 24 | 1 | 2,400 | 2,400 |  |
| Demolition. |  |  |  |  |  |  |  |  |
| Revetment wall | ... | ... | 300 | 4 | 12.50 | 15,000 | 15,000 | 15,000 |
| Grating | ... | 10 | 15 | $37 \cdot 5$ | ... | 5,625 | $\ldots$ | 5,625 |
| Girders .. | ... | 30 | 17 | ... | ... | 510 | ... | 510 |
| Wall-plates | ... | 10 | 15 | $\left\{\begin{array}{c} \cdots \\ 13 \\ 17 \end{array}\right\}$ | ... | 150 | ... | 150 |
| Lock gates ... | ... | 4 | 9-54 |  | ... | 1,144.8 |  | 1,144.8 |
| Crabs ... ... | ... | 8 | $\cdots$ | $\cdots$ | ... | 8 | 8 |  |
| Chain ... | ... | 44 | $\begin{aligned} & 27 \cdot 50 \\ & 29 \cdot 50 \end{aligned}$ |  |  | $\begin{aligned} & 110 \\ & 118 \end{aligned}$ | 228 |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  | 228 |

ABSTRACT OF COST OF ESTIMATE No. 51.

| Quantity. |  | Description. | Total |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 90,336 | Boulder masonry in foundations, at Rs. 14 per $100 . .$. | 12,647.04 |
| " | 74,370 | " \# superstructure, „ \# 20 , $100 \ldots$ | 14,874.00 |
| " | 30,000 | " ", \#, \# 24 , 100 .. | 7,200.00 |
| " | 10,636 | Arch-work, at Rs. 26 per $100 \ldots$... ... ... | 2,765•36 |
| " | 15,258 | Brick-on-edge, at Rs. 25 per 100 ... | 3,814.50 |
| " | 40,887 | Concrete, at Rs. 14 per 100 ... ... | 5,724.18 |
| " | 11,540 | Puddle, at Rs. 10 per 1,000 ... ${ }^{\text {a }}$.. | $115 \cdot 40$ |
| " | 5,310 | Boulder flocring, dry, at Rs. 7 per $100 . .$. | 371.70 |
| " | 2,400 | Pitching slopes, at Rs. 10 per $100 \ldots \ldots$ | 240.00 |
| " | 15,000 | Demolition, old masonry, at Re. 1 per 100 ... | 150.00 |
|  | 1,454,474 | Excavation, at Rs. $2 \cdot 5$ per 1,000 | 3,636•18 |
| Square feet | 5,625 | Grating, at Rs. $1 \cdot 15$ per square foot | 6,468.75 |
| Lineal feet | 510 | Girders, at Rs. $4 \cdot 25$ per foot | 2,167.50 |
|  | 150 | Wall-plates, at Rs. $6 \cdot 25$ per foot | 937.50 |
| Number | 72 | Sleepers, at Rs. 11.625 each ... | $837 \cdot 00$ |
| Square feet | 1,145 | Lock gates, at Rs. 6.5 per foot ... | 7,442.50 |
| Number |  | Crabs, at Rs. 135 each ... | 1,080.00 |
| " |  | Sluices, at Rs. 125 each ... | 100.00 |
| I. 1 |  | Pulleys, at Rs. 5 each .. | 40.00 |
| Lineal feet | 228 | Chain, at Re. 1 per foot ... | 228.00 |
| Number | 2 | Hooks, at Rs. 7 each ... ... ... ... | 14.00 |
|  |  | Grand Total | 71,753.61 |

ESTIMATE No. 16.-ADDITIONS, \&c., TO GUNESHPOOR, LIBURHEREE AND DIMAT BRIDGES.


ESTIMATE NO. 16.-ADDITIONS, \&c., TO GUNESHPOOR, LIBURHEREE AND DIMAT BRIDGES,-continued.


ABSTRACT OF COST OF ESTIMATE No. 16.

| Quantity. |  | Description. | Total |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 18,326 | Digging boulder out of cribs, at Rs. 7 per 1,000 | 128.28 |
| , | 26,898 | Demolition, old masonry, at Re. 1 per 100 ... ... | 268.98 |
| " | 34,170 | Brick-work, at Rs. 22 per 100 ... ... ... | 7,517-40 |
| " | 3,591 | Arch-work, heavy, at Rs. 32 per 100 ... .. | 1,149.12 |
| " | 2,940 | Brick-on-edge, at Rs. 25 per 100 ... | $735 \cdot 00$ |
| " | 5,500 | Boulder flooring, dry, at Rs. 7 per 100... | $385 \cdot 00$ |
| [. ${ }^{\prime}$ | 24,900 | Filling old boulders, at Rs. 7 per 1,000... | 174.30 |
| Lineal feet | 45 | Well, sinking, at Rs. 7 per foot... ... | 315.00 |
| Number | 262 | Piles, at Rs. 5,625 each ... ... ... | 1,473.75 |
| Cubic feet | 55 | Waling pieces, at Re. 1 per foot | 55.00 |
|  | 682 | Metalling, at Rs. 6 per 100 ... | 40.92 |
| Number | 5 | Large curbs, at Rs. 90 each ... | $450 \cdot 00$ |
| Cubic feet | 171,600 | Excavation, at Rs. $2 \cdot 3$ per 1,000 ... ... ... | $480 \cdot 48$ |
|  |  | Tow-path ... ... ... ... ... ... | 595.60 |
|  |  | Grand Total | 13,768.83 |

ESTIMATE No. 17.-ALTERATIONS TO HAFEZNUGGUR FALLS.


ESTIMATE No. 17.-ALTERATIONS TO HAFEZNUGGUR FALLS,-continued.


ABSTRACT OF COST OF ESTIMATE No. 17.

| Quantity. |  | Description. |  |  | Cost. <br> Rs. <br> 2,174.60 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cubic feet | 966,490 |  |  |  |  |
| " | 91,973 | Demolition, old masonry, at Re. 1 per 100 |  |  | 919.73 |
| " | 15,859 | Boulder digging, at Rs. 7 per 1,000 |  |  | 111.01 |
| " |  | Rajbuha head ... ... ... |  | .. | 1,410.78 |
| " | 31,954 | Masonry, at Ks. 22 per 100 ... |  | .. | 7,029•88 |
| " | 3,024 | Arch-work at Rs. 28 per 100 ... |  | .. | $846 \cdot 72$ |
| " | 10,609 | Brick-on-edge, at Rs. 25 per 100 |  | . | 2,652.25 |
|  | 8,175 | Stone flooring, at Re. 2 per foot |  |  | 16,350.00 |
| Square feet | 4,752 | Gratirg, at Rs. $1 \cdot 15$ per foot ... |  |  | 5,464•80 |
| Lineal feet | 416 | Girders, at Rs. $4 \cdot 25$ per foot ... |  |  | 1,768.00 |
|  | 6,200 | Wall-plates, at Rs. $6 \cdot 2 \mathrm{o}$ per foot |  | , | 1,100.00 |
| Cubic feet |  | Pitching, at Ks. 10 per 1,000 .. |  | . | -620.00 |
|  |  |  | Grand Total |  | 40,447.77 |

ESTIMATE No. 18.-ALTERATIONS TO MUNGLOUR BRIDGE.


ESTIMATE No. 18.-ALTERATIONS TO MUNGLOUR BRIDGE,-continued.


ABSTRACT OF COST OF ESTIMATE No. 18.


ESTIMATE No. 19.-FALL 6.87 AT LIBURHEREE BRIDGE.


ESTIMATE No. 19.-FALL 6.87 AT LIBURHEREE BRIDGE.-continued.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Brought forward ... | ... | $\cdots$ | $\cdots$ | $\cdots$ | 124,672.54 |  |  |
| Superstructure-contd. |  |  |  |  |  |  |  |
| Abutment of bridge Retaining walls down-stream | 2 2 | 3 105 | 3 3.5 | $9 \cdot 5$ 14 | $\begin{array}{r} 171 \\ 10,290 \end{array}$ |  |  |
| Deduct- |  |  |  |  |  |  |  |
| Vide Estimate No. 22 ... | ... | ... | .. | $\ldots$ | ... | 25,082.50 |  |
| Lock gates... ... ... | 4 | 9.54 | .. | $\left\{\begin{array}{l}11 \\ 18\end{array}\right\}$ | 1,106.64 |  |  |
| RemainderasEstimate No. 22. |  |  |  |  |  |  |  |

ABSTRACT OF ESTIMATE NO. 19.


ESTIMATE No. 20.-ALTERATIONS TO MUNDOWLEE TOGLUGHPOOR BAILRA BHOPA BRIDGE.


ABSTRACT OF ESTIMATE No. 20.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 19,904 | Demolition, old masonry, at Re. 1 per 100 | $\begin{gathered} \text { Rs. } \\ 199 \cdot 04 \end{gathered}$ |
| " | 28,226 | Brick-work, at Rs. 22 per 100 ... ... | 6,209•72 |
| Number | 245 | Sheet piles, at Rs. $5 \cdot 625$ each ... .. | 1,378•12 |
|  |  | Remainder as Estimate No. 16 ... ... | 4,508.7 |
|  |  | Grand Total | 12,295.58 |

ESTIMATE No. 21.-ADDITIONS TO MUHMOODPOOR FALLS.


ABSTRACT OF ESTIMATE No. 21.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  | 111,837 |  | Rs. |
| Cubic feet.. |  | Demolition old masonry, at Re. 1 per 100 ... ... | 1,118•37 |
|  | 24,755 | Boulder digging, at Rs. 7 per 1,000 ... ... ... | 173.28 |
| " |  | Remainder as Estimate No. 17 ... ... ... | 59,417.08 |
|  |  | Grand Total | 40,708.68 |

ESTIMATE No. 22.-TOGLUGHPOOR FALLS.

| Description of Work. | No. | L. | B. | D. | Quantity | Toral. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Foundations. |  |  |  |  |  |  |  |
| Water-wings up-stream ... | 2 | 25 | $5 \cdot 5$ | 8 | 2,200 |  |  |
| Curtain ... ... ... |  | 16 | 3 | 5 | 480 |  |  |
| Drop walls ... ... | 2 | 16 | 3 | 12.54 | 1,203•84 |  |  |
| " $\ldots$... ... | 2 | 16 | 3 | 4 | 384 |  |  |
| Curtains ander lower gates ... | 4 | 16 | 3 | 3 | 516 |  |  |
| ," sides of wells ... | 2 | 14 | 6 | 4 | 672 |  |  |
| A butments of gates up-stream | 2 | 16.5 | 6.5 | 8 | 1,716 |  |  |
| Sid" $"$ down-stream | 2 | $16 \cdot 5$ | 12 | $2 \cdot 5$ | 990 |  |  |
| Side walls of locks ... | 2 | 106 | 9 | $2 \cdot 5$ | 4,770 |  |  |
| A batments of bridge... | 2 | 3 | 6 | $2 \cdot 5$ | 90 |  |  |
| Side wall down-stream ... | 2 | 50 | $2 \cdot 5$ | 6 | 1,500 |  |  |
| Flooring up-stream | 2 | 16 | 17 | 3 | 1,632 |  |  |
| \# of well ... | 2 | 16 | $7 \cdot 25$ | 3 | 696 |  |  |
| \# under gates... ... | 2 | 16 | 10.5 | 2.5 | 840 |  |  |
| Backing of inverts ... ... | 2 | 168.5 | 16 | 1.75 | 9,436 |  |  |
| Division wall of lock ... | 2 | 18 | 7 | 8 | 2,016 |  |  |
| " " sides of well ... | 2 | 14 | 7 | $15 \cdot 54$ | 3,045-84 |  |  |
| \# \# of lock ... | 2 2 | 77 | 7 | ${ }_{5}^{8}$ | 8,624 |  |  |
| Drop wall of fall ".. | ... | 94 213 | 7 5.5 | $5 \cdot 5$ 14.04 | 7,238 $16,447 \cdot 96$ |  |  |
| Curtain of fall $\quad$... | $\ldots$ | 213 | 3 | 4 | 2,556 |  |  |
| ." $\quad$.. | ... | 213 | 3 | 1.5 | 958.50 |  |  |
| Flooring of fall ... | $\cdots$ | 213 | 66 | 1.5 | 21,087 |  |  |
| Wells ... | 26 | 14.12 | 1.5 | 7 | 3,854•76 |  |  |
| " |  | $18 \cdot 8$ | 2 | 9 | 2,030.40 |  |  |
| " ... .. | 30 | 14.12 | 1.50 | 9 | 5,718.60 |  |  |
| Covering of wells in curtains... | 26 | 7.0 |  | 1.5 | $275 \cdot 34$ |  |  |
| , " division wall | 2 | 22 | 7 | 2 | 616 |  |  |
| " " basin ... | 2 | 105 | 6 | 2 | 2,520 |  |  |
| Deduct Brick-on-edge. |  |  |  |  |  |  |  |
| Lock flooring ... ... | 2 | 16 | 17 | 1 | 544 |  |  |
| " ... ... | 2 | 16 | 17.25 | 1 | 232 |  |  |
| Mon ${ }^{\prime \prime}$... ... | 2 | 16 | $10 \cdot 5$ | 1 | 336 |  |  |
| Flooring of fall ... ... | ... | 213 | 66 | 1 | 14,058 |  |  |
| Tops of drop wall and curtain. . | $\because$ | 213 | 8 | 1 | 1,704 |  |  |
| Hollow foundations ... | 2 | 25 | $1 \cdot 5$ | 4 | 300 |  |  |
| Superstructure. |  |  |  |  |  |  | 87,000 |
| Water-wings up-stream ... | 2 | 25 | $3 \cdot 5$ | 14 | 2,450 |  |  |
| Abutments of gates up-stream. | 2 | 16.5 | 6 | 14 | 2,772 |  |  |
| " ," down-stream | 2 | 16.5 | 9 | 22.54 | 6,694.38 |  |  |
| Abutments of bridge ... ... | 2 | 3 | 5 | 14 | - 390 |  |  |
| Counterforts ... ... | 2 | 3 | $3 \cdot 5$ | 8 | 168 |  |  |
| Wing walls of bridge... | 2 | 12 | $9 \cdot 5$ | 9 | 2,052 |  |  |
| Side walls down-stream | 2 | 50 | $3 \cdot 5$ | 14 | 4,900 |  |  |
| D. of lock ... | 2 | 112.5 | 6 | 22.54 | 30,429 |  |  |
| Division wall of lock ... | 2 | 109 | 7 | 14 | 21,364 |  |  |
| 》 ... | 2 | 41 | 7 | 22.54 | 12,937-96 |  |  |
| \% $\quad$.. | , | 53 |  | 14 | 10,388 |  |  |
| Piers ... ... ... | 11 | 37 | 8 | 26.54 | 32,405•34 |  |  |
| " ... ... ... | 11 | 5 | 3 | 14 | 2,310 |  |  |
| Steps ... ... ... | 2 | 18 | 7 | 2 | 504 |  |  |
| " ${ }^{\text {a.. }}$... ... | 12 | 2 | 1 | 2 | 8 |  |  |
| Backing of arches ... ... | 12 | 15 | 3 | 1 | 540 |  |  |
| Carried over ... | ... | ... | ... | ... | 130,312.68 |  |  |

ESTIMATE No. 22.-TOGLUGHPOOR FALL,-continued.


ESTIMATE No. 22.-TOGLUGHPOOR FALL,-contivuer.


ABSTRACT OF COST OF ESTIMATE No. 2i.


## 

| Dramiriotion or Work. | Ns. | 1. | B. | D. | Qramtrit | Total | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Fexcavalion. |  |  |  |  |  |  |  |
| Enek Channel .. |  | 1,500 | 38 | 12 | P9, 3,000 |  |  |
| Filling behind nide walle | 2 | 4) | 16 | 8 | - 10,240 |  |  |
| Demolition, old Manonry. |  |  |  |  |  |  |  |
| At head of Look Channel .. Tide Eatimate No. 17. | $\cdots$ | $\cdots$ | ... | $\cdots$ | 22,713-50 |  |  |
| Flooring of falls ... ... | 4 | 95 | 54.3 | 5 | 103,550 |  |  |
| " " ... | 3 | 12.5 | $4 \cdot 5$ | 5 | 843.75 |  |  |
| Digging boulders out of cribs | ... | 350 | 80 | 5 | 140,000 |  |  |
| Masonry. |  |  |  |  |  |  |  |
| Fide Eatimate No. 17 | $\ldots$ | $\ldots$ | $\cdots$ | ... | 31,954 |  |  |
| wings ... ... ... | 2 | 30 | 4.5 | 1 | 270 |  |  |
| Lock bridge vide Estimate No. 17 | . | $\cdots$ | $\cdots$ | $\cdots$ | ... | 1,542.52 |  |
| Remainder as Estimate No. 17. |  |  |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. 23.

| Quantity. |  | Description of Work. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Re. |
| Cabic feet | 694,240 | Excavation, at Rs. 2.25 per 1,000 ... ... ... | 1,562.04 |
| " | 127,107 | Demolition, old masonry, at Re. 1 per 100 ... ... | 1,271.07 |
| " | 140,000 | Excavating boulders, Rs. 7 per 1,000 ... ... | $980 \cdot 00$ |
| " | 140,000 | Relaying boulders, at Rs. 7 per 1,000 ... ... ... | $980 \cdot 00$ |
| " | 30,68 I | Brick-work, at Rs. 22 per 100 ... ... ... ... | 6,749.82 |
|  |  | Remainder as Estimate No. 17 ... ... ... | 30,212 56 |
|  |  | Grand Total | 41,755•49 |

$$
(41 \quad)
$$

ESTIMATE No. 24.-ALTERATIONS TO JAOLI FALLS.

| Description. | No. | L. | B. | D. | Quantity. | Total | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old Masonry. |  |  |  |  |  | 139,186 |  |
| Vide Estimate No. 23 ... ... | $\cdots$ | $\cdots$ | $\cdots$ | $\cdots$ | 127,107 |  |  |
| Additional for round ends of walls, vide Estimate No. 17 ... | ... | $\cdots$ | $\ldots$ | $\cdots$ | 8,830 |  |  |
| Widening of flooring differently from Nerzajnee falls, vide Estimate No. 23 | 2 | 95 | 5 | $3 \cdot 42$ | 3,249 |  |  |
| Deduct- |  |  |  |  |  |  |  |
| In flooring ... ... ... | 4 | 95 | 54.5 | 1.58 | 32,721 |  |  |
| „ ... ... ... | 2 | 12.5 | $4 \cdot 5$ | 158 | 177.75 |  |  |
| Boulder digging ... ... | ... | 177 | 80 | 1.21 | 17,133.60 |  | 106,287 |
| , ... .. | 2 | 70 | 37 | 1-21 | 6,267•80 |  |  |
| Remainder as Estimate No. 23. |  |  |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. 24.


ESTIMATE No. 25.-ALTERATIONS AT FUTTEYGURH BRANCH HEAD.


ESTIMATE No. 25.-ALTERATIONS AT FUTTEYGURH BRANCH HEAD,-contd.


## ABSTRACT OF COST OF ESTIMATE No. 25.



$$
\begin{gathered}
(44 \mathrm{l}) \\
\text { ESTIMATE No. 26.-DUKHERI BRIDGE AND FALL- } 8.53 .
\end{gathered}
$$

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Foundations. |  |  |  |  |  |  |  |
| Water-wings up-steam ... | 2 | 25 | $5 \cdot 50$ | 8 | 2,200 |  |  |
| Curtain ... ... ... | 2 | 16 | 3 | 5 | 480 |  |  |
| Drop walls... ... ... | 2 | 16 | 8 | 7-53 | $72.2 \cdot 88$ |  |  |
| \# ... ... ... | 2 | 16 | 3 | 4 | 384 |  |  |
| Curtains under lower gates | 4 | 16 | 3 | 3 | 576 |  |  |
| Sides of wells .. ... | 2 | 14. | 6 | 4 | $\begin{array}{r}672 \\ \hline 716\end{array}$ |  |  |
| Abutments of gatesup-stream | 2 | 16.50 | 6.50 | 8 | 1,716 |  |  |
| , ", down-stream | 2 | 16.50 | 12 | $2 \cdot 50$ | 990 |  |  |
| Side walls of locks ... | 2 | 106 | 9 | $2 \cdot 50$ | 4,770 |  |  |
| Abutments of bridge ... | 2 | 3 | 6 | $2 \cdot 50$ | 90 |  |  |
| Side wall down-stream ... | 2 | 50 | 6 | $2 \cdot 50$ | 1,500 |  |  |
| Flooring up-stream ... | 2 | 16 | 17 | 3 | 1,632 |  |  |
| \% well ... ... | 2 | 16 | 7.25 | 3 | 696 |  |  |
| ", under gates ... | 2 | 16 | $10 \cdot 50$ | $2 \cdot 50$ | 840 |  |  |
| Backing of inverts ... | 2 | 168.50 | 16 | 175 | 9,436 |  |  |
| Division wall of lock | 2 | 18 | 7 | 8 | 2,016 |  |  |
| , \# sides of well | 2 | 14 | 7 | $10 \cdot 53$ | 2,063.88 |  |  |
| \# \# of lock ... | 2 | 80 | 7 | 8 | 8,960 |  |  |
| " ${ }^{\prime \prime}$ | 2 | 70 | 7 | $5 \cdot 50$ | 5,390 |  |  |
| Drop wall of fall ... ... | $\ldots$ | 159 | 5.50 | $9 \cdot 03$ | 7,896.73 |  |  |
| Curtain of fall ... .. | $\cdots$ | 159 | 3 | 4 | 1,908 |  |  |
| " ${ }^{\prime}$. ${ }^{\text {a }}$. | $\cdots$ | 159 | 3 | $1 \cdot 50$ | $715 \cdot 50$ |  |  |
| Flooring of fall $\quad \cdots \quad \cdots$ |  | 159 | 64 | 150 | 15,264 |  |  |
| Wells in curtain of fall .. | 20 | $14 \cdot 12$ | 1.50 | 7 | 2,965-20 |  |  |
| \#, walls of basin $\begin{gathered}\text { division walls } \\ \text { \% }\end{gathered}$ | 28 | $14 \cdot 12$ | 1.50 |  | 5,337-36 |  |  |
| ", division walls ... | 4 | $\begin{array}{r} 18.08 \\ \mathrm{Ar} \end{array}$ | 2 | 9 | 1,353:60 |  |  |
| Covering of wells in curtain | 20 |  |  | $1 \cdot 50$ | $211 \cdot 80$ |  |  |
| " " in division wall |  | 18 | 7 | 2 | 252 2 |  |  |
| Flooring of wridge... | 2 3 | 95 55 | ${ }^{6}$ | 2 | 2,280, 3,465 |  |  |
| Flooring of bridge... ... | 3 | 55 | 21 |  | 3,465 | 86,783.97 |  |
| Deduct, Brick-on-edge. |  |  |  |  |  |  |  |
| Lock flooring ... ... | 2 | 16 | 17 |  | 544 |  |  |
| " $\quad$. |  | 16 | 7.25 | 1 | 232 |  |  |
| Mor ${ }^{\prime \prime}$... | 2 | 16 | $10 \cdot 50$ | 1 | $\begin{array}{r}336 \\ \hline 176\end{array}$ |  |  |
| Flooring of fall $\ldots$.... | ... | 159 | 64 | 1 | 10,176 |  |  |
| Top of drop wall and curtains | ... | 159 | 8 | 1 | 1,272 |  |  |
| $\left.\begin{array}{rcc}\text { Hollows in } & \text { water-wings } \\ \text { up-stream } & \ldots & \ldots\end{array}\right\}$ | 2 | 25 | 1.50 | 4 | 300 | 12,860 |  |
| Superstructure. |  |  |  |  |  |  |  |
| Water-wings up-stream... | 2 | 25 | $3 \cdot 50$ | 13 | 2,275 |  |  |
| Abutments of gates , ... | 2 | 16.50 | 6 | 13 | 2,574 |  |  |
| ," down-stream | 2 | $16 \cdot 50$ | 9 | 16.53 | 4,909•41 |  |  |
| Abutments of bridge ... | 2 | 3 | 5 | 13 | 390 |  |  |
| " " | 2 | 3 | 3 | 9.50 | 171 |  |  |
| Counterforts ... | 2. | 3 | $3 \cdot 50$ | 8 | 168 |  |  |
| Wing-walls of bridge | 2 | 12 | $2 \cdot 50$ | 9 | 540 |  |  |
| Side walls down-stream | 2 | 15 | $3 \cdot 50$ | 13 | 4,550 |  |  |
| " of lock | 2 | 112.50 | 6 | 16.53 | 22,315.50 |  |  |
| Division wall of lock | 2 | 112 | 7 | 13 | 20,384 |  |  |
| " " | 2 | 38 | 7 | 16.53 | 8,793.96 |  |  |
| " $\quad$ | 2 | 53 | 7 | 13 | 9,646 |  |  |
| Carried over | .. | ... | ... | ... | 76,716.87 |  |  |

( 45 )
ESTIMATE No. 26.—DUKHERI BRIDGE AND FALL, 3•53-continued.


ESTIMATE No. 26.-DUKHERI BRIDGE AND FALL, 3.53-continned.


ABSTRACT OF ESTIMATE No, 26.

| Quantity. |  | Description: | Total |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet: | $\begin{gathered} 73,914 \\ 57,569 \end{gathered}$ | Brick-work in foundations, at Re. 20 per 100 ... <br> " " superstructure, at Rs. 20 per $100 \quad \cdots$ | $\begin{aligned} & 14,782 \cdot 80 \\ & 11,513 \cdot 80 \end{aligned}$ |
|  | 30,000 |  | 7,200.00 |
| " | 1,878 | Arch-work light, at Rs. 25 per 100." ... ... | $469 \cdot 50{ }^{-}$ |
| " | 8,341 | B, inverts, at Rs. 26 per 100 ... ... | 2,168.66 |
| " | 16,839 | Brick-on-edge, at Rs. 25 per 100 ... ... | 4,084.75 |
| $\cdots$ | 33,390 | Concrete, at Rs. 14 per 100 ... ... ... | 4,674:60 |
| " | 10,880 | Puddle, at Rs. 10 per 1,000 .... ... | 108.80 |
|  | 4,770 | Kunkur flooring, at Rs. 8 per 100 ... | $381 \cdot 60$ |
| Lineal feet | 428 | Well sinking, at Rs. 5 per foot ... ... | 2,14000 |
| Number | 52 | Curbs, at Rs. 20 each ... $\ldots$ | 1,040.00 |
| Cubic feet | 127,488 | Excavation, at Rs. 2.5 per 1;000 | 318.72 |
| " | 17,900 | Boulder digging, at Rs. 7 per 1,000 ... | 125.30 |
| " | 23,120 | Relaying boulder flooring, at Rs. 7 per 1,000 | 161.84 |
| ", | 2,400 | Pitching slopes, at Rs. 10 per $100 \ldots \ldots$ | 240.00 |
| " | 7,039 | Demolition, old masonry, at Re. 1 per 100 | $70 \cdot 39$ |
|  |  | Towing-path 12' wide ... ... .. | 59\%'60 |
|  |  | Carried over | 50,076.36 |

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(47)
$$

ABSTRACT OF ESTIMATE No. 26,-continued.


## ESTIMATE No. 27.-NUGLA BRIDGE.



ABSTRACT OF COST OF ESTIMATE No. 27.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cabic feet | 8,732 | Demolition, old masonry, at Re. 1 per 100 ... ... | 87.32 |
| " | 5,693 | Brick-work, at Res. 22 per 100 ... ... ... ... | 1,252•46 |
| " | 1,897 | Brick-on-edge at Rs. 25 per 100 ... ... ... | $474 \cdot 25$ |
|  |  | Tow-path 12' wide ... ... ... ... ... | $595 \cdot 60$ |
|  |  | $G_{\text {rand }}$ Total | 2,409 63 |

Same for Suraiee, Sutheree, and Aternak Bridges.

ESTIMATE No. 28.-SIRDHUNNA AND KHUTOWLEE BRIDGES.

| Description. | No. | L. | B. | D | Quantity. | Total. | Grand <br> Total. |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Masonry. <br> Ghats and water-wings | $\ldots$ | 2 | 160 | 2 | 1.5 | 960 |  |  |

ABSTRACT OF COST OF ESTIMATE No. 28.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Cubic feet | 960 | Brick-work, at Rs. 22 per 100 ... Tow-path $12{ }^{\prime}$ wide | $\cdots \quad \cdots \quad . .$. | Rs. <br> $211 \cdot 20$ <br> $772 \cdot 80$ |
|  |  |  | Grand Total | 984.00 |

ESTIMATE No. 29.-ALTERATIONS TO CHITOWRA FALLS.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Excavation. <br> Lock channel Filling behind side walls <br> Demolition. | $\ddot{2}$ | $\begin{array}{r} 1,500 \\ 40 \end{array}$ | $\begin{aligned} & 35 \\ & 16 \end{aligned}$ | $\begin{array}{r} 10 \\ 8 \end{array}$ |  | 535,240 | 535,240 |
|  |  |  |  |  | $\begin{array}{r} 525,000 \\ 10,240 \end{array}$ |  |  |
|  |  |  |  |  |  | 135,937 |  |
| Old masonry, as Estimate No. 23 Tail walls, vide Estimate No. 17 | $\ldots$ | $\ldots$ | $\ldots$ | $\ldots$ | $\begin{array}{r} 127,107 \\ 8,830 \end{array}$ |  | 135,937 |
| Masonry. |  | $\begin{aligned} & 120 \\ & 314 \\ & 150 \end{aligned}$ | $\begin{aligned} & \dddot{3} \\ & 4 \cdot 5 \\ & 9 \end{aligned}$ | 222 | $\begin{array}{r} 30,681 \\ 720 \\ 2,826 \\ 2,700 \end{array}$ | 36,927 |  |
| As Estimate No. 23   <br> Raising lock walls $\ldots$ $\ldots$ <br> ..   | ... |  |  |  |  |  |  |
| , " ... . | ... |  |  |  |  |  |  |
| \# " $\quad$ ". ${ }^{\text {\% }}$ |  |  |  |  |  |  |  |
| Remainder as Estimate No. 23. |  |  |  |  |  |  | 36,927 |

ABSTRACT OF COST OF ESTIMATE No. 29.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Ro. |
| Cubic feet | 5,35,240 | Excavation, at Rs. 2:25 per 1,000 ... ... ... | 1,204.29 |
|  | 1,34,937 | Demolition, old masonry, at Re. 1 per 100 ... ... | 1,359•37 |
| " | 36,927 | Brick-work, at Rs. 22 per 100 ... ... ... ... |  |
|  |  | Remainder as Estimate No. 23 ... | $32,173 \cdot 56$ |
|  |  | Grand Total | 42,861•16 |

ESTIMATE No. 30.-l—KHUTOWLEE ESCAPE; 2-ABOONULLAH; 3-JANNI HEAD.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand <br> Total. |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Demolition, old Masonry. |  |  |  |  |  |  |  |  |
| Flooring ... | $\ldots$ | $\ldots$ | 4 | 24 | 6 | 2.5 | 1,440 | 1,440 |
| New Masonry | $\ldots$ | $\ldots$ | 4 | 24 | 6 | 1.5 | 864 | 864 |
| Brick-on-edge | $\ldots$ | $\cdots$ | 4 | 24 | 6 | $\cdot 5$ | 288 | 288 |

ABSTRACT OF COST OF ESTIMATE No. 30.

| Quantity. |  | Descripiton. | Cosr. |
| :---: | :---: | :---: | :---: |
| Cabic feet „ \% | 1,449 864 288 | Demolition, old masonry, at Re. 1 per 100 <br> Brick-work, at Rs. 20 per 100 <br> Brick-on-edge, at Rs. 25 per 100 | $\begin{gathered} \mathrm{Ks} . \\ 14 \cdot 40 \\ 1,72 \cdot 80 \\ 72 \cdot 00 \end{gathered}$ |
|  |  | Grand Total | $259 \cdot 20$ |

ESTIMATE No. 31.-FALLS AT KHUTOWLEE AND SIRDHUNNA.

| Drscription of Work. | No. | L. | B. | D. | Quantity. | Тотal | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Foundation. <br> As Estimate No. 26 Wells under curtain of basin |  |  |  |  |  |  |  |
|  |  |  |  | $\ddot{9}$ | 73,914 |  |  |
|  | 25 | $\underset{\substack{\text { Area. } \\ 7 \cdot 06}}{\left.\dddot{4} \cdot 12\right\|^{1} 9}$ |  | 9 | 4,765:50 |  |  |
| Covering of wells ... <br> Curtain of basin ... <br> Brick-on-edge. <br> As Estimate No. 26, deducting bridge flooring <br> Well Sinking. | $25$ |  |  | 1.5 | $\begin{gathered} 264.75 \\ 808 \end{gathered}$ | 79,752-25 |  |
|  |  | 202 | 2 | 2 |  |  | 79,752 |
|  |  |  |  |  |  |  |  |
|  | ... | $\cdots$ | $\cdots$ | $\ldots$ | $\cdots$ | $\ldots$ | 12,874 |
|  |  |  |  |  |  |  |  |
| As Estimate No. 26 |  |  |  |  | 428 |  |  |
| $\begin{array}{cc} \text { Curtain walls } & \text { Curbs. } \end{array}$ | 25 | 9 | .. | ... | 225 | $\ldots$ | 653 |
| As Estimate No. 26 | $\cdots$ | $\ldots$ | ... | ... | 52 |  |  |
| Cortain walls $\quad . . . \quad .$. | $\cdots$ | $\ldots$ | . | ... | 25 | ... | 77 |
| Remainder as Estimate No. 26 (without bridge) |  |  |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. sl.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 79,752 | Brick-work in foundation, at Rs. 20 per 100 | $\begin{gathered} \text { Rs. } \\ 15,950 \cdot 40 \end{gathered}$ |
|  | 12,874 | Brick-on-edge, at Rs. 25 per 100 ... ... ... | 3,218.50 |
| Lineal feet | 653 | Under-sinking wells, at Rs. 5 per lineal foot ... ... | 3,265.00 |
| No. | 77 | Curbs, at Rs. 20 per each ... ... ... | 1,540.00 |
| Cubic feet | 1,878 | Arching light, at Rs. 25 per 100 ... ... | $469 \cdot 50$ |
| , | 8,341 | Inverts, at Rs. 26 per 100 ... ... | 2,168.66 |
| , | 33,390 | Concrete, at Rs. 14 per 100 ... | 4,674.60 |
| " | 10,880 | Puddle, at Rs. 10 per 1,000 $\ldots$ | 108.80 |
| " | 4,770 | Kunkur flooring, at Rs. 8 per 100 | 381.60 |
| " | 127,488 | Excavation, at Rs. 2.5 per 1,01)0 | 318.72 |
| " | 2,400 | Pitching slopes, at Rs. 10 per 100 | $240 \cdot 00$ |
| " | ... | Lock and grating gear as Estimate No. 26 | 17,128•55 |
|  |  | Grand Total | 49,464-33 |

ESTIMATE No. 32.-ADDITIONS, \&c., TO SALAWUR AND MUSSOORI FALLS.


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\text { ( } 51 \text { ) }
$$

ESTIMATE No. 32.-ADDITIONS, \&c., TO SALAWUR \& MUSSOORI FALLS,-contd.


ABSTRACT OF ESTIMATE No 32.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 612,490 | Excavation, at Rs. $2 \cdot 25$ per 1,000 ... ... | 1,378•10 |
| " | 97,887 | Demolition, old masonry, at Re. 1 per 100 ... ... | 978.87 |
| " | 70,987 | Digging boulders out of cribs, at Rs. 7 per 1,000 ... | 496.90 |
| " | 36,300 | Boulder flooring, dry relaying, at Rs. 7 per 1,000 ... | 254.10 |
| " | 65,998 | Brick-work, at Rs. 22 per 100 ... ... | 14,519.56 |
| " | 2,126 | Arch-work, at Rs. 26 per 100 ... ... | $552 \cdot 76$ |
| " | 8,143 | Brick-on-edge, at Rs. 25 per 100 ... ... | 2,035.75 |
| " | 6,131 | Stone flooring at Rs. 2 per foot ... ... ... | 12,262.00 |
|  |  | Grating Gear. |  |
| Square feet | 4,158 | Grating, at Re. 1-15 per square foot | 4,781.70 |
| Lineal feet | 468 | Girders, at Rs. 4.25 per foot ... ... ... ... | 1,989.00 |
| " | 132 | Wall-plates, at Rs. 6.25 per foot... ... ... ... | 825.00 |
| Cabic feet | 6,000 | Pitching, at Rs. 30 per 100 ... | $600 \cdot 00$ |
|  |  | New Rajbuha head ... ... ... ... | 1,410.78 |
|  |  | Grand Total | 41,484.52 |

ESTIMATE No. 33.-ADDITIONS, \&c., TO NUNOON AND MORADNUGGUR BRIDGES.


ABSTRACT OF ESTIMATE No. 33.

| Quantity. |  | Description. | Total |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 26,844 | Demolition, old masonry, at Re. 1 per 100 ... ... | 268.44 |
| " | 13,195 | Digging boulders out of cribs, at Rs. 7 per 1,000 ... | 92.36 |
| " | 8,428 | Brick-work, at Rs. 22 per 100 ... ... ... ... | 1,854•16 |
| " | 2,058 | Brick-on-edge, at Rs. 25 per 100 ... ... ... | 514.5 |
| Lineal feet | 150 | Girder bridge, at Rs. 96.65 per foot ... '... ... | 14,497-50 |
|  |  | 7' Tow-path, Estimate No. ... ... ... ... | 501.5 |
|  |  | Grand Total | 37,728•46 |

( 53 )
ESTIMATE No. 34.-ALTERATIONS TO $\left\{\begin{array}{l}\text { JUTPOORA } \\ \text { NEWAREE } \\ \text { ARIDGE, } \\ \text { ABOOPOR } \\ \text { NOORPOOR }\end{array}\right.$

| Drsoription. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old Masonry. |  |  |  |  |  | 21,174•17 | 21,174 |
| Arches ... ... ... | 3 | 54. | 21 | 2.75 | 9,355•50 |  |  |
| Backing of arches ... ... | 2 | 21 | 10 | 2 | 840 |  |  |
| , ... | 2 | 21 | 13 | 4 | 2,184 |  |  |
| Spandrils ... ... ... | 2 | 163 | $2 \cdot 5$ | $2 \cdot 5$ | 2,037.50 |  |  |
| Parapets ... ... ... | 2 | 235 | $1 \cdot 5$ | $2 \cdot 87$ | 2,023.35 |  |  |
| Pillars ... ... ... | 4 | 3 | 3 | $2 \cdot 87$ | $103 \cdot 32$ |  |  |
| Flooring ... ... ... | 3 | 49 | 21 | $1 \cdot 5$ | 4,630.50 |  |  |
| Digging boulders out of cribs | 2 | 163 | 10 | $2 \cdot 11$ | 6,878.60 | 7,722.60 |  |
| " \# ... | 2 | 20 | 10 | $2 \cdot 11$ | 844.00 |  |  |
| New Masonry. |  |  |  |  |  |  | 7,723 |
| Raising piers ... ... | 2 | 21 | 6.5 | 12 | 3,276 | 6,685 17 |  |
| \% Abutments | 2 | 21 | 6.5 | 1.5 | $409 \cdot 50$ |  |  |
| \# Wings ... | 4 | 35 | $2 \cdot 5$ | 1.5 | 525 |  |  |
| \# Pillars ... ... | 4 | 4 | 4 | 1.5 | 96 |  |  |
| Parapets ... ... | 4 | 36 | $1 \cdot 5$ | 2.87 | 619.92 |  |  |
| Pillars .... ... | 4 | 3 | 3 | $2 \cdot 87$ | $103 \cdot 32$ |  |  |
| Parapets on piers ... ... | 4 | $6 \cdot 5$ | $1 \cdot 5$ | $2 \cdot 87$ | 111.93 |  |  |
| Flooring ... ... ... | 3 | 49 | 21 | 0.5 | 1,543.50 |  | 6,685 |
| Brick-021-edge ... ... | 3 | 49 | 21 | $\cdot 5$ | 1,54.3.5 | 1,543 | 1,543 |
|  |  |  |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. 34.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 21,174 | Demolition, old masonry, af Re. 1 per 100 ... ... | 211.74 |
| " | 7,723 | Digging boulders out of cribs, at Rs. 7 per 1,000 ... | 54.06 |
| " | 6,685 | Masonry, at Rs. 22 per 100 ... ... ... ... | 1,470.70 |
| " | 1,543 | Brick-on-edge, at Rs. 25 per 100 ... ... ... | 385•75 |
| Lineal feet | 150 | Girder bridge, at Rs. 90.21 per foot ... ... ... | 13,531-50 |
|  |  | 7' towing path ... ... ... ... ... ... | 377.30 |
|  |  | Grand Total | 16,031.05 |

(54)

ESTIMATE No. 35.—ADDITIONS TO SOUNDA BRIDGE.

\begin{tabular}{|c|c|c|c|c|c|c|c|}
\hline Description. \& No. \& L. \& B. \& D. \& Quantity. \& Total. \& Grand Total. <br>
\hline Demolition, old Masonry. \& \& \& \& \& \& \multirow[b]{3}{*}{16,543} \& \multirow[b]{7}{*}{16,543

5,141} <br>

\hline | Vide Estimate No. 34 |
| :--- |
| Deduct- | \& ... \& $\cdots$ \& ... \& ... \& 21,174 \& \& <br>

\hline Flooring .. ... ... \& ... \& \multirow[t]{2}{*}{$\cdots$} \& \multirow[t]{2}{*}{$\cdots$} \& \multirow[t]{2}{*}{$\cdots$} \& 4,630.5 \& \& <br>
\hline Masonry \& \& \& \& \& \& \multirow{4}{*}{5,141} \& <br>

\hline | Tide Estimate No. 34 ... |
| :--- |
| Deduct- | \& ... \& $\ldots$ \& ... \& ... \& 6,685 \& \& <br>

\hline Flooring ... ... ... \& \multirow[t]{2}{*}{...} \& \multirow[t]{2}{*}{...} \& \multirow[t]{2}{*}{$\cdots$} \& \multirow[t]{2}{*}{...} \& 1,543•5 \& \& <br>
\hline \& \& \& \& \& \& \& <br>
\hline
\end{tabular}

ABSTRACT OF COST OF ESTIMATE No. 35.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 16,543 | Demolition, old masonry, at Re. 1 per 100 ... ... | $165 \cdot 43$ |
| " | 5,141 | Brick-work, at Rs. 22 per 100 ... ... ... ... | 1,131.02 |
| Lineal feet | 150 | Girder bridge, at Rs. 90.21 per foot ... ... ... | 13,531-50 |
|  |  | Towing path ... ... ... .. | 377.30 |
|  |  | Grand Total | 15,205 25 |

ESTIMATE No. 36.-ADDITIONS, \&c., TO POOTH AND NUGLA BRIDGES.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old Masonry. |  |  |  |  |  | 22,794•17 | 22,794 |
| Arches ... ... ... | 3 | 54 | 23 | 2.75 | $\begin{gathered} 10,246 \cdot 50 \\ 920 \end{gathered}$ |  |  |
| Backing of arches ... ... | 2 | 23 | 10 | 2 |  |  |  |
| Spandrils"... $\ldots$... $\ldots$ | 2 | 23 | 13 | 4. | $\begin{aligned} & 2,392 \\ & 2,037 \cdot 5 \end{aligned}$ |  |  |
|  | 2 | 163 | 2.5 1.5 | 2.5 2.87 |  |  |  |
| Pillars ... ... ... | 43 | 3 3 | 3 | 2.87 | $2,023 \cdot 35$ $103 \cdot 32$ |  |  |
| Flooring ... ... ... |  | 49 | 23 | 1.5 | 5,071:50 |  |  |
|  |  |  |  |  |  |  |  |
| Diggingbouldersout of cribs | 2 | $\begin{array}{r} 163 \\ 20 \end{array}$ | 1010 | $\begin{aligned} & 2 \cdot 23 \\ & 2 \cdot 23 \end{aligned}$ | $\begin{array}{r} 7,269 \cdot 80 \\ 892 \cdot 00 \end{array}$ | 8,161•80 |  |
|  |  |  |  |  |  |  |  |
| New Masonry. |  |  |  |  |  |  |  |
| Raising piers ... ... | 2 |  |  | 12 |  |  |  |
| " Abutments | 2 | 23 | 6.5 | 1.5 | ${ }^{3,588.5}$ |  |  |
| " Wings ... | 4 | 35 | $2 \cdot 5$ | 1.5 | 525 |  |  |
| Prapets Pillars ... | 4 | 4 | 4 | $1 \cdot 5$ | 96 |  |  |
| $\begin{array}{lll}\text { Parapets } & \text {... } & \text {... } \\ \text { Pillars } & \text {... } & \text {.. }\end{array}$ | 4 | 36 | 1.5 | 2.87 | 619.92 | 7,183•17 |  |
| $\begin{array}{llll}\text { Pillars } \\ \text { Parapets on piers } & \text {... } & & \text {... } \\ \text { ar }\end{array}$ | 4 | 3 | 3 | 2.87 | $103 \cdot 32$ |  |  |
| $\begin{array}{lll}\text { Parapets on piers } & . . . & . . \\ \text { Flooring ... } & \text {... } & \end{array}$ | 4 | $6 \cdot 5$ | $1 \cdot 5$ | 2.87 | 111.93 |  |  |
| Flooring ... ... ... | 3 | 49 | 23 | 0.5 | 1,690.50 |  |  |
|  |  |  |  |  | , |  |  |
| Brick-on-edge ... ... | 3 | 49 | 23 | $\cdot 5$ | 1,690.5 | 1,690.5 | 7,183 |
|  |  |  |  |  |  |  | 1,690 |
| Girder bridge ... ... | 3 | 50 | $\cdots$ | $\cdots$ | 150 | 150 |  |
|  |  |  |  |  |  |  | 150 |
|  |  |  |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. 36.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 22,794 | Demolition, old masonry, at Re. 1 per 100 ... ... | 227-94 |
| " | 8,162 | Digging boulders out of cribs, at Rs. 7 per 1,000 ... | 57•13 |
| " | 7,183 | Brick-work, at Rs. 22 per 100 ... ... ... | 1,580.26 |
| " | 1,690 | Brick-on-edge, at Rs. 25 per 100 ... ... | $422 \cdot 5$ |
| Lineal feet | 150 | Girder bridge, at Rs. 92.05 per foot ... ... ... | 13,807.50 |
|  |  | 7 7' towing path ... ... ... ... ... | $431 \cdot 00$ |
|  |  | Grand Total | 16,526.33 |

ESTIMATE No. 37.-ADDITIONS, \&c., TO BHOLA FALLS.

| Description. | No. | L. | B. | D. | Quantity. | Total | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Excavation. |  |  |  |  |  |  |  |
| Fide Estimate No. 32 Demolition, old Masonry. | ... | $\cdots$ | ... | ... | ... | ... | 612,490 |
| At head of lock channel, ride Estimate No. 17 | $\cdots$ | ... | ... | $\cdots$ | 22,713•50 |  |  |
| Round ends of chamber walls, vide Estimate No. 17 | ... | ... | ... | ... | 8,830 |  |  |
| Loek bridge, vide Estimate No. 17 ... ... ... | $\ldots$ | ... | $\ldots$ | $\ldots$ | 2,582.99 | 34,126.49 |  |
| Masonry. |  |  |  |  |  | 34,126 4 | 34,126 |
| Raising lock walls, vide Estimate No. 29 | ... | ... | $\cdots$ | ... | 6,246 |  |  |
| Lock bridge, vide Estimate No. 17 | ... | ... | ... | $\ldots$ | 1,542•59 |  |  |
| Fide Estimate No. 32 ... | $\ldots$ | $\ldots$ | ... | ... | 41,130.53 |  |  |
| Deduct- |  |  |  |  |  |  |  |
| Openings and arches of piers, vide Estimate No. 32 ... | ... | ... | $\cdots$ | ... | ... | 13,470.97 |  |
| In fall, vide Estimate No. 32 | ... | ... | ... | .. | 2,126 |  |  |
| In lock, vide Estimate No. 17 | ... | ... | ... | ... | 504 |  |  |
| Stone flooring. |  |  |  |  |  |  |  |
| Tide Estimate No. 32 ... | $\ldots$ | ... | ... | ... | 6,131 | 6,131 |  |
| Grating girders, wall-plates, pitching, vide Estimate No.32. |  |  |  |  |  |  |  |

ABSTRACT OF ESTIMATE No. 37.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 612,490 | Excavation, at Rs. $2 \cdot 25$ per 1,000 ... ... ... | 1,378•10 |
| ग | 34,126 | Demolition, old masonry, at Re. 1 per 100 ... | $341 \cdot 26$ |
| \% | 35,448 | Brick-work, at Rs. 22 per 100 ... ... ... ... | 7,798.56 |
| " | 2,630 | Arch-work, at Rs. 26 per 100 ... ... ... ... | 683.8 |
| " | 6,131 | Stone-flooring, at Rs. 2 per 100... ... ${ }^{\text {. }}$. ${ }^{\text {.. }}$ | 12,262.00 |
| \% |  | Grating girders and wall-plates, vide Estimate No. 32 ... | 7,595•7 |
| " | 6,000 |  | $600 \cdot 00$ |
| \% |  | New rajbuha head ... ... ... ... ... | $1,410.78$ |
|  |  | Grand Total | 32,070 20 |

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(57)
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ESTIMATE No. 38.-ADDITIONS, \&c., TO JANEE KHOORD BRIDGE.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old Masonry. | ... |  | ... | ... | 22,794 | 17,722 | 17,722 |
| Tide Estimate No. 36 ... |  | ... |  |  |  |  |  |
| Deduct- | $\cdots$ | $\cdots$ | $\cdots$ |  |  |  |  |
| Flooring ... ... ... |  |  |  | $\cdots$ | 5,071•5 |  |  |
| Masonry. |  |  |  |  |  |  |  |
| Tide Estimate No. 36 Deduct- | $\cdots$ | $\cdots$ | $\cdots$ | $\cdots$ | 7,183 |  |  |
| Flooring ... ... ... | $\cdots$ | $\cdots$ | -• | $\cdots$ | 1,690.5 |  |  |
|  |  |  |  |  |  | 5,482 | 5,492 |

ABSTRACT OF ESTIMATE No. 38.

( 58 )
ESTIMATE No. 39.-ADDITIONS, \&c., TO $\left\{\begin{array}{l}\text { 1. RAOLI BRIDGE, } \\ \text { 2. JARSHA " } \\ \text { 3. PUKKANA " } \\ \text { 4. MAMNIE " } \\ \text { 5. SUHENDA ". }\end{array}\right.$


ABSTRACT OF COST OF ESTIMATE No. 39.


$$
\text { ( } 59 \text { ) }
$$

ESTIMATE No. 40.-ADDITIONS, \&C., TO PEEPULHERA BRIDGE.

\begin{tabular}{|c|c|c|c|c|c|c|c|}
\hline Description. \& No. \& L. \& B. \& D. \& Quantity. \& Total. \& Grand Total. <br>
\hline Demolition, old Masonry. \& \& \& \& \& \& \multirow[b]{5}{*}{17,208} \& \multirow[b]{11}{*}{17,208

9,153} <br>
\hline Fide Estimate No. 39 ... \& ... \& ... \& ... \& ... \& 16,091 \& \& <br>
\hline Additional in arches ... \& 3 \& 48 \& 2 \& 2.5 \& 720 \& \& <br>
\hline  \& 2 \& 7-5 \& 2 \& 2 \& 60 \& \& <br>
\hline " in flooring ... \& 3 \& 45 \& 2 \& $1 \cdot 25$ \& 337-5 \& \& <br>
\hline Masonry. \& \& \& \& \& \& \multirow{6}{*}{9,152•5} \& <br>
\hline Tide Estimate No. 39 ... \& $\cdots$ \& $\cdots$ \& ... \& ... \& 8,489 \& \& <br>
\hline Additional in piers . ... \& 2 \& 2 \& $5 \cdot 5$ \& $10 \cdot 5$ \& 231 \& \& <br>
\hline \% in abutments ... \& 2 \& 2 \& 5.5 \& 1.5 \& $27 \cdot 5$ \& \& <br>
\hline \% in floorings ... \& 8 \& 45 \& 2 \& 1.5 \& 405 \& \& <br>
\hline \& \& \& \& \& \& \& <br>
\hline
\end{tabular}

ABSTRACT OF COST OF ESTIMATE No. 40.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cabic feet | 17,208 | Demolition, old masonry, at Re. 1 per 100 ... ... | 172.08 |
| " | 9,153 | Kunkur masonry, at Rs. 20 per 100 ... ... ... | 1,830.60 |
| Lineal feet | 135 | Girder bridge, at Rss. $92 \cdot 05$ per foot ... ... ... | 12,426.75 |
|  |  | Towing path ... ... ... ... ... ... | $431 \cdot 00$ |
|  |  | Grand Total | 14,860.43 |

ESTIMATR No. 41.-ADDITIONS, \&c., TO BUOLUNDSHUHCR BRANCH HEAD.
Mank Lise.


## ( 61 )

ESTIMATE No. 41.-ADDITIONS, \&c., TO BOOLUNDSHUHUR, \&c.-continued.

| Description. | No. | L. | B. | D. | Quantity. | 'Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upright girders for regulator | 3 12 | $\begin{aligned} & 11 \cdot 50 \\ & 15 \end{aligned}$ | $\ldots$ | $\ldots$ | 84 180 |  | 214 |
| Iron drop gates ... ... | 2 | 20.50 | ... | 10 | 410 | 410 |  |
| Wooden gates ... ... | 18 2 3 | 6.66 7.50 6.66 | … $\cdots$ $\cdots$ | 10 8 8 | $\begin{gathered} 1,198 \cdot 80 \\ 120 \\ 159.84 \end{gathered}$ |  | 410 |
| Chain (light) ... ... | 46 | 16.50 | $\cdots$ | $\cdots$ | 759 |  | 1,479 |
| Pitcking slopes ... ... | $\begin{aligned} & 2 \\ & 2 \end{aligned}$ | $\begin{aligned} & 50 \\ & 50 \end{aligned}$ | $\begin{aligned} & 20 \\ & 10 \end{aligned}$ | 1 | $\begin{aligned} & 2,000 \\ & 1,000 \end{aligned}$ |  | 759 |
| Metalling roadway. |  |  |  |  |  |  | 3,000 |
| Relaying ... ... ... | ... | 526 | $\cdot 15$ | -75 | 5,917•5 | 5,917 |  |
|  |  |  |  |  |  |  | 5,917 |

ABSTRACT OF ESTIMATE No. 41.


ESTIMATE No. 42.-ADDITIONS, \&c., TO NIDHOOLI, GESOOPOOR, AND SUNOWTA BRIDGES.

\begin{tabular}{|c|c|c|c|c|c|c|c|}
\hline Description. \& No. \& L. \& B. \& D. \& Quantity. \& Total. \& Grand Total <br>
\hline Demolition, old masonry, vide Estimate No. 39 ... Deduct- \& ... \& $\cdots$ \& $\cdots$ \& $\cdots$ \& 16,091 \& \multirow[b]{2}{*}{12,547} \& \multirow{4}{*}{12,547} <br>
\hline Flooring ... ... ... \& $\cdots$ \& $\cdots$ \& $\cdots$ \& $\cdots$ \& 3,543•75 \& \& <br>
\hline New masonry, vide Estimate No. 39 ... \& \& \& \& \& 8,489 \& \& <br>
\hline Raising water-wings ... \& 2
2 \& 27
100 \& 2 \& 1.5
1.5 \& 162
600 \& \& <br>
\hline Deduct flooring, vide Estimate No. 39 \& 2
$\cdots$ \& 100
$\ldots$ \& 2 \& 1.5

. \& 600

$\ldots$ \& $$
\begin{aligned}
& 9,251 \\
& 4,252 \cdot 5
\end{aligned}
$$ \& <br>

\hline \& \& \& \& \& \& \& 4,998.5 <br>
\hline
\end{tabular}

ABSTRACT OF ESTIMATE No. 42.


ESTIMATE No. 43.-ADDITIONS, \&c., TO DAMKOURA BRIDGE.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old masonry, vide Estimate No. 42 <br> Masonry, vide Estimate No. 39 <br> Raising water-wings <br> Deduct flooring, vide Estimate No. 39 <br> ... | ... | ... | ... | $\cdots$ | ... | ... | 12,547 |
|  | $\ddot{2}$ | 175 | 2 | 1:5 | 8,489 1,050 |  |  |
|  | ... | ... | -• | ... | ... | 4,252 |  |
|  |  |  |  |  |  |  | 5,287 |

ABSTRACT OF ESTIMATE No. 48.

( 63 )
ESTIMATE No. 44.-ADDITIONS, \&c., TO WALLIPOORA BRIDGE.


ABSTRACT OF ESTIMATE No. 44.


ESTIMATE No. 45.-FALLS AT WALLIPOORA, NUGLA, ABOOPOOR, JARCHA and pukkana.

| Description of Work. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Foundations. |  |  |  |  |  |  |  |
| Water-wings up-stream ... | 2 | 25 | $5 \cdot 5$ | 8 | 2,200 |  |  |
| Curtain ... | 2 | 16 | 3 | 5 | 480 |  |  |
| Drop walls ... ... ... | 2 | 16 | 3 | 10.25 | 984 |  |  |
| " . $\quad .$. | 2 | 16 | 3 | 4 | 384 |  |  |
| Curtains under lower gates | 4 | 16 | 3 | 8 | 576 |  |  |
| Sides of wells $\ldots$... | 2 | 14 | 6 | 4 | 672 |  |  |
| Abutments of gates upstream ... ... .. | 2 | $16 \cdot 5$ | 6.5 | 8 | 1,716 |  |  |
| St ${ }^{\text {d }}$ down-stream ... | 2 | 16.5 | 12 | 2.5 | 990 |  |  |
| Side walls of locks $\quad$.. | 2 | 106 | 9 | $2 \cdot 5$ | 4,770 |  |  |
| Abutments of bridge ... | 2 | 3 | 6 | $2 \cdot 5$ | 90 |  |  |
| Side wall down-stream ... | 2 | 50 | 6 | $2 \cdot 5$ | 1,500 |  |  |
| Flooring up-stream ... | 2 | 16 | 17 | 3 | 1,632 |  |  |
| " of well .. | 2 | 16 | $7 \cdot 25$ | 3 | 696 |  |  |
| Ba, under gates $\quad \cdots$ | 2 | ${ }_{16}^{16}$ | $10 \cdot 5$ | 2.5 | 840 |  |  |
| Backing of inverts ... | 2 | 168.5 | 16 | $1 \cdot 75$ | 9,436 |  |  |
| Division wall of lock ... | 2 | 18 | 7 | 8 | 2,016 |  |  |
| \# \# sides of well... | 2 | 14 | 7 | 13.25 | 2,597 |  |  |
| " \#, of lock ... | 2 | 80 | 7 | 8 | 8,960 |  |  |
| Drop"wall of fall"... | 2 | 70 | 7 | 5.5 | 5,390 |  |  |
| $\begin{array}{ll}\text { Drop wall of fall } & \text {.. } \\ \text { Curtain of fall } & \text {... } \\ \text { a }\end{array}$ | ... | 141 | $5 \cdot 5$ | 11.75 | 9,112•12 |  |  |
| Curtain of fall | ... | 141 | 3 | 4 | 1,692 |  |  |
|  | $\cdots$ | 141 | 3 | 1.5 | $634 \cdot 5$ |  |  |
|  | 18 | 141 | 64 | $1 \cdot 5$ | 13,536 |  |  |
| Wells in curtain of fall $\ldots$ | 18 | 14.12 | 1.5 | 7 | 2,668.68 |  |  |
| ", walls of basin ", division wall ... | 32 4 | $\begin{aligned} & 14 \cdot 12 \\ & 18 \cdot 8 \end{aligned}$ | 1.5 | 9 | 6,099.84 |  |  |
| " division wall ... | 4 | $.18 \cdot 8$ | ${ }^{2}$ | 9 | 1,353.60 |  |  |
| Covering of wells in curtain | 18 |  | 06 | 1.5 | 190.62 |  |  |
| $\begin{array}{ccc}  \\ \text { walls } & \text {... } & \text { division } \\ \hline \end{array}$ |  | 18 | 7 | 2 | 252 |  |  |
| basin ... ... ... | 2 | 106 | 6 | 2 | 2,544 |  |  |
| Brick-on-edge lock flooring | 2 | 16 | 17 | 1 | 544 |  |  |
| " " | 2 | 16 | $7 \cdot 25$ | 1 | 232 |  |  |
| Flo fing " ... | 2 | 16 | $10 \cdot 5$ | 1 | 336 |  |  |
| Flooring of fall ... ... | ... | 141 | 64 | 1 | 9,024 |  |  |
| Hollows in water-wings upstream ... ... ... | 2 | 25 | $1 \cdot 5$ | 4 | 300 | 10,436 |  |
| Superstructure. |  |  |  |  |  |  | 7,576 |
| Water-wings up-stream ... | 2 | 25 | $3 \cdot 5$ | 12 | 2,100 |  |  |
| Abutments of gates up$\begin{array}{ccc}\text { stream } & \cdots & \cdots \\ , \ldots & , \ldots & \\ \text { down- }\end{array}$ | 2 | 16.5 | 6 | 12 | 2,376 |  |  |
| stream ... ... ... | 2 | 16.5 | 9 | 18.25 | 5,420.25 |  |  |
| Abutment of bridge ... | 2 | 3 | 5 | 12 | 360 |  |  |
|  | 2 | 3 | 3 | $9 \cdot 5$ | 171 |  |  |
| Counterforts | 2 | 3 | $3 \cdot 5$ | 8 | 168 |  |  |
| Wing-walls of bridge | 2 | 12 | $2 \cdot 5$ | 9 | 540 |  |  |
| Side walls down-stream | 2 | 50 | $3 \cdot 5$ | 12 | 4,200 |  |  |
| Carried over | . | ... | $\cdots$ | ... | 15,335 25 |  |  |

ESTIMATE No. 45.-FALLS AT WULLIPOORA, NUGLA, ABOOPOOR, JARCHA, AND PUKKANA,-continued

estimate no. 45.-FALLS at wUllipoora, NUGLA, AbOOPOOR, JARCHA
AND PUKKANA,-continued.


ABSTRACT OF COST OF ESTIMATE No. 45.

| Quantity. |  | Deschiption. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet., | 73,576 | Kunkur masorry in foundations, at Rs. 17 per 100 ... | 12,507.92 |
| Cubio | 66,237 | , " in superstructure, at Rs. 19 per $100 \ldots$ | 12,585.03 |
| ", | 22,079 | Brick-work, at Rs. 22 per 100 ... ... | 4,857.38 |
| " | 8,341 | Arch-work Inverts, at Res. 26 per $100 \ldots$ | 2,168.66 |
| " | 1,632 | B." light, at Re. 26 per 100 ... | 424.32 |
| " | 10,431 | Brick-on-edge, at Rs. 25 per 100 ... ... | 2,607.75 |
| " | 29,610 | Concrete, at Rs. 14 per 100 ... | 4,145•40 |
| , | 11,625 | Puddle, at Rs. 10 per 1000 ... ... ... | 116.25 |
|  | 4,230 | Kunkur flooring, at Rs. 8 per 100 ... ... | 338.40 |
| Lineal feet No. | 4 |  | ${ }_{1}^{2,080} 0$ |
| Cubic feet | 120,108 | Excavation, at Rs. $2 \cdot 5$ per 1000 ... ... ... | $300 \cdot 27$ |
| Square feet | 1,040 | Lock gates, at Rs. 6.5 per foot ... | 6,760.00 |
| qur |  | Pulleys, crabs, and sluices, as Estimate No. 26 ... | 2,120.00 |
| Lineal feet | 220 | Chain, at Re. 1 per foot ... ... | $220 \cdot 00$ |
| No. | 63 | Sleepers, at Rs. 11.625 each | $732 \cdot 37$ |
|  |  | Hoops, at Rs. 7 each $\ldots^{-} . .$. | 14.00 |
| Square feet | 3,600 | Grating, at Re. 1.15 per foot ... | 4,140.00 |
| Lineal feet. | 408 | Girders, at Rs. $4 \cdot 25$ per foot ... ... ... | 1,734.00 |
| " | 120 | Wall-plates, at Rs. 625 per foot .. ... ... | 750.00 |
|  |  | Grand Total | 59,851 75 |

$$
\begin{gathered}
(67) \\
\text { ESTIMATE No. 46.-FALLS AT MOONDA KHERA. }
\end{gathered}
$$

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Masonry. |  |  |  |  |  | 3,723•3 | 3,728126 |
| In wells of curtain of basin ... | 18 | 14.12 | 1.5 | 7 | 2,668.68 |  |  |
| Covering of ditto ... ... | 18 | $7 \cdot 6$ |  | $1 \cdot 5$ | 19.262 |  |  |
| Cartain on wells ... .. | ... | 144 | 3 | 2 | 864 |  |  |
|  |  |  |  |  | 126 |  |  |
| Well sinking ... ... ... | 18 | 7 | ... | ... |  | 126 |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |

ABSTRACT OF ESTIMATE No. 46.


ESTIMATE No. 47.-ALTERATIONS TO URROWLI BRIDGE.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand. Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old masonry, ride Estimate No. 44 | $\cdots$ | $\cdots$ | ... | $\ldots$ | 20,002 |  |  |
| Dedact flooring ... ... | 3 | 45 | 28 | 1.25 | 4,725 | 15,277 |  |
| Fide Estimate No. 44 ... <br> Raising water-wings ... | $\ddot{2}$ | 143 | 2 | 2.5 | $\begin{array}{r} 10,811 \\ 1,430 \end{array}$ |  |  |
| Dedact flooring ... ... | 3 | 45 | 28 | 1.5 | $\ldots$ | 5,670 |  |

ABSTRACT OF OF ESTIMATE No. 47.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cabic feet <br> Lineal' feet | 15,277 | Demolition, old masonry, at Re. 1 per 100 | 152.77 |
|  | 6,571 | Kunkur masonry, at Rs. 20 per 100 ... | 1,314.2 |
|  | 135 | Girder bridge, at Rs. $96 \cdot 65$ per foot ... .. ... Tuwing-path | 13,047.75 |
|  |  |  | 501.5 |
|  |  | Grand Total | 15,016.22 |

ESTIMATE No. 48.-ALTERATIONS TO MOONDA KHERA BRIDGE.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old masonry, vide Estimate No. 39 <br> Deduct flooring | $\ldots$ | $\cdots$ | $\cdots$ | $\cdots$ | 16,091 3,544 |  | 12,547 |
| Masonry, vide Estimate <br> No. 39 <br> Raising water-wings | $\cdots$ | 410 | $\ddot{2}$ | 2 | $\begin{aligned} & 8,489 \\ & 3,280 \end{aligned}$ | 11,769 |  |
| Deduct flooring, vide Estimate No. 39 | $\cdots$ | '.. | ... | .. | ... | 4,252 | 7,517 |
|  |  |  |  |  |  |  |  |

ABSTRACT OF ESTIMATE No. 48.


ESTIMATE 49.-ALTERATIONS TO UCHEHJA BRIDGE.

| Description. | No. | L. | B. | D. | Quantity. | Total | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old masonry, vide Estimate No. 42 | $\cdots$ | -• | $\cdots$ | $\cdots$ | ... | ... | 12,547 |
| Masonry, vide Estimate No. 39 | . |  |  |  | 8,489 |  |  |
| Raising water-wings $\quad . .$. | 2 | 205 | $\ddot{2}$ | $\dddot{1} 5$ | 1,230 |  |  |
| Deduct flooring, vide Estimate No. 39 | -• | ... | -• | ... | - | 4,252 | 5,467 |
|  |  |  |  |  |  |  |  |

ABSTRACT OF ESTIMATE No. 49.


## ( 69 )

ESTIMATE No. 50.-ALTERATIONS TO PULRA FALLS.


## ( 70 )

ESTIMATE NO. 50.-ALTERATIONS TO PULRA FALLS,-continued.


ABSTRACT OF ESTIMATE No. 50.

| Quantity. |  | Description. | Total |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 323,450 | Excavation, at Re. 2.25 per 1,000 ... ... | 727.76 |
| " | 85,639 | Demolition, old masonry, at Re. 1 per 100 | 856.39 |
| " | 24,480 | Lifting and relaying kunkur flooring, at Rs. 7 per 1,000 | 171.36 |
| " | 29,426 | Kunkur masonry, at Rs. 17 per 100 ... ...' ... | 5,002•42 |
| " | 9,809 | Brick-work, at Rs. 22 per 100 ... ... ... | 2,157.98 |
| " | 1,636 | Arch-work, at Rs. 26 per 100 ... ... ... | $425 \cdot 36$ |
| " | 8,792 | Brick-on-edge, at Rs. 25 per 100 ... ... ... | 2,198.00 |
| " | 1,500 | Stone flooring, at Rs. 2 per foot. . ... ... ... | 3,000.00 |
| " | 1,500 | Pitching, at Rs. 10 per 100 ... ... ... | 150.00 |
| Square feet | 1,807 | Grating, at Rs. $1 \cdot 15$ per foot ... ... ... | 1,848.05 |
| Lineal feet | 210 | Girders, at Rs. 4.25 per foot ... ... ... | $892 \cdot 50$ |
| " | 85 | Wall-plates, at Rs. 6.25 per foot ... ... ... | 531-25 |
|  |  | Rajbuha head A. I. ... ... . ... ... ... | 1,410.78 |
|  |  | Grand Totar | 19,371-85 |

## ( 71 )

ESTIMATE No. 51.-ALTERATIONS TO KOEL BRANCH HEAD.

| Description. | No. | L. | B. | D. | Quanttry | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old masonry, vide Estimate No. 41 <br> New masonry. | . | $\cdots$ | $\ldots$ | $\cdots$ | ... | ... | 2,700 |
| Abutments ... ... ... | 2 | 18 | 6 | 10 | 2,160 |  |  |
| Backing of arches ... | 4 | 18 | 10 | 15 3 | 1,620 2,160 |  |  |
| \% | 2 | 18 | 78 | ${ }_{8}^{8}$ | 2,160 |  |  |
| Raising spandrils \& roadway | 4 | 92 |  | $5 \cdot 5$ | 4,048 |  |  |
| " " | 4 | 43 |  | 5.5 | 1,892 |  |  |
| Raising piers "... .... | 3 | 135 22 | ${ }_{4}^{3 \cdot 33}$ | 1 | 1,350 |  |  |
| Roising retaining $\ldots$ walls ${ }^{\text {of }}$ | 22 | 6 | 3 | 5 | 1,980 |  |  |
| Roising retaining walls of platform |  | 91 | 2 | 3 | 546 |  |  |
| " pillars ... | 4 | 6.25 | $6 \cdot 25$ | 3 | 468.75 |  |  |
| Raising water-wings down- | 2 | 125 | 2.50 | 3 | 1,875 |  |  |
| stream ... ... ... | 2 | 30 | 2 | 3 | 360 |  |  |
| New wings and steps ... | 6 | 18.50 | 3 | 11 | 2,673 |  |  |
| Look walls | 4 | 17 | 3 | 1.50 | 306 |  |  |
| ${ }_{\text {Curtains }} \quad . . . \quad \ldots \quad . .$. | $\cdots$ | 127 | 6 | ${ }_{6}^{16.50}$ | 25,146 270 |  |  |
| ". $\quad .$. | ... | 15 | 2 | 4 | 120 |  |  |
| Flooring … ... ... | ... | 18 | 16 |  | 288 |  |  |
| Backing of inverts ${ }_{\text {Water-wings of }}$ | $\cdots$ | 16 | 109 | 1.75 | 3,052 |  |  |
| Water-wings of lock ... | ... | 27 |  | 6 | 648 |  |  |
| " " ... | $\ldots$ | 30 | 4 | 6 | 720 |  |  |
| " " ... | ... | 27 | $2 \cdot 5$ | $10 \cdot 5$ | 708.75 |  |  |
| Weir ${ }^{\text {in }}$ branch ${ }^{\text {heead }}$.... | ... | 30 35 | 2.5 | 2 | 525 |  |  |
| Raising wing-walls | 8 | 20 | $2 \cdot 50$ | ${ }_{5}^{2}$ | 2,200 |  |  |
| Parapets Pillars | 8 | 20 | 1.5 | $2 \cdot 25$ | 540 |  |  |
| Pillars ... ... ... | 8 | , | 3 | $2 \cdot 25$ | 162 |  |  |
| Deduet hollow part of walls | ... | 90 | 2 | 12.5 | 2,250 |  |  |
| Areking Inrerts, brick-onedge, same as Estimate No. 41. |  |  |  |  |  |  | 56,242 |
| Lock gates ... ... | 2 | $9 \cdot 54$ | ... | $\left\{\begin{array}{l} 7 \cdot 5 \\ 9 \cdot 5 \end{array}\right.$ | \} 324:36 | 324:36 |  |
| Crabs, Pulleys, Sluices, Chain, as Estimate No. 41. |  |  |  |  |  |  | 324 |
| Opright girders ... ... | $3$ | $\begin{aligned} & 15 \\ & 11.50 \end{aligned}$ | $\ldots$ | $\cdots$ | $\begin{gathered} 120 \\ 34 \cdot 5 \end{gathered}$ |  |  |
|  |  |  |  |  |  |  | 154 |
| Iran arop gaks $\quad .$. | 2 | $20 \cdot 50$ | ... | 95 | $389 \cdot 5$ | 389 |  |
| Wooden gates ... | $\begin{array}{r} 12 \\ 2 \\ \mathbf{3} \end{array}$ | $\begin{gathered} 6 \cdot 66 \\ 7 \cdot 50 \\ 6 \cdot 66 \end{gathered}$ | $\begin{aligned} & \ldots \\ & \ldots \\ & \ldots \end{aligned}$ | $\begin{aligned} & 9.5 \\ & 7.5 \\ & 7.5 \end{aligned}$ |  | 1,021 | 389 |
|  |  |  |  |  | 112.50 |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  | 1,021 |

$$
(72)
$$

ESTIMATE No. 5l.-ALTERATIONS TO KOEL BRANCH HEAD,-continued.


ABSTRACT OF ESTIMATE No. 51.


$$
\text { ( } 73 \text { ) }
$$

ESTIMATE No 52.-ALTERATIONS TO BURROWLEE, DUBTHULLA, AND CHUNGEYREE BRIDGES.


ABSTRACT OF ESTIMATE No. 52.

| Quantity. |  | Drscription. | Total |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cabic feet | 18,903 | Demolition, old masonry, at Rs. 1 per 100 '... ... | $139 \cdot 03$ |
| " | 8,071 | Kunkur masonry, at Rs. 20 per 100 ... ... | 1,794.2 |
| Lineal feet | 120 | Girder bridge, at Rs. $90 \cdot 21$ per foot ... ... ... | 10,825.00 |
|  |  | Towing-path ... ... ... ... ... ... | $377 \cdot 3$ |
|  |  | Grand Total | 13,135.53 |

ESTIMATE No. 53.-FALLS AT BURROWLEE AND CHUNGEYREE.


ESTIMATE No. 53.-FALLS AT BURROWLEE AND CHUNGEYREE,-continued.


ESTIMATE No. 53.-FALLS AT BURROWLEE AND CHUNGEYREE,-continued.


ABSTRACT OF COST OF ESTIMATE No. 53.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 68,533 | Kunkur masonry in foundations, at Rs. 17 per 100 ... | $\frac{\text { Rs, }}{11,650.61}$ |
| , | 49,540 | , ", superstructure, at Rs. 19 per $100 .$. | 9,412.60 |
| " | 24,082 | Brick-work, at Rs. 22 per $100 \ldots$... ... ... | 5,298.01 |
| ", | 1,368 | Arch-work, light, at Rs. 26 per 100 | 355.68 |
| ", | 8,341 | ") inverts, at Rs. 26 per 100. | 2,168.66 |
| ", | 9,858 | Brick-on-edge, at Rs. 25 per 100 . | 2,464:50 |
| " | 24,723 | Concrete, at Rs. 14 per 100 ... ... | 3,461-22 |
| " | 9,160 | Puddle, at Rs. 10 per $1000 \quad \ldots$ | 91.60 |
|  | 3,690 | Kunkur flooring, "dry," at Rs. 8 per 100 ... | 295.50 |
| Lineal feet | 465 | Well sinking, at Rs. 5 per foot ... ... ... | 2,325.00 |
| No. | 55 | Crabs, at Re. 20 each ... ... ... | 1,100.00 |
| Cubic feet | 108,444 | Excavation, at Rs. $2 \cdot 5$ per 1000 | $271 \cdot 11$ |
|  | 2,400 | Pitching, at Rs. 10 per 100 ... | 240.00 |
| Squarefeet. | 921 | Lock gates, at Rs. 6.5 per foot ... | 5,986.50 |
| No. | 8 | Pulleys, at Rs. 5 each ... ... | $40 \cdot 00$ |
| , | 8 | Crabs, at Rs. 135 each ... ... | 1,080.00 |
|  | 8 | Sluices, at Rs. 125 each ... ... | 1,000.00 |
| Lineal feet. | 212 | Chain, at Re. 1 per foot ... ... | 212.00 |
| No. | 57 | Sleepers, at Rs. 11,625 each | $662 \cdot 62$ |
| Square feet | 2,835 | Grating, at Rs. $1 \cdot 15$ per foot | 3,260.25 |
| Lineal feet | 357 | Girders, at 4.25 per foot ... | 1,517.25 |
|  | 195 | Wall-plates, at Rs. 6.25 per foot... | 656.25 |
| No. | 2 | Sleeper hooks, at Rs. 7 each ... .. | 14.00 |
|  |  | Grand Total | 53,563.09 |

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ESTIMATE No. 54.-HUSIMPOOR FALLS.

|  | Description. |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

ESTIMATE No. 55.-ADDITIONS TO SIMRA FALLS.


ABSTRACT OF COST OF ESTLMATE No. 55.


ESTIMATE No. 56.-ADDITION:S TO KASUNPOOR BRIDGE.

| Description of Work. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, old masonry. <br> Tide Estimate No. 58 ... <br> Masonry, as Estimate No. 58 $\qquad$ <br> Raising ghats $\qquad$ | $\cdots$$\cdots$$\cdots$ | $\ddot{416}$ | $\cdots$ | $\cdots$$\cdots$3.6 | $\begin{aligned} & 5, \dddot{911} \\ & 5,990 \cdot 4 \end{aligned}$ | ... | 10,753 |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  | 1 |  |

ABSTRACT OF COST OF ESTIMATE No. 56.


ESTIMATE No. 57.-ADDITIONS TO BAROTHA AND SHEKLA BRIDGES.


ABSTRACT OF COST OF ESTIMATE No. 57.


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ESTIMATE No. 58.-ALTERATIONS TO DUOPOOR AND MACHOOA BRIDGES.


ABSTRACT OF COST OF ESTIMATE No. 58.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Cubic feet. <br> Lineal" feet | $\begin{array}{r} 10,753 \\ 5,191 \\ 120 \end{array}$ | Demolition, old masonry, at Rs. 1 per 100 |  | Rs. |
|  |  |  |  | $107 \cdot 53$ |
|  |  | Kunkur masonry, at Rs. 20 per 100 ... | . | 1,038.20 |
|  |  | Girder bridge, at Rs. $90 \cdot 21$ per foot | - ... | 10,825.00 |
|  |  | Towing-path ... ... ... | ... ... | $377 \cdot 30$ |
|  |  |  | Grand Total | 12,348.03 |

ESTIMATE No. 59.-ADDITIONS, \&c. TO NUNOON REGULATOR.



ABSTRACT OF ESTIUATE No. 39.


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\text { ( } 81 \text { ) }
$$

ESTIMATE No. 60.-ALTERATIONS, \&c., TO KEYLUMPOOR BRIDGE. Cawnpoor Brance.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition. |  |  |  |  |  |  |  |
| Arches ... | 3 | 35 | 21 | $2 \cdot 5$ | 5,512.5 |  | 9,376 |
| Spandrils ... ... ... | 12 | 18.75 | $2 \cdot 5$ | $2 \cdot 5$ | 1,406.25 |  |  |
| Parapets ... ... ... | 2 | 108 | 15 | 3 | 972 |  |  |
| Ogees ... ... ... | 3 | 33 | 5 | 3 | 1,485 |  |  |
| Masonry. |  |  |  |  |  | 9,375•75 |  |
| Raising piers and abutments | 443 | $\begin{aligned} & 21 \\ & 45 \\ & 33 \end{aligned}$ | $\begin{aligned} & 4 \cdot 5 \\ & 1 \cdot 5 \\ & 8 \end{aligned}$ | $\begin{aligned} & 6.5 \\ & 3 \\ & 0.5 \end{aligned}$ | $\begin{array}{r} 2,457 \\ 81 \\ 396 \end{array}$ | 2,934 |  |
| Parapets on piers ... ... |  |  |  |  |  |  |  |
| Flooring ... ... ... |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  | 2,984 |

ABSTRACT OF ESTIMATE No. 60.

| Quantity. |  | Drscription. |  |  |  | $\begin{gathered} \text { Toral. } \\ \hline \text { Rs. } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |
| Cabic feet <br> Lineal feet | 9,376 | Demolitions, at Re. 1 per 100 <br> Kunkur masonry, at Rs. 19 per 100 <br> Girder bridge, at Rs. 74.14 per foot <br> Towing-path |  |  |  | 93.76 |
|  | 2,934 |  |  | ... |  | $557 \cdot 46$ |
|  | 99 |  |  |  | . | 7,839•86 |
|  |  |  |  |  |  | 327.30 |
|  |  | Grand Total |  |  |  | 8,318.88 |

ESTIMATE No. 61.-ALTERATIONS, BRIDGES, 3 ARCHES OF $33^{\prime}-18^{\prime}$ ROAD-WAY.

| Description. | No. | L. | B. | D. | Quantity. | Total | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolitions, vide Estimate, No. 60, deduct Ogee | ... | ... | $\ldots$ | $\cdots$ | $\cdots$ | ... | 7,891 |
| Masonry, vide Estimate, - No. 60, deduct Flooring... | $\cdots$ | ... | $\cdots$ | ... | ... | ... | 2,538 |

ABSTRACT OF ESTIMATE No. 61.

| Quantity. |  | Description. |  | Total. |
| :---: | :---: | :---: | :---: | :---: |
| Cabic feet <br> Lineal feet | $\begin{array}{r} 7,891 \\ 2,538 \\ 99 \end{array}$ | Demolitions, at Re. 1 per 100 <br> Kunkur masonry, at Res. 19 per 100 <br> Girder Bridge, at Rs. 74.14 per foot <br> Towing-path <br> ... ... ... |  | Rs. |
|  |  |  |  | 78.91 |
|  |  |  | $\ldots$ | 782.22 |
|  |  |  | $\ldots$... ... ... | 7,339.86 |
|  |  |  | ... | 327-30 |
|  |  |  | Grand Total | 8,228-29 |

ESTIMATE No. 62.-ALTERATIONS, BRIDGES OF 3 ARCHES OF 33'-20' ROAD-WAY.


ABSTRACT OF ESTIMATE No. 62.

| Quantity. |  | Description. | , |  | Total. |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cubic feet <br> Lineal feet | $\begin{array}{r} 8,416 \\ \mathbf{2 , 7 7 2} \\ 99 \end{array}$ | Demolitions, at Re. 1-per 100 <br> Kunkur masonry, at Rs. 19 per 100 <br> Girder bridge, at Rs. 75.98 per foot | ...$\ldots$$\ldots$$\ldots$ |  | Rs. |
|  |  |  |  | $\ldots$ | $84 \cdot 16$ |
|  |  |  |  | ... | 526.68 |
|  |  |  |  | ... | 7,522.02 |
|  |  |  |  |  | 368.50 |
|  |  |  |  | and | 8,501•36 |

ESTIMATE No. 63.-ALTERATIONS, BRIDGES, 3 ARCHES OF 32'-18' ROAD-WAY.

| Quantity. |  | Description. | Toral. |
| :---: | :---: | :---: | :---: |
| Lineal feet | 96 |  | Rs. |
|  |  | Demolition and masonry, vide Estimate, No. 61 ... Girder bridge, at Rs. 74.14 per foot | $561 \cdot 13$ $7,117 \cdot 44$ |
|  |  | $\mathrm{G}_{\text {rand }}$ Total | 8,005•87 |

ESTIMATE No. 64.-ALTERATIONS, BRIDGE, 3 ARCHES OF 31'—18' ROAD-WAY.

| Quantity. |  | Description. | Total |
| :---: | :---: | :---: | :---: |
| Lineal feet | 93 |  | Rs. |
|  |  | Demolitions and masonry, vide Estimate No. 61 Girder bridge, at Rs. $74 \cdot 14$ per foot ... $\ldots$ | $561 \cdot 13$ |
|  |  | Towing-path ... ... ... | 327-30 |
|  |  | Grand Total | 7,783•45 |

ESTIMATE No. 65.-1 BRIDGE, 3 OF $30^{\prime}-18^{\prime}$ ROAD-WAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Lineal feet | 90 | Demolition, masonry Vide Estimate No. 61 <br> Girder bridge, at Rs. 74.14 per foot <br> Towing-path | $\begin{array}{r} 561 \cdot 13 \\ 6,672 \cdot 60 \\ 377 \cdot 30 \end{array}$ |
|  |  | Grand Total | 7,611.03 |

ESTIMATE No. 66.-1 BRIDGE, 3 OF $30^{\circ}-20^{\circ}$ ROAD.WAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Lineal feet | 90 |  | $\begin{array}{r} 610 \cdot 84 \\ 6,838 \cdot 20 \\ 368 \cdot 50 \end{array}$ |
|  |  | Grand Total | 7,817.54 |

ESTIMATE No. 67.-1 BRIDGE 3 OF $29^{\prime}-18^{\prime}$ ROAD-WAY.

| Description of Work. | No. | L. | B. | D. | Quantity. | Total. | Grand <br> Total. |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, Arches | .. | 3 | 29 | 21 | $2 \cdot 5$ | $4,567 \cdot 5$ |  |  |
| Spandrils <br> Parapets | $\ldots$ | 12 | 16.75 | $2 \cdot 5$ | $2 \cdot 5$ | $1,256 \cdot 25$ |  |  |
|  |  |  |  |  |  |  |  | $6,687 \cdot 75$ |

ABSTRACT OF COST, ESTIMATE No. 67.

| Quantity. |  | Description. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cabic feet <br> Lineal feet | $6,688$ | Demolition, at Re. 1 per 100 ... <br> Masonry, vide Estimate No. 61 <br> Girder bridge, at Rs. 74.14 per foot <br> Towing-path |  |  | 66.88 |
|  |  |  | ... | ... | 482.22 |
|  |  |  | ... | ... | 6,450•18 |
|  |  |  | ... | ... | 327.30 |
|  |  |  |  | ND | 7,326-58 |

ESTIMATE No. 68.-1 BRIDGE, 3 OF $29^{\prime}-20^{\prime}$ ROAD-WAY.


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ABSTRACT OF COST OF ESTIMATE No. 68.


ESTIMATE No. 69.-2 BRIDGES, 3 OF $28^{\prime}-18^{\prime}$ ROAD-WAY.


ESTIMATE No. 70.-1 BRIDGE, 3 OF $28^{\prime}-20^{\prime}$ ROAD-WAY.


ESTIMATE No. 71.-1 BRIDGE, 3 OF 27'—18' ROAD-WAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Lineal feet | 81 |  | Rs. |
|  |  | Demolition masonry, vide Estimate No. 67 ... ${ }^{\text {a }}$.. Girder bridge, at Ris. 74.14 per foot ... ... | $549 \cdot 10$ $6,005 \cdot 34$ |
|  |  | Towing-path ... ... ... ... ... ... | 327.30 |
|  |  | Grand Total | 6,881.74 |

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ESTIMATE No. 72.-1 BRIDGE, 3 OF 17'-20 ROAD-WAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Lineal feet | 81 |  | Rs. |
|  |  | Demolition, masonry, vide Estimate No. 68 ... Girder bridge, at Rs. $75 \cdot 98$ per foot $\ldots$... | 597.91 $6,154.38$ |
|  |  | Towing-path ... ... ... ... ... ... | $368 \cdot 50$ |
|  |  | Grand Total | 7,120.79 |

ESTIMATE No. 73.-2 BRIDGES, 3 OF 26'-20' ROAD-WAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Lineal feet | 78 |  | Rs. |
|  |  | Demolition, masonry, vide Estimate No. 68 | 597.91 |
|  |  | Girder bridge, at Rs. 75.98 per foot | 5,926.44 |
|  |  | Towing-path ... ... | 368.50 |
|  |  | Grand Total | 6,892•85 |

ESTIMATE No. 74.-l BRIDGE 3 OF 26 $\mathbf{0}^{\prime} \mathbf{- 2 5}$ ROAD-WAY.

| Description of Work. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, Arches $\begin{aligned} & \text { Spandrils } \\ & \text { Sader }\end{aligned}$ | 3 | 29 | 28 | $2 \cdot 5$ | 6,090 | 8,210 | 8,210 |
| $\left.\begin{array}{l}\text { Spandrils } \\ \text { Parapets }\end{array}\right\}$ vide Estimate No. 67 |  |  |  |  | 2,120•25 |  |  |
| Raising Piers and Abutments | N |  | $4 \cdot 5$ | 6.5 | 3,27681 | 3,357 |  |
|  |  | 28 |  |  |  |  |  |
| Parapets on Piers, vide Estim | te No. |  | ... | ... |  |  |  |
|  |  |  |  |  |  |  | 3,357 |

ABSTRACT OF COST OF ESTIMATE No. 74.


ESTIMATE No. 75.-1 BRIDGE, 2 OF 35'-18' ROAD-WAY.

| Description of Work. | No. | L. | B. | D. | Qunntity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolitions, Arches | 2 | 35 | 21 | $2 \cdot 5$ | 3,675 | 5,333 | 5,333 |
| Spandrils ... | 8 | 19.75 | 2.5 | $2 \cdot 5$ | c.87.5 |  |  |
| Parapets ... | 2 | 74.5 | 1.5 | 3 | $670 \cdot 5$ |  |  |
| Masonry. |  |  |  |  |  |  |  |
| Raising piers and abutments | 3 | 21 | 4:5 | 6.5 | $\begin{array}{r} 1,842 \cdot 75 \\ 40.50 \end{array}$ | 1,883 |  |
| Parapets on piers ... ... | 1 | $4 \cdot 5$ | 1.5 | 3 |  |  |  |
|  |  |  |  |  |  |  | 1,883 |

ABSTRACT OF COST OF ESTIMATE No. 75.

| Quantity. |  | Description. |  |  | Grand <br> Total |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cubic feet <br> Lineal feet | $\begin{array}{r} 5,333 \\ 1,883 \\ 70 \end{array}$ | Demolition, at Re. 1 per 100 Kunker masonry, at Rs. 19 per foot Girder bridge at Rs. $74 \cdot 14$ per foot |  |  | Rs. |
|  |  |  | ... | $\cdots$ | 53.33 |
|  |  |  | ... | ... | 357.77 |
|  |  |  | ... | ... | 5,189•80 |
|  |  |  | ... |  | $327 \cdot 30$ |
|  |  | Grand Total |  |  | 5,928.20 |

ESTIMATE No. 76.-1 BRIDGE, 2 OF 35'-20' ROAD-WAY,

| Description of work. | No. | L. | B. | D. | Quantity. | Toral | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{aligned} & 4,025 \\ & 1,658 \end{aligned}$ | 5,683 | 5,683 |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Raising piers and abutmentsParapets on piers vide Estimate |  | $\begin{array}{l\|l} 3 & 23 \\ \text { No. } 75 \ldots \end{array}$ | $4 \cdot 5$ |  | $6 \cdot 5$ | $2,018.25$40.50 | 2,059 |  |
|  |  | ... |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. 76.

| Quantity. |  | Description. |  | Grand Total. |
| :---: | :---: | :---: | :---: | :---: |
| Cubic feet Lineal feet | $\begin{array}{r} 5,683 \\ 2,059 \\ \quad 70 \end{array}$ | Demolition, at Re. 1 per 100... Masonry, at Rs. 19 per foot ... Girder bridge, at Rs. 75.98 per foot Towing-path |  | Rs. |
|  |  |  | $\cdot$ | 56.83 |
|  |  |  | ... ... | $391 \cdot 21$ |
|  |  |  | ... ... | 5,318.60 |
|  |  |  | ... ... | 368.50 |
|  |  |  | Grand Total | 6,135•14 |

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\text { ( } 87 \text { ) }
$$

ESTIMATE No. 77.-3 BRIDGES, 2 OF $33^{\prime}-18^{\prime}$ ROAD-WAY.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Lineal feet | 66 |  |  | Rs. |
|  |  | $\left.\begin{array}{l}\text { Demolition, } \\ \text { Masonry, }\end{array}\right\}$ vide Estimate No. 75 | .. ... | $411 \cdot 10$ |
|  |  | Girder bridge, at Rs. 74-14 per foot | . | 4,893.24 |
|  |  | Towing-path ... ... | . | 327.30 |
|  |  |  | Grand Total | 5,631 64 |

ESTIMATE No. 78.-2 BRIDGES, 2 OF 32'-18' ROAD-WAY.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Lineal feet | 64 | Demolition, \}ride Estimate No. 75 Masonry, , Girder bridge, at Rs. 74.14 per foot Towing-path |  | Rs. |
|  |  |  | $\ldots$... | 411.10 |
|  |  |  | ... | $\begin{array}{r} 4,744 \cdot 96 \\ 327 \cdot 30 \end{array}$ |
|  |  |  | Grand Total | 5,483 36 |

ESTIMATE No. 79.-1 BRIDGE, 2 OF 32'-20' ROAD-WAY.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Lineal feet | 64 | Demolition, \} vide Estimate No. 76 Masonry, Girder bridge, at Rs. 75.98 per foot Towing-path |  | Rs. |
|  |  |  |  | $\begin{array}{r} 448 \cdot 04 \\ 4,862 \cdot 72 \\ 368 \cdot 50 \end{array}$ |
|  |  |  | Gband Total | 5,679•26 |

ESTIMATE No. 80.-5 BRIDGES, 2 OF $30^{\prime}-18^{\prime}$ ROAD-WAY.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Lineal feet | 60 | Demolition, \} vide Estimate No. 75 Masonry, Girder bridge, at Rs. 74•14 per foot Towing-path |  | Rs. |
|  |  |  | ... | $411 \cdot 10$ |
|  |  |  | ... | $\begin{array}{r} 4,448 \cdot 40 \\ 327.30 \end{array}$ |
|  |  |  | Grand Total | 5,186.80 |

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ESTIMATE No. 81.-3 BRIDGES, 2 OF $30^{\prime}-20^{\prime}$ ROAD-WAY.

| Quantity. |  | Description. |  | Совт. |
| :---: | :---: | :---: | :---: | :---: |
| Lineal feel | 60 | Demolition, \} vide Estimate No. 76 Masonry, Girder bridge, at Rs. 75.98 per foot Towing-path |  | Rs. |
|  |  |  |  | 448.04 |
|  |  |  | $\ldots$ | $\begin{array}{r} 4,558 \cdot 80 \\ 368.50 \end{array}$ |
|  |  |  | Grand Total | 5,375•34 |

ESTIMATE No. 82.-5 BRIDGES, 2 OF $25^{\prime}-18^{\prime}$ ROAD-WAY.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Lineal feet | 50 | Demolition, $\}$ vide Estimate No. 75 Masonry, Girder bridge, at Rs. 74.14 per foot Towing-path $\qquad$ ... |  | Rs. |
|  |  |  |  | $\begin{array}{r} 411 \cdot 10 \\ 3,707 \cdot 00 \\ 327 \cdot 30 \end{array}$ |
|  |  |  | Grand Total | 4,445•40 |

ESTIMATE No. 83.-1 BRIDGE, 2 OF $25^{\prime}-20^{\prime}$ ROAD-WAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Lineal feet | 50 |  | Rs. |
|  |  | Demolition, <br> Masonry, vide Estimate No. 76 | 448.04 |
|  |  | Girder Bridge, at Rs. 75.98 per foot .. | 3,799.00 |
|  |  | Towing-path ... .. ... ... ... | 368.50 |
|  |  | Grand Total | 4,615.54 |

ESTIMATE No. 84.-ALTERATIONS, 4 BRIDGES, 2 ARCHES OF 25'-15' ROAD-WAY.

\begin{tabular}{|c|c|c|c|c|c|c|c|}
\hline Description. \& No. \& L. \& B. \& D. \& Quantity. \& Total. \& Grand Total. <br>
\hline Demolitions. \& \& \& \& \& \& \multirow{5}{*}{3,478} \& \multirow[b]{9}{*}{3,478

1,620} <br>
\hline Arches ... \& 2 \& $2 \cdot 5$ \& 18 \& $2 \cdot 5$ \& 2,250.0 \& \& <br>
\hline Spandrils ... ... ... \& 8 \& 14.75 \& 2.5 \& $2 \cdot 5$ \& $737 \cdot 5$ \& \& <br>
\hline Parapets ... ... ... \& 2 \& $54 \cdot 5$ \& \multirow[t]{2}{*}{1.5} \& 8 \& $490 \cdot 5$ \& \& <br>
\hline Masonry. \& \& \& \& \& \& \& <br>
\hline \multirow[t]{3}{*}{Raising piers and abutments Parapets on piers, vide Estimate No. 75} \& \multirow[t]{2}{*}{3
$\ldots$} \& 18 \& $4 \cdot 5$ \& 6.5 \& 1,579•5 \& \multirow[b]{3}{*}{1,620} \& <br>
\hline \& \& \& \& \& $40 \cdot 5$ \& \& <br>
\hline \& . \& \multirow{2}{*}{...} \& \multirow{2}{*}{...} \& \multirow{2}{*}{...} \& \& \& <br>
\hline \& \& \& \& \& \& \& <br>
\hline
\end{tabular}

ABSTRACT OF ESTIMATE No. 84.

| Quantity. |  | Description. |  |  | Total. |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cubic feet <br> Lineal feet | $\begin{array}{r} 3,478 \\ 1,620 \\ 50 \end{array}$ | Demolition, at Re. 1 per 100 <br> Masonry kunkur, at Rs. 19 per 100 <br> Girder loridge, at Ks. 71.38 per foot <br> Towing-path |  |  | Rs. |
|  |  |  | ... |  | 34.78 |
|  |  |  | ... | $\ldots$ | 307.80 |
|  |  |  | ... |  | 3,569.00 |
|  |  |  |  |  | $310 \cdot 50$ |
|  |  |  |  | nd | 4,222.08 |

ESTIMATE No. 85.-ALTERATIONS, 5 BRIDGES, 1 ARCH OF 30', $18^{\prime \prime}$ ROADWAY.

| Description. |  | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition. |  |  |  |  |  |  |  |  |
| Arch <br> Parapets | $\stackrel{.}{\square}$ | $\ddot{2}$ | 30 30 | ${ }_{1}^{21} 5$ | $\begin{aligned} & 2 \cdot 5 \\ & 3 \end{aligned}$ | 1,575 270 |  |  |
| Masonry. |  |  |  |  |  |  |  |  |
| Raising abutments | ... | 2 | 21 | $4 \cdot 5$ | $6 \cdot 5$ | 1,228•5 | 1,228 |  |
|  |  |  |  |  |  |  |  | 1,228 |

abStract of estimate No. 85.


ESTIMATE No. 86.-ALTERATIONS, BRIDGES 1 ARCH OF 30', 20' ROADWAY.



ESTIMATE No. 87.-ALTERATIONS, BRIDGES, 1 ARCH OF 25', 15' ROADWAY.


ABSTRACT OF ESTIMATE No. 87.


ESTIMATE No. 88.-ALTERATIONS, 1 BRIDGE, 1 ARCH OF 25',20' ROADWAY.


## ( 91 ) <br> ABSTRACT OF ESTIMATE No. 88.

| Quantity. |  | Description. |  | Total. |
| :---: | :---: | :---: | :---: | :---: |
| Cubic feet Lineal feet |  |  |  | Rs. |
|  | $\begin{array}{r} 1,662 \\ 1,345 \\ 25 \end{array}$ | Demolition, at Re. 1 per 100 <br> Kunkur masonry, at Rs. 19 per 100 Girder Bridge, at Rs. 75.98 per foot Towing-path |  | 16.62 |
|  |  |  |  | 255.55 |
|  |  |  |  | 1,899.50 |
|  |  |  |  | +368.50 |
|  |  |  |  | 2,540•17 |

ESTIMATE No. 89.-ALTERATIONS AIMA BRIDGE.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand <br> Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Masonry | 2 | 15 | 4.5 | 2.5 | 337 | 387 |  |
|  |  |  |  |  |  |  | 337 |

ABSTRACT OF ESTIMATE No. 89.

| Qoantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 337 | Kunkur masonry, at Rs. 19 per 100 <br> Towing-path $\qquad$ $\qquad$ <br> Grand Total | $\begin{gathered} \text { Rs. } \\ 64 \cdot 03 \\ 310 \cdot 50 \end{gathered}$ |
|  |  |  | 374.53 |

ESTIMATE No. 90.—ALTERATIONS UPPER RAILWAY BRIDGE.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand <br> Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Masonry. <br> Raising abutments | 2 | 33 | 4 | 2 | 528 |  |  |

ABSTRACT OF ESTIMATE No. 90.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
| Cubic feet <br> Square feet | $\begin{array}{r} 528 \\ 56 \end{array}$ | Kunkur masonry, at Rs. 19 per 100 <br> Towing-path for 25 feet roadway <br> Add planking, $8 \times 7$, at Rs. 8 per foot ... <br> Grand Total | $\begin{gathered} \text { Rs. } \\ 100 \cdot 32 \\ 426 \cdot 50 \\ 44 \cdot 80 \end{gathered}$ |
|  |  |  | 571.62 |

## ( 92 )

ESTIMATE No. 91.-ALTERATIONS, DUBOWLEE BRIDGE.


ESTIMATE No. 92.-ALTERATIONS DUBOWLEE LOCKS.


ESTIMATE No. 93.-ALTERATIONS, DAKNAPOOR BRIDGE AND OTHERS.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolitions. |  |  |  |  |  |  | 720 |
| Flooring ... ... ... |  | 20 | 20 | 1 | 400 |  |  |
| Curtains ... ... ... | 2 | 20 | 2 | 2 | 160 |  |  |
| Part of waterwings ... | 2 | 10 | 4 | 2 | 160 |  |  |
| Masonry. |  |  |  |  |  | 1.451 |  |
| Flooring ... ... ... |  | 20 | 20 | 1 | 400 |  |  |
| Curtains ... ... ... | 2 | 20 | 2 | 2 | 160 |  |  |
| Underpinning abutments... | 2 | 20 | 3 | 3 | 360 |  |  |
| Underpinning waterwings... | 2 | 20 | $1 \cdot 5$ | ${ }_{2} 138$ | 82.80 |  |  |
|  |  | 28 | 2 | 2 | 448 |  |  |
|  |  |  |  |  |  |  | 1,451 |


|  |  | $\begin{array}{cl} (93) \\ \text { ABSTRACT } & \text { OF ESTIMATE } \end{array}$ | No. 93. |  |
| :---: | :---: | :---: | :---: | :---: |
| Quantity. |  | Description. |  | Total. |
| Cubic feet <br> " | $\begin{array}{r} 720 \\ 1,451 \end{array}$ | Demolitions, at Re. 1 per 100 ... Kunkur masonry, at Rs. 19 per 100 Tow-path. | $\begin{array}{ccc} \cdots & \cdots & \cdots \\ \cdots & \cdots & \cdots \end{array}$ | $\begin{gathered} \text { Rs. } \\ 7 \cdot 20 \\ 275 \cdot 69 \end{gathered}$ |
| Number Square feet | 4 56 | Iron uprights, at Rs. 8 each Planking, at Rs. 3 per foot | $\begin{array}{ccc} \ldots & \ldots & \ldots \\ \cdots & \cdots & \cdots \end{array}$ | $\begin{aligned} & 32 \cdot 00 \\ & 44 \cdot 80 \end{aligned}$ |
|  |  |  | Grand Totai | $359 \cdot 69$ |

ESTIMATE No. 94.-ALTERATIONS, GHATS AT CAWNPOOR.


ESTIMATE No. 95.-No. 1 LOCK.

| Description of Work. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolitions. |  |  |  |  |  | 1,993 | 1,993 |
| Upper flooring of lock ... | $\cdots$ | 22 | 32 | $1 \cdot 5$ | 1,056 |  |  |
| Drop-wall ... ... | ... | 32 | 3 | $3 \cdot 5$ | 336 |  |  |
| Curtain ... | $\cdots$ | 45 | 2 | $4 \cdot 5$ | 405 |  |  |
| Counterforts | $\ddot{2}$ | 7 | 4 | 3.5 | 196 |  |  |
| . Masonry. |  |  |  |  |  |  |  |
| $\begin{array}{lll} \text { Curtain } & \ldots & \ldots \\ \text { Flooring } & \ldots & \ldots \end{array}$ | $\ldots$ | 45 | 3 | 2 | 270 | 974 |  |
|  | ... | 22 | 32 | 1 | 704 |  |  |
| Brick-on-edge. | ... | 22 | 32 | 0.5 | 352 | 352 |  |
|  |  |  |  |  |  |  | 352 |

ABSTRACT OF COST OF ESTIMATE No. 95.

| Quantity. |  | Description of Work. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cubic feet. <br> 28 | $\begin{array}{r} 1,993 \\ 974 \\ 352 \end{array}$ | Demolitions, at Re. 1 per 100 <br> Kunkur masonry, at Rs. 19 per 100 <br> Brick-on-edge, at Rs. 25 per 100 |  |  | Rs. ${ }_{19.93}$ |
|  |  |  |  |  | 185.06 |
|  |  |  |  |  | 88.00 |
|  |  |  |  | nd | 292.99 |

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\text { ( } 94 \text { ) }
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ESTIMATE No. 86.-GENERAL GUNJ BRIDGE.

| Description of Work. | No. | L. | B. | D. | Quantity. | Total. | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition. |  |  |  |  |  |  | 1,3591,370 |
| Arch ... ... ... | 2 | $\begin{aligned} & 22 \\ & 80 \end{aligned}$ | $\stackrel{23}{1 \cdot 5}$ | $\begin{aligned} & 1 \cdot 5 \\ & 2 \cdot 5 \end{aligned}$ | $\begin{aligned} & 759 \\ & 600 \end{aligned}$ | 1,359 |  |
| Parapets ... ... ... |  |  |  |  |  |  |  |
| Masonry. |  |  |  |  |  |  |  |
| Raising abutments... ... | 2 | 25 | 4. | 2 | 400 |  |  |
| " wings ... ... | 4 | 26 | 2.5 1.5 | 2 $2 \cdot 5$ | 520 450 |  |  |
| Parapets .. ... ... | 4 | 30 | $1 \cdot 5$ | $2 \cdot 5$ | 450 | 1,370 |  |
|  |  |  |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. 96.

| Quantity. |  | Description. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cubic feet | $\begin{aligned} & 1,359 \\ & 1,370 \end{aligned}$ | Demolitions, at Re. 1 per 100 ... Kunkur masonry, at Rs. 19 per 100 Girder bridge, at Rs. 75.98 per foot | $\ldots$ |  | Rs. ${ }_{13.59}$ |
|  |  |  | ... |  | $260 \cdot 30$ |
| Lineal feet | 20 |  |  |  | 1,519.60 |
|  |  | Girder bridge, at Rs. 75.98 per foot Tow-path ... |  |  | $76 \cdot 80$ |
|  |  |  |  | nd | 1,870.29 |

ESTIMATE No. 97.-LOCK No. 3.


ABSTRACT OF COST OF ESTIMATE No. 97.

| Quantity. |  | Description of Work. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet <br> " | $\begin{array}{r} 914 \\ 1,154 \\ 640 \end{array}$ | Demolitions, at Re. 1 per 100 ... Kunkur masonry, at Rs. 19 per 100 Arch-work, at Rs. 26 per 100 ... | $\begin{array}{r} \text { Rs. } \\ 9 \cdot 14 \\ 219 \cdot 26 \\ 166 \cdot 40 \end{array}$ |
|  |  | Grand Total | 394.80 |

J. CROFTON, Captain, R. E.

## ALTERNATIVE LINE.

ABSTRACT OF COST OF ESTIMATE No. 98.-EXCAVATION, \&c.

| Quantity. |  | Description. | Costs. |
| :---: | :---: | :---: | :---: |
|  |  | 1 st Section. | Rs. |
| Cabic feet | 455,134,834 | Excavation canal channel, at Ks. 2.75 per 1,000 ... | 12,51,620.79 |
| , | 53,525,227 | Embankment Kalee Nuddee, upper and lower crossings, at Ks. 8 per 1,000 | 4,28,201 $\cdot 81$ |
| " | 32,314,880 | Excavation on junction, at Re. $2 \cdot 5$ per 1,000 ... | 80,787-20 |
| " | 8,717,457 | , ${ }^{\prime}$, Kalee Nuddee escape, at Rs. 2.5 per 1,000 | 21,793.64 |
| " | 4,200,000 | Extra excavation, widening at bridges, \&c., at Rs. 2.5 per 1,000 | 9,240.00 |
| " | 125,292 | Rubble masonry wall in embankment, upper Kalee Nuddee crussing, at Rs. 14 per 100 | 17,540.88 |
| " | 705,516 | Ditto ditto 2nd Section. | 98,772.24 |
| " | 173,548,968 | Excavation canal channel, at Rs. $2 \cdot 5$ per 1,000 $\ldots$ | 4,33,872-42 |
| " | 25,881,232 | Escape into the Hindun Nuddee, at Rs. $2 \cdot 5$ per 1,000 | 64,703.08 |
| " | 1,305,000 | Widening at bridges, falls, \&c., at Rs. $2 \cdot 2$ per $1,000 \ldots$ $3 r d$ Section. | 2,871.00 |
| " | 151,288,356 | Excavation canal channel, at Rs. $2 \cdot 5$ per 1,000 ... | 378,220.89 |
| " | ],252,800 | Widening at bridges, falls, \&c., at Rs. 2.2 per 1,000 4th Section. | 2,756•16 |
| " | $84,886,493$ | Excavation canal channel, at Rs. $2 \cdot 5$ per 1,000 | 212,216.23 |
| " | 584,400 | Widening at bridges, falls, \&c., at Rs. $2 \cdot 2$ per $1,000 \ldots$ | 1,285•68 |
|  |  | Grand Total | 30,03,882.02 |

ABSTRACT OF. COST OF ESTIMATE No. 99.—REGULATOR AT ROORKEE.


ABSTRACT OF COST OF ESTIMATE No. 100.—JUNCTION REGULATOR.


ABSTRACT OF COST OF ESTIMATE No. 101.-FALLS 6', 11 BAYS OF 15', ROADWAY 18'.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | $100 \cdot 999$ | Brick-work in foundations, at Rs. 20 per 100 | $\begin{gathered} \text { Rs. } \\ 20,199 \cdot 80 \end{gathered}$ |
| ," | 50,193 | ," in superstructure, at Rs. 20 per 100 ... | 10,038.60 |
| , | 85,958 | ", $\quad$, at ks. 24 per 100 ... | 20,629.92 |
| " | 13,734 | Arch-work, heavy, at Rs. 32 per 100 ... ... | 4,394.88 |
| " | 645 | , light, at Rs. 28 per 100 ... ... ... | $180 \cdot 60$ |
| " | 11,946 | Inverts, at Rs. 26 per 100 ... ... ... | 3,105.96 |
| " | 14,632 | Brick-on-edge, at Rs. 25 per 100 ... ... ... | 3,658.00 |
| " | 799 | Stone capping, at Rs. 2 per foot ... ... ... | 1,598.00 |
| " | 39,900 | Concrete, at Rs. 14 per $100 \ldots$... ... ... | 5,586.00 |
| Lineal feet | 264 | Stone grooving, at Re. 1 per foot ... ... ... | $264 \cdot 00$ |
| , | 7,600 | Kunkur flooring (dry), at Rs. 8 per foot ... ... | 608.00 |
| " | 23,903 | Puddle, at Rs. 10 per $1,000 \ldots$ | $239 \cdot 03$ |
| " | 153,496 | Earth-work, at Rs. 3.25 per 1000 ... | $498 \cdot 86$ |
| " | 5,400 | Metalling, at Rs. 6 per $100 \ldots$.... ... ... | 324.00 |
| Square Feet | 992.16 |  | 6,449.04 |
| Number | 8 | Sluices, at Rs. 125 each ... ... ... \& | 1,000.00 |
| , | 8 | Pulleys, at Rs. 5 each ... ... ... | $40 \cdot 00$ |
|  | 8 | Crabs, at Rs. 135 each ... ... ... . | 1,080.00 |
| Lineal feet | 220 | Chain, at Re. l per foot ... ... ... ... | 220.00 |
| Number | 60 | Sleepers $15^{\prime} \times 5^{\prime} \times 5$, at Rs. 11.625 each ... ... | 697.50 |
|  | 2 | Hooks for lifting sleepers, at Rs. 7 each ...) | 14.00 |
| Square feet | 4,762.5 | Grating, at Re. $1 \cdot 15$ per foot $\quad . . \quad$... $\}$ 立 | 5,607.87 |
| Lineal feet | 561 | Girders, at Rs. $4 \cdot 25$ per foot ... ... ...\} 弫 | 2,384-25 |
| , | 165 | Wall-plates, at 6.25 per foot ... ... ...) | 1,031-25 |
|  |  | Grand Total | 89,649-56 |

( 97 )
ABSTRACT OF COST OF ESTIMATE No. 101.—GRATING OF FALLS ON
lst SEC'fION.

ABSTRACT OF COST OF ESTIMATE No. 102.-FALLS 6', 11 BAYS OF 15', ROADWAY $20^{\circ}$.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 101,407 | Brick-work in foundations, at Rs. 20 per 100 ... | 20,281 40 |
|  | 50,193 | " ", superstructure, at Rs. 20 per 100 ... | 10,038.60 |
| ", | 88.429 | \#, " at Rs. 24 per 100 ... | 21,222.96 |
| " | 15,042 | Arch-work, heavy, at Rs. 32 per 100 ... ... | 4,813•44 |
| " | 645 | " light, at Rs. 28 per 100 ... ... | $180 \cdot 60$ |
| " | 12,558 | Inverts, at Rs. 26 per $100 \ldots \ldots$ | 3,265.08 |
| " | 14,632 | Brick-on-edge, as Estimate No. 101, at Rs. 25 per 100 | 3,658.00 |
| " | 799 | Stone capping, as Estimate No. 101, at Rs. 2 per foot | 1,598.00 |
|  | 39,900 | Concrete, as Estimate No. 101, at Rs. 14 per 100 ... | 5,586.00 |
| Lineal feet Cubic feet | 264 | Stone grooving, as Estimate No. 101, at Re. 1 per foot | $264 \cdot 00$ |
|  | 7,600 | Kunkur flooring, dry, as Estimate No. 101, at Rs. 8 per 100 | 608.00 |
| " | 6,075 | Metalling, at $\begin{aligned} & \text { Rs } 6 \\ & 6 \text { per } \\ & 100 \\ & \end{aligned}$ | 364.50 |
| " | 23,903 | Puddle, as Estimate No. 101, at Rs. 10 per $\begin{aligned} & 1,000 \\ & \ldots\end{aligned}$ | 239.03 |
| " | $153 \cdot 496$ | Earth-work, as Estimate No. 101, at Rs. $3 \cdot 25$ per 1,000 | 498.86 |
|  |  | Lock apparatus, $\}$ as per Estimate No. 101 | $\{8,789 \cdot 04$ |
|  |  | gratings, \&c. ... $\}$ as per Estimate No. | \{ $9,534 \cdot 87$ |
|  |  | Grand Total | 90,942.38 |

ABSTRACT OF COST OF ESTIMATE No. 103.-FALL $6^{\prime}, 6$ BAYS OF 14' (ON JUNCTION).

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 86,380 | Kunkur masonry in foundations, at Rs. 17 per 100... | 14,684•60 |
| " | 6,555 | , \% superstructure, at Rs. 17 per 100... | 1,114.35 |
| ", | 19,617 | ", at Rs. 20 per 100 | 3,933.40 |
| " | 13,112 | Brick-work, at Rs. 20 per 100 | 2,622-40 |
| " | $39 \cdot 336$ | , at Rs. 24 per 100 .. | 9,440.64 |
| \% | 5,616 | Arch-work, heavy, at Rs. 32 per 100 | 1,797•12 |
| ", | -387 | " light, at Rs. 28 per 100 | $108 \cdot 36$ |
| " | 5,520 | , ${ }^{\text {inverts, at Rs. } 26 \text { per } 100}$ | 1,435.20 |
| " | 10,794 | Brick-on-edge, at Rs. 25 per 100 ... ... | 2,698.5 |
| " | 406 | Stone capping, at Rs. 2 per foot | $812 \cdot 00$ |
| " | 20,265 | Concrete, at Rs. 14 per 160 ... | 2,837•10 |
|  |  | Carried over ... | 41,483.67 |

ABSTRACT OF COST OF ESTIMATE No. 103,-continued.


ABSTRACT OF COST OF ESTIMATE No. 104.-FALL, ESCAPE, AND DAM BRIDGE AT THE 51st MILE.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| $\begin{gathered} \text { Cubic feet } \\ " \\ \# \end{gathered}$ | 172,480 | Brick-work in foundation, at Rs. 20 per 100 | 34,496.00 |
|  | 43,50] | " „, superstructure, at Rs. 20 per 100 | 8,700.20 |
|  | 117,511 | ", ", at Rs. 24 per 100 | 28,202.64 |
| ", |  | Arch-work, large, at Rs. 32 per $100 . .$. ... | 4,164.48 |
|  | 3,187 | " small, at Rs. 28 per 100... | 892.36 |
| " | 5,520 | ", inverts, at Rs. 26 per 100 | 1,435-20 |
| " | 32,059 | Brick-on-edge, at Rs. 25 per 100 ... | 8,014.75 |
| " | 799 | Stone capping, at Rs. 2 per foot $\ldots$ | ],598.00 |
|  | 39,900 | Concrete flooring, at Rs. 14 per $100 . .$. ... | 5,586.00 |
| Lineal feet | 1,272 | Stone grooves, at Re. 1 per foot ... ... | 1,272.00 |
| Cubic feet | 23,760 | Kunkur (dry), at Rs. 8 per 100 ... ... | 1,900•80 |
| ", | 7,800 | Metalling, at Rs. 6 per 100 ... ... ... ... | 468.00 |
| " | 25,463 | Puddle, at Rs. 10 per $1.000 \ldots$... ... | $254 \cdot 63$ |
| Lineal feet | 260,120 | Earth-work, at Rs. 3.25 per 1,000 ... | $845 \cdot 39$ |
|  | 94 | Iron grooves, at Rs. 5 per foot ... | 470.00 |
|  |  | Lock gear, as Estimate No. 101 ... | 8,789.04 |
| Square feet Number |  | Gratings, \&c., " ", ... | 9,534-87 |
|  | 2,658•6 | Regulating gates, wooden, at Res. 1.25 per foot | 3,323.20 |
|  | 42 | Axle blocks, small, at Rs. 5 each ... | 210.00 |
|  | 4 | . ${ }^{\text {c }}$ large, at Rs. 12 each ... | $48 \cdot 00$ |
| $\begin{aligned} & \text { Lineal feet } \\ & \text { Number } \end{aligned}$ | 840 | Lifting chains, small, at Rs. 0.9 per foot | 756.00 |
|  | 42 | Windlasses, small, at Rs. 22 each ... | 924.00 |
| Square feet Lineal feet | 2 | \%. large, at Rs. 66 each ... | 132.00 |
|  | $406 \cdot 6$ | Regulating gates, large, at Rs. 5 each | 2,033.00 |
|  | 132 | Lifting chains large, at Re. 1 per foot | $132 \cdot 00$ |
|  |  | Grand Total | 124,182.56 | KALLEE NUDDEE.


| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 70,000 | Excavation in foundations, at Rs. $3 \cdot 25$ per 1,000 ... | $\underset{2: 7 \cdot 50}{\mathrm{Ks}_{2}}$ |
|  | 90,159 | Brick-work in foundations, at ks. 20 per 100 ... | 18,031.80 |
| Lineal feet | 210 | Sinking large wells, at Rs. 7 per foot ... | 1,470.00 |
|  | 750 | ,", small ", at Rs 5 per foot | 3,750.00 |
| Cubic feet | 18,662 | Filling in wells, at Rs. 12 per $1,000 \ldots$... | 2,239•44 |
| , | 52,773 | Brick-work in superstructure, at Rs. 20 per 100 ... | 10,554.60 |
| " | 5,265 |  | 1,263.60 |
| , | 1,074 | Arch-work, light, at Rs. 28 per 100 ... ... | $300 \cdot 72$ |
| - " | 4,325 | Brick-on-edge, at Rs. 25 per 100 ... ... | 1,081.25 |
| Lineal feet | 132 | Stone grooving, at Re. 1 per foot ... ... | $132 \cdot 00$ |
| Cubic feet | 13,876 | Puddle, at Rs. 10 per 1,000 ... ... | 138.76 |
| Number | 14 | Curbs, large, at Rs. 90 each | 1,260.00 |
| , | 15 | ," small, at Rs. 20 each | 1,000.00 |
| " | 160 | Piles, at Rs. 3 each ... ... ... | 480.00 |
|  | 60 | Sleepers, at Rs. 11.625 each ... ... | 697.50 |
| Square feet | 2,565 | Grating, at Re. $1 \cdot 15$ per foot ... ... | 2,949.75 |
| Lineal feet | 306 | Girders, at Rs. $4: 25$ per foot ... ... | 1,300.50 |
|  | 90 | Wall-plates, at Rs. 6.25 per foot ... ... | $562 \cdot 50$ |
| Number | 2 | Hooks, at Rs. 7 each ... ... ... | 14.00 |
|  |  | Grand Total | 47,453-92 |

abStract of cost of estimate no. 105.-AQUEDUCT ON THE KALLEE NUDDEE, UPPER CROSSING.

| Quantity. |  | Drscription. | Cost. |
| :---: | :---: | :---: | :---: |
| Cabic feet | 86,675 | Brick-work in foundations, at Rs. 20 per 100 | $\underset{17,985 \cdot 00}{\mathrm{Rs}_{\mathrm{s}}}$ |
| " | 243,713 | ,, , superstructure, at Rs. 20 per 100 | 48,742.60 |
| " | 85,4.32 | ", ", at Rs. 24 per 100 | 20,503.68 |
| " | 4,031 | Arch-work, small, at Rs. 28 per 100 ... | 1,128•68 |
| " | 55,281 | ." large, at Rs. 32 per 100 ... | 17,689.92 |
| " | 12,060 | Brick-on-edge, at Rs. 25 per 100 ... | 3,015.00 |
| " | 64,160 | Kunkur flooring, dry, at Rs. 8 per 100... | 5,132.80 |
| " | 7,200 | Pitching slopes with boulders, at Rs. 10 per 100 | $720 \cdot 00$ |
|  | 102,300 | Puddle, at Rs. 10 per 1,000 ... | 1,023.00 |
| Lineal feet | 336 | Well-sinking, large, at Rs. 7 per foot ... | 2,352.00 |
|  | 900 | \% small, at Rs. 5 per foot | 4,500.00 |
| Cabic feet | 31,411 | Filling in wells, at Rs. 12 per 100 | 3,769•32 |
|  |  | Grand Total | 125,912.00 |

ABSTRACT OF COST OF ESTIMATE No. 106.-AQUEDUCT OVER KALLEE NUDDEE, LOWER CROSSING.

| Quantity. |  | Description. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cubic feet <br> 28 <br> 23 <br> 93 <br> 98 | $\begin{array}{r} 117,694 \\ 359,771 \\ 178,058 \\ 179,885 \\ 89,028 \end{array}$ | Brick-work in foundations, at Rs. 20 per 100 <br> " ,, superstructure, at Rs. 20 per 100 |  | $\ldots$ | $\begin{gathered} \text { Rs. } \\ 23,538 \cdot 80 \end{gathered}$ |
|  |  |  |  | $\ldots$ | 71,954.20 |
|  |  | " $\quad$ | , at Rs. 24 per 100 | ... | 42,733.92 |
|  |  | Kunkur masonry | \# at Rs. 17 per 100 | ... | 30,580-45 |
|  |  | " | " at Rs. 20 per 100 | $\cdot$ | 17,805•60 |
|  |  |  | Carried over | .. | 186,612.97 |

$$
\text { ( } 100 \text { ) }
$$

ABSTRACT OF COST OF ESTIMATE No. 106,-continued.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
|  |  | Brought forward ... | 186,612.97 |
| Cubic feet | 4,398 | Arch-work, light, at Rs. 28 per 100 ... ... | 1,231-44 |
| " | 181,472 | ." heavy, at Rs. 32 per 100 ... ... | 58,071.04 |
| $\stackrel{ }{*}$ | 25,725 | Brick-on-edge, at Rs. 25 per 100 ... ... | 6,431-25 |
| $\stackrel{ }{*}$ | 126,800 | Kunkur flooring, dry, at Rs. 8 per 100. . ... | 10,144.00 |
| $\stackrel{ }{*}$ | 7,200 | Pitching of slopes, at Rs. 10 per 100 ... ... | 720.00 |
| Linal ${ }^{\text {en }}$ | 148,575 | Puddle, at Rs. 10 per 1,000 ... ... | 1,485.75 |
| Lineal feet | - 360 | Sinking wells, large, at Rs. 7 per foot ... ... | 2,520.00 |
|  | 960 | , small, at Rs. 5 per foot ... ... | 4,800.00 |
| Cubic feet | 54,209 | Filling in wells, at Rs. 12 per 100 ... ... | 6,505•80 |
|  |  | .1 Grand Total | 278,521-58 |

ABSTRACT OF COST OF ESTIMATE NO. 107.-BRIDGE 4 ARCHES OF 51' SPAN, $18^{\prime}$ ROADWAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 16,160 | Brick-work in foundations, at Rs. 20 per 100 | 3,232.00 |
| \# | 32,260 | " in superstructure, at Rs. 20 per 100 | 6,452.00 |
| " | 26,333 | " ditto, at Rs. 24 per 100 | 6,319.92 |
| " | 13,734 | Arch-work, large, at Rs. 32 per 100 | 4,394:88 |
| " | 6,426 | ditto inverts, at Rs. 26 per 100 ... ... ... | 1,670.76 |
| " | 5,400 | Metalling, at Rs. 6 per 100... .. ... ... | 324.00 |
| " | 16,160 | Earth-work, at Rs. $3 \cdot 25$ 1,000 ... ... ... | 52.52 |
|  |  | Grand Total | 22,446.08 |

ABSTRACT OF COST OF ESTIMATE No. 108.__BRIDGE, 4 ARCHES OF $51^{\prime}$ SPAN, $20^{\prime}$ ROADWAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Re. |
| Cubic feet | 16,568 | Brick-work in foundations, at Rs. 20 per 100 | 3,313.60 |
| " | 32,235 | , superstructure, at Rs. 20 per 100 | 6,447.00 |
| " | 28,830 | ", at Rs. 24 per 100 | 6,919.20 |
| " | 15,042 | Arch-work, large, at Rs. 32 per 100 ... | 4,813-44 |
| " | 7,038 | , inverts, at Rs. 26 per 100 ... | 1,829.88 |
| " | 6,075 | Metalling, at Rs. 6 per 100 ... | 364.50 |
| " | 16,563 | Earth-work, at Rs. $3 \cdot 25$ per 100 ... . ... | 53.84 |
|  |  | Grand' Total | 23,741-46 |

$$
\text { ( } 101 \text { ) }
$$

## AbStract of Cost of estimate No. 109.-BRIDGE, 4 ARCHES OF 5$]^{\prime}$ SPAN, BOADWAY $25^{\prime}$.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 17,588 | Brick-work in foundations, at Rs. 20 per 100 ... | 3,517.60 |
| , | 34,652 | ", superstructure, at Rs. 20 per 100 ... | 6,930-40 |
| " | 32,591 | ", ", at Rs. 24 per 100 ... | 7,821-84 |
| " | 18,312 | Arch-work, large, at Rs. 32 per $100 \ldots$.... | 5,859.84 |
| " | 8.568 | inverts, at Rs. 26 per 100 ... | 2,227.68 |
| " | 7,762 | Metalling, at Rs. 6 per 100 . . ... | 465.72 |
| " | 17,588 | Earth-work, at Re. $3 \cdot 25$ per 1,000 ... | 57-16 |
|  |  | Grand Total | 26,880-24 |

ABSTRACT OF COST OF ESTIMATE No. 110.-BRIDGE ON JUNCTION, $3^{\prime}$ ARCHES OF $40^{\circ}$ SPAN, ROADWAY $18^{\circ}$.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 14,394 | Kunkur masonry in foundations, at Rs. 17 per 100 .. | 2,4.46.98 |
| „ | 5,224 | " in superstructure, at Rs. 17 per 100 .. | 888.08 |
| " | 3,977 | B." $\quad$ " at Rs. 20 per $100 \ldots$ | 795.40 |
| " | 10,450 | Brick-work in superstructure, at Rs. 20 per 100 ... | 2,090.00 |
| " | 7,955 | " ", at Rs. 24 per 100 ... | 1,909•20 |
| " | 9,072 | Arch-work, heavy, at Rs. 32 per 100 ... ... | 2,903.04 |
| " | 2,520 | Brick-on-edge, at Rs. 25 per 100 ... ... | 630.00 |
| " | 3,480 | Metalling, at Rs. 6 per 100 ... ... | 208:30 |
| " | 14,394 | Earth-work, at Rs. 3.25 per 1,000 ... ... | 46.78 |
|  |  | Grand Total | 11,918.28 |

abstract of cost of estimate no. 1ll-CULVERT under channel; 2 ARCHES $20^{\circ}$.

| Quantity. |  | Drscription. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet. | 39,504 | Masonry in foundations, at Rs. 17 per 100 ... | 6,715.68 |
| " | 86,106 | Superstructure, at Rs. 17 per 100 | 14,638.02 |
| " | 24,048 | Arch-work, light, at Rs. 28 per 100 ... | 6,733.44 |
| " | 21,042 | " inverts, at Rs. 26 per $100 \ldots$ | 5,470.92 |
| " | 2,880 | Kanker flooring, dry, at Rs. 8 per $100 .$. | $230 \cdot 40$ |
| " | 5,460 | Puddle, at Rs. 10 per 1,000 ... ... | $54 \cdot 60$ |
| " | 61,104 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... ... | $198 \cdot 58$ |
|  |  | Grand Total | 34,041 $\cdot 64$ |

ABSTRACT OF COST OF ESTIMATE No. 112.-CULVERT UNDER CHANNEL, 1 ARCH OF 18 .

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet. |  | Masonry in foundations, at Rs. 17 per 100 | $\begin{gathered} \text { Rs. } \\ 1,852 \cdot 32 \end{gathered}$ |
| , | 43,455 | " in superstructure, at Rs. 17 per 100 | 7,387.35 |
| " | 4,128 | Arch-work, at Rs. 28 per 100 ... | 1,155.84 |
| " | 3,612 | Invert, at Rs. 26 per 100 ... ... | $939 \cdot 12$ |
| " | 960 | Kunkur flooring, dry, at Rs. 8 per 100 | 76-80 |
| " | 1,470 | Puddle, at Rs. 10 per $1,000 \ldots$. | 14.70 |
| " | 14,336 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... | 46.59 |
|  |  | Grand Total | 11,472.72 |

ABSTRACT OF COST OF ESTIMATE No. 113.-INLET FOR DRAINAGES, SECTION No. 1.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet <br> 38 <br> 98 98 | 3,822 6,390 253 220 | Masonry in foundations, at Rs. 17 per 100 ,, in superstructure, at Rs. 17 per 100 Arch-work, light, at Rs. 28 per 100 Kunkur flooring, dry, at Rs. 8 per 100... | $\begin{array}{r} \text { Rs. } \\ 649 \cdot 74 \\ 1,086 \cdot 30 \\ 70 \cdot 84 \\ 17 \cdot 60 . \end{array}$ |
|  |  | Grand Total | 1,828.48 |

ABSTRACT OF COST OF ESTIMATE No. 114.-RAJBUHA OUTLET $6^{\prime}$ WIDE, SECTION No. 1.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet <br> 28 38 | 2,816 |  | Rs. $478 \cdot 72$ |
|  | 4,325 | Masonry in foundations, at Rs. 17 per 100 <br> " in superstructure, at Rs. 17 per $100 \quad \ldots$ | $735 \cdot 25$ |
|  | 196 | Arch-work, at Rs. 28 per 1,000 ... ... | 54.88 |
|  | 2,658 | Puddle, at Rs. 10 per $100 . . . \quad$... ... | 26.58 |
| Number | 1 | Windlasses, at Rs. 20 each .... ... | 20.00 |
| Lineal feet | 1 | Sluice gate $6.5 \times 8$, at Rs. 50 per 100 ... ... | 50.00 |
|  | 18 | Chain, at Rs. 9 per foot ... ... ... | 16.20 |
| Cubic ${ }^{\text {feet }}$ | 20 | Stone grooves, at Re. 1 per foot | 20.00 |
|  | 2,816 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... | $9 \cdot 15$ |
|  |  | Grand Total | 1,410.78 |

ABSTRACT OF COST OF ESTIMATE No. 115.-3nd CLASS CHOWKIES.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 186 | Masonry in fo undations, at Rs. 17 per 100 ... | Rs. 31.62 |
| \# | 1,276 | " superstructure, at Rs. 18 per 100 ... | 229.68 |
| " | 188 | Arch-work, light, at Rs. 28 per 100 ... ... | 52.64 |
| " | 96 | Brick-on-edge, at Rs. 25 per 100 ... ... | 24.00 |
| " | 186 | Excavation, at Rs. 2 per 1,000 ... ... | 0.37 |
| Number | 1 | Door, at Rs. 25 ... | $25 \cdot 00$ |
|  |  | Grand Total | 363.31 |

ABSTRACT OF COST OF ESTIMATE No. 116.-6' FALLS, 7 BAYS OF 15', ROADWAY 18'.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 11,482 | Kunkur masonry in foundations, at Rs. 17 per 100 | 1,951.94 |
| " | 22,964 | Brick-work , at Rs. 20 per 100... | 4,582.80 |
| " | 8,000 | Kunkur masonry in superstructure, at Rs. 17 per 100 | 1,360.00 |
| " | 16,000 | Brick-work , at Rs. 20 per 100 ... | 3,200.00 |
| " | 9,762 | Kunkur masonry \#, at Rs. 20 per 100 ... | 1,952-40 |
| " | 19,525 | Brick-work \#, at Rs. 24 per 100 ... | 4,686.00 |
| " | 336 | Arch-work, light, at Rs. 28 per 100 ... | 94.08 |
| " | 8,190 | ;, heavy, at Rs. 32 per 100 ... | 2,620.80 |
| " | 3,780 | , inverts, at Rs. 26 per 100 ... ... | $982 \cdot 80$ |
| " | 7,789 | Brick-on-edge, at Rs. 25 per 100 ... | 1,947.25 |
| " | 551 | Stone capping, at Rs. 2 per foot ... | 1,102.00 |
| I. | 25,200 | Concrete, at Rs. 14 per 100 ... | 3,528.00 |
| Lineal feet | 140 | Stone grooves, at Rs. 1 per foot ... | $140 \cdot 00$ |
| Cubic feet | 5,920 | Kunkur flooring, dry, at Rs. 8 per 100... ... | $473 \cdot 60$ |
| " | 3,480 | Metalling, at Rs. 6 per 100 ... ... | 208.80 |
| ," | 9,687 | Puddle, at Rs. 10 per 1,000 ... | 96.87 |
|  | 80,212 | Excavation, at Rs. 3.25 per 100 | $260 \cdot 69$ |
| Number | -60 | Sleepers, at Rs. 11.625 each | $697 \cdot 50$ |
| Square feet | 2,362.5 | Grating, at Rs. $1 \cdot 15$ per foot | 2,716.87 |
| Lineal feet | 238 | Girders, at Rs. $4 \cdot 25$ per foot | 1,011.50 |
|  | 105 | Wall-plates, at Rs. 6.25 per foot | 655.75 |
| Number |  | Hooks, at Rs. 7 each ... | 14.00 |
|  |  | Grand Total | 34,293.65 |

## ÁBSTRACT OF COST OF ESTIMATE No. 117.-FALL REGULATOR AND ESCAPE AT BOOLUNDSHUHUR BRANCH HEAD.

| Quantity. |  | Description. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Rs. |
| Cabic feet | 62,949 | Masonry in foundation, at Rs. 17 per 100 |  |  | 10,700.65 |
|  | 51,612 | ," superstructure, at Rs. 17 per |  | ... | 8,774.04 |
| " | 41,717 | Brick-work, at"Rs. 24 per Rs. 100 |  | ... | 8,343.40 |
| " | 4,529 |  |  | ... | 1,086.96 |
| " | 1,236 | Arch work, light, at Rs. 28 per 100 |  | ... | 346.08 |
| " | 7,875 | , heavy, at Rs. 32 per 100 |  | ... | 2,520.00 |
| " | 17,246 | Brick-on-edge, at Rs. 25 per 100 |  | $\ldots$ | 4,311-50 |
| " | 5006 | Stone capping, at Rs. 2 per foot |  | ... | 1,012.00 |
|  | 25.200 | Concrete, at Rs. 14 per 100 |  | . | 3,528.00 |
| Lineal feet | 740 | Stone grooves, at Re. 1 per foot |  | ... | 740.00 |
| Cubic feet | 14,800 | Kunkur flooring, dry, at Rs. 8 per 100 |  | $\ldots$ | 1,184.00 |
| \% | 7,313 | Metalling, at Rs. 6 per 100 ... |  | ... | 438.78 |
| " | 3,780 | Pitching, at Rs. 10 per $100 \ldots$ |  | $\ldots$ | 378.00 |
| " | 9,687 87 | Puddle, at Rs. 10 per 1,000 ... |  | . | 96.87 |
| " | 87,413 | Excavation, at Rs. $3 \cdot 25$ per 100 |  | . | 284.09 |
| Square feet |  | Grating, as per Estimate No. 116Gates, at Rs. $1 \cdot 25$ per foot ... | ... | . | 5,095.62 |
|  | 1,519 |  | ... |  | 1,898.75 |
| " | 33 | Gates, at Rs. $1 \cdot 25$ per foot $\ldots$. Axle blocks, small, at Rs. 5 each |  | ... | $165 \cdot 00$ |
|  | 30 | Windlasses, at Rs. 22 each .. |  |  | $660 \cdot 00$ |
| Lineal feet | 510 | Feet of chain, at Rs. 0.9 per foot | ... |  | 459.00 |
|  |  |  | Grand Total |  | 52,022.74 |

## ( 104 )

ABSTRACT OF COST OF ESTIMATE No. 118.-BRIDGE, 3 ARCHES OF 40', ROADWAY 25'.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 13,288 | Kunkur masonry in foundation, at Rs. 17 per $100 \ldots$ | 2,258.96 |
| " | 4,831 | " $\quad$ superstructure, at Rs. 17 per 100 | $821 \cdot 27$ |
| " | 5,321 | \# " $\quad \prime \prime \quad$ at Rs. 20 per 100 | 1,064.20 |
| " | 9,661 | Brick-work in superstructure, at Rs. 20 per 100 ... | 1,932-20 |
| " | 10,641 | , " ", at Rs. 24 per 100 | 2,553.84 |
| " | -10,920 | Arch-work, heavy, at Rs. 32 per 100 ... | 3,494.40 |
| " | 5,040 | , inverts, at Rs. 26 per $100 \ldots$... | 1,310.40 |
| " | 5,003 | Metalling, at Rs. 6 per 100 ... ... | 300.18 |
| " | 13,288 | Earth-work, at Rs. 3.25 per 1,000 ... | $43 \cdot 18$ |
|  |  | Grand Total | 13,778.63 |

abstract of cost of estimate No. 119.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | , 11,998 | Kunkur masonry in foundations, at Rs. 17 per $100 \ldots$ | 2,039.66 |
| " | 4,331 | " $\quad$, in superstructure, at Rs. 17 per 100 | $736 \cdot 27$ |
| , | 4,697 | , " $\quad$, at Rs. 20 per 100 | $939 \cdot 40$ |
| " | 8,661 | Brick-work „ $\quad$, at Rs. 20 per 100 | 1,732.20 |
| " | 9,396 | ", " ${ }^{\prime \prime}$ at Rs. 24 per 100 | 2,255.04 |
| " | 8,970 | Arch-work, heavy, at Rs. 32 per 100 | 2,870.40 |
| " | 4,140 | , inverts, at Rs. 26 per $100 \ldots$ | 4,076.40 |
| " | 3,915 | Metalling, at Rs. 6 per 100 ... | 2,34:90 |
| " | 11,998 | Earth-work, at Rs. $3 \cdot 25$ per 1,000 ... | 38.99 |
|  |  | Grand Total | 11,923.26 |

ABSTRACT OF COST OF ESTIMATE NO. 120.-BRIDGES, 3 ARCHES OF 40', ROADWAY 18.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 11,482 | Kunkur masonry in foundations, at Rs. 17 per $100 \ldots$ | 1,951.94 |
| " | 4,131 | , ", in superstructure, at Rs. 17 per $100 \ldots$ | $702 \cdot 27$ |
| , | 4,448 | ." ${ }^{\prime}$ ditto, at Rs. 20 per 100 ... | 889.60 |
| " | 8,261 | Brick-work \#, in ditto, at Rs. 20 per 100 ... | 1,652:20 |
| ", | 8,898 | , ' ${ }^{\prime}$ ditto, at Rs. 24 per 100 | 2,135.52 |
| " | 8,190 | Arch-work, he avy, at Rs. 32 per 100 ... | 2,620.80 |
| " | 3,780 | ". inverts, at Rs. 26 per 100 ... ... | $982 \cdot 80$ |
| " | 3,480 | Metalling, at Rs. 6 per $100 \ldots$ | 208.80 |
| " | 11,482 | Earth-work, at Rs. $3 \cdot 25$ per 1,000 ... | $37 \cdot 31$ |
|  |  | Grand Total | 11,181-24 |

ABSTRACT OF COST OF ESTIMATE NO. 121.—SYPHON FOR DRAINAGE, 2 ARCHES OF $10^{\prime}$.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 13,035 | Masonry in foundations, at Rs. 17 per 100 | 2,215.95 |
| " | 12,916 | , ${ }^{\text {a }}$ superstructure, at Rs. 17 per 100 | 2,195.72 |
| " | 5,856 | Arch-work, light, at Rs. 28 per 100 | 1,639.68 |
| " | 5,124 | ", inverts, at Rs. 26 per 100 | 1,332.24 |
| " | 1,040 | Kunkur flooring, dry, at Rs. 8 per 100 | 83.20 |
| " | 1,872 | Puddle, at Rs. 10 per 1,000 $\ldots$ | 18.72 |
| " | 58,176 | Excavation, at Rs. 3.25 per 1,000 | $189 \cdot 07$ |
|  |  | Grand Total | 7,674-58 |

abstract of cost of estimate No. 122.-CULVERT, l arch of $15^{\prime}$.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cabic feet. | 17,971 | Masonry in foundations, at Rs. 17 per 100 ... | 3,055.07 |
| " | 35,266 | , ", superstructure, at Rs. 17 per 100 | 5,995•22 |
| " | 3,760 | Arch-work, light, at Rs. 28 per 100 ... | 1,052:80 |
| " | 3,642 | " inverts, at Rs. 26 per 100 | $946 \cdot 92$ |
| " | 900 | Kunkur flooring, dry, at Rs. 8 per 100 | $72 \cdot 00$ |
| " | 1,656 | Puddle, at Rs. 10 per $1,000 \ldots$ | 16.56 |
| " | 23,259 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... ... | $75 \cdot 59$ |
|  |  | Grand Total | 11,214•16 |

abstract of Cost of estimate no. 123.-bridge on escape, 2 arches OF $30^{\prime}$, ROADWAY $25^{\prime}$. SPAN.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet. | 6,952 | Kunkur masonry in foundations, at Rs. 17 per 100 ... | 1,181•84 |
| " | 5,100 | " $\quad$, in superstructure, at Rs. 17 per 100... | $867 \cdot 00$ |
| \% | 10,208 | ", ". at Rs. 20 per 100... | 2,041 $\cdot 60$ |
| " | 4,158 | Arch-work, heary, at Rs. 32 per 100 ... | 1,330.56 |
| " | 2,520 | Travests, at Res. 26 per 100 ... ... | $655 \cdot 20$ |
| ", | 2,933 | Metalling, at Rs. 6 per $100 \ldots$... ... | $175 \cdot 98$ |
| " | 7,000 | Earth-work, at Rs. 3.25 per 1,000 ... ... | 22.75 |
|  |  | Grand Total | 6,274•93 |

ABSTRACT OF COST OF ESTIMATE No. 124.-BRIDGE ON ESCAPE, 2 ARCHES OF $30^{\circ}$, ROADWAY $30^{\prime}$.

| Quantity. |  | Descripion. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet.. | 7,602 | Kunkar masonry in foundation, at Rs. 17 per $100 \ldots \ldots$ | 1,292.34 |
|  | 5,500 | ", superstructure, at ks. 17 per 100.. | $935 \cdot 00$ |
| \% | 11,148 | " " ${ }^{\prime \prime}$ at Rs. 20 per 100... | 2,229.60 |
| " | 4,960 | Arch-work, heavy, at Rs. 32 per 100 ... ... | 1,568.00 |
| , | 2,970 | Inverts, at Rs. 26 per 100 ... | $772 \cdot 20$ |
| " | 2,250 | Metalling, at Rs. 6 per $100 \ldots$ | 135.00 |
| " | 8,000 | Earth-work, at Rs. 3.25 per 1,000 | 26.00 |
|  |  | Grand Total | 6,958•14 |

ABSTRACT OF COST OF ESTIMATE No. 125.-FALL ON ESCAPE INTO HINDUN FROM 100th MILE.


ABSTRACT OF COST OF ESTIMATE No. 126.-RAJBUHA OUTLET, SECTION No. 2.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  | - | Rs. |
| Cubic feet | 2,798 | Masonry in foundations, at Rs. 17 per 100 | 475.66 |
| " | 3,942 | Ditto in superstructure, at Rs. 17 per 100 | 670.14 |
| " | 196 | Arch-work, light, at Rs. 28 per 100 ... ... | 54.88 |
| \% | 2,403 | Puddle, at Rs. 10 per 1,000 ... | 24.03 |
|  | 2,798 | Excavation, at Rs. 3.25 per 1,000 ... | 9.09 |
| Number | 1 | Gate, $6.5 \times 6$, at Rs. 40 each -.. | $40 \cdot 00$ |
|  | 1 | Windlass, at Rs. 20 each ... | 20.00 |
| Lineal feet | 15 | Chain, small, at Rs. 0.9 per foot | 13.50 |
| " | 16 | Stone grooves, at Re. 1 per foot | 16.00 |
|  |  | Grand Total | 1,323•30 |

## ABSTRACT OF COST OF ESTIMATE No. 127.-6' FALLS, 6 BAYS OF 15', ROADWAY 18'.

| Quantity. |  | Description. | Cosr. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 31,498 | Kunkur masonry in foundation, at Rs. 17 per 100 | 5,354.66 |
| " | 22,809 | " , " superstructure, at Rs. 17 per 100 | 3,877.53 |
| " | 25,954 | " ", " 20 per 100 | 5,190.80 |
| " | 7,245 | Arch-work, heavy, at Rs. 32 per 100 ... | 2,318.40 |
| " | 288 | (" light, at Rs. 28 per 100 ... | $80 \cdot 64$ |
| " | 6,646 | Brick-on-edge, at Rs. 25 per 100 ... ... | 1,661.50 |
| " | 3,307 | Arch-work, inverts, at Rs. 26 per 100 ... | 859.82 |
| " | 4788 | Stone capping, at Rs. 2 per foot ... | 956.00 |
|  | 21,525 | Concrete, at Rs. 14 per 100 ... | 3,013.50 |
| Lineal feet | 108 | Stone grooves, at Re. 1 per foot $\ldots$ | 108.00 |
| " | 5,200 | Kunkur flooring, dry, at Rs. 8 per 100... ... | 416.00 |
| " | 8,300 | Metalling, at Rs. 6 per 100 ... | 198.00 |
| " | 8,515 | Puddle, at Rs. 10 per 1,000 ... ... | $85 \cdot 15$ |
| " | 71,314 | Excavation, at Rs. 3.25 per 1,000 $\ldots$ | 231.77 |
| Square feet | 600 | Sleepers, at ks. 11.625 each $\quad . .7$ | 697.50 |
| Square feet | 1,755 | Grating, at Rs. $1 \cdot 15$ per foot ... | 2,018.25 |
| Lineal feet | 204 | Girders, at Rs. 4.25 per foot ... $\}$ grating, \&c. | 867.00 |
|  |  |  | $562 \cdot 50$ |
| Number | 2 | Hooks, at Rs. 7 per each | ( 14.00 |
|  |  | Grand Total | 28,511•02 |

## ABSTRACT OF COST OF ESTIMATE No. 128.-5' FALLS, 6 BAYS OF 15',

 ROADWAY $20^{\prime}$.| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 23,510 | Kunkur masonry in foundation, at Rs. 17 per 100 | 3,996.70 |
| , | 22,929 | " $\quad$, superstructure, at Rs. 17 per 100 | 3,897-93 |
| , | 27,107 | " ", " at Rs. 20 per 100 | 5,421•40 |
| " | 7,935 | Arching, heavy, at Rs. 32 per 1!:0 ... | 2,539-20 |
| \% | 289 | ." light, at Rs. 28 per 100 ... ... | $80 \cdot 64$ |
| , | 6,646 | Brick-on-edge, at Rs. 25 per 100 ... . ... | 1,661-50 |
| " | 4788 | Stone capping, at Rs. 2 per foot ... ... | 956.00 |
|  | 21,525 | Concrete, at Re. 14 per foot ${ }^{\text {a }}$-.. | 3,013.50 |
| Lineal feet | 108 | Stone grooves, as estimate No. 127, at Re. 1 per foot ... | 108.00 |
| Cubic feet | 3,712 | Metalling roadwas, at Rs. 6 per 100 ... ... | 222.72 |
| " | 8,515 | Puddle, at Rs. 10 per 1,000 $\quad$.. | $85 \cdot 15$ |
| , | 70,880 | Excavation, at Rs. 3.25 per 1,000 ... | $230 \cdot 36$ |
|  | 12,340 | Kunkur flooring, at Rs. 8 per 100 ... | 987.20 |
| Square feet | 2,616 | Well foundations, at Rs. 301 per 100 ... ... | 7,874•16 |
| Number | 72 | Piles, at Rs. 3 each ... ... ... | 216.00 |
|  |  | Gratings, \&c., as estimate $\begin{aligned} & \text { No. } 327 \\ & \text {... }\end{aligned}$ | 4,159•25 |
|  |  | Grand Total | 35,449•71 |

ABSTRACT OF COST OF ESTIMATE No. 129.—COEL BRANCH HEAD.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 20,166 | Kunker masonry in foundations, at Rs. 17 per $100 . .$. | $\begin{gathered} \text { Rs. } \\ 3,428 \cdot 22 \end{gathered}$ |
|  | 20,333 | Brick work, ", at Rs. 20 per 100... | 4,066.60 |
| ", | 23,314 | Kunker masonry in superstructure, at Rs. 17 per 100 | 5,663.38 |
| " | 34,020 | $" \# \#{ }^{\prime \prime}{ }^{\prime}$ at Rs. 20 per 100 | 6,804.00 |
| " | 4,878 | Arch-work, light, at Rs. 29 per 100 ... ... | 1,365-84 |
| " | 12,271 | Brick-on-edge, at Rs. 25 per 100 ... | 3,067.75 |
|  |  | Stone capping, as estimate No. 127 ... | 956.00 |
|  | 21,525 | Concrete, at Rs. 14 per 100 ... | 3,013.50 |
| Lineal feet | ${ }^{396}$ | Stoue grooves, Re. 1 per foot $\ldots$... | 396.00 |
| Cubic feet | 10,830 | Kunkur flooring, dry, at Rs. 8 per 100 ... | 866.40 |
| " | 5,265 | Road metalling, at Rs. 6 per 100 ... ... | 315.90 |
|  | 2,400 | Pitching with boulders, at Rs. 10 per 100 ... | $240 \cdot 00$ |
| " | 10,161 | Puddle, at Rs. 10 per 1,000 ... ... | $101 \cdot 61$ |
|  | 6,778 | Filling in wells, at Rs. 12 per 100 ... ... | 813.36 |
| " | 240 | Piles, at Rs. 3 each ... ... | 720.00 |
|  | 120 | Curbs, small, at Rs. 20 each ... ... | 2,400.00 |
|  | 960 | Undersinking wells, $120 \times 8$, at Rs. 5 per foot | 4,800.00 |
|  |  | Gratings, as estimate No. 127 ... | 4,159.25 |
| Square feet Number | 798 | Regulating gates, at Rs. 1.25 per foot | 997.50 |
|  | 20 | Axle blocks, at Rs. 5 each | 100.00 |
|  | 18 | Windlasses, at Rs. 22 each | 396.00 |
| Lineal feet | 288 | Chain, at Re. 0.9 per foot ... | $259 \cdot 20$ |
|  |  | $G_{\text {rand }}$ Total | 44,930.51 |

## ABSTRACT OF COST OF ESTIMATE No. 130.-BRIDGES, 3 ARCHES OF 35' SPAN,

 18' ROADWAY.| Quantity. |  | Description | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 10,848 | Kunkur masonry in foundations, at Rs. 17 per $100 . .$. | $\begin{gathered} \text { Rs. } \\ 1,814 \cdot 16 \end{gathered}$ |
| \# | 11,604 | \% $\quad$, in superstructure, at Rs. 17 per 100 | 1,972.68 |
| " | 13,076 | ", " at Rs. 20 per 100 | 2,615-20 |
| " | 7,245 | Arch-work, heavy, at Rs. 32 per $100 . .$. | 2,318.40 |
| " | 3,307 | ", inverts, at ks. 26 per 100 | 859.82 |
| " | 3,300 | Metalling, at Rs. 6 per 100 | 198.00 |
| " | 10,848 | Earth-work, at Rs. $3 \cdot 25$ per 1,000 | 35.25 |
|  |  | Grand Total | 9,843.51 |

ABSTRACT OF COST OF ESTIMATE No. 130A.-BRIDGE, AS ESTIMATE No. 130A, BUT WITH WELL FOUNDATIONS.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | $\begin{gathered} 2,346 \\ \text { as Estimate } \\ \text { No. } 130 \end{gathered}$ | Kunkur masonry in foundation, at Rs. 17 per $100 . .$. „ $\quad$, in superstructure at ... |  |
| " | ditto | Arch-work, heavy ... ... | 2,318•40 |
| " | ditto | Metalling ... ... ... ... | 198.00 |
| " | ditto | Earth-work ... ... | $35 \cdot 25$ |
| - | 6,720 | Kunkur flooring, at Rs. 8 per 100 ... ... | 537.60 |
| Square feet | 1,173 | Well foundations, at Rs. 301 per 100 ... ... | 3,530•73 |
|  |  | Grand Total | 11,606.68 |

ABSTRACT OF COST OF ESTIMATE No. 130A.-WELL FOUNDATIONS FOR ESTIMATE No. 130 A, WELL SUNK $8^{\prime}$.

| Quantity. |  | Description. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |

Area of pier foundations, $2 \times 176=352$
" abutments " $2 \times 168=336$
$1,980 \cdot 52 \div 688=$ Rs. 301 per 100 superficial foot.

ABSTRACT OF COST OF ESTIMATE No. 131.-BRIDGES, 3 ARCHES 35' SPAN, $20^{\prime}$ ROADWAY.

| Quantity. |  | Drscription. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 11,326 | Kunkur masonry in foundations, at Rs. 17 per 100 | 1,925-42 |
| " | 12,129 | " \# superstructure, at Rs. 17 per 100 | 2,061.93 |
| \% | 13,823 | " $\quad \prime \quad \prime \quad$ at Rs. 20 per 100 | 2,764.60 |
| " | 7,935 | Arch-work, heavy, at Rs. 32 per 100 ... ... | 2,539•20 |
| " | 8,622 | , inverts, at Re. 26 per 100 ... | 941.72 |
| " | 3,712 | Metalling, at Rs. 6 per 100 ... | 222.72 |
| " | 11,320 | Earth-work, at Rs. $3 \cdot 25$ per 1,000 ... ... | 36.80 |
|  |  | Grand Total | 10,492.39 |

abSTRACT OF COST of estimate No. 131 A-bridge as before, but WITH WELL FOUNDATIONS.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet$"$$"$$"$Square feet | $\begin{gathered} 2,454 \cdot 00 \\ \text { as before } \\ " \\ " \\ 7,140 \\ 1,227 \end{gathered}$ | Kunkur masonry in foundations, at Rs. 17 per 100... " $\quad$, in superstructure ... ... | $\begin{array}{r} 417 \cdot 18 \\ 4,826 \cdot 53 \end{array}$ |
|  |  | Arch-work, heavy | 2,539.20 |
|  |  | Metalling ... ... ... | 222.72 |
|  |  | Earth-work ... $\ldots$.. $\ldots$ | 36.80 |
|  |  | Kunkur flooring, dry, at Rs. 8 per 100... ... | 571-20 |
|  |  | Well foundations, at Rs. 301 per 100 ... ... | 3,693-27 |
| Square feet |  | $\mathrm{Gr}_{\text {rand }}$ Total | 12,306.90 |

## ABSTRACT OF COST OF ESTIMATE No. 132.-BRIDGES, 3 ARCHES OF

 $35^{\prime}$ SPAN, 20' ROADWAY.| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 12,523 | Kunkur masonry in foundations, at Rs. 17 per $100 .$. |  |
| " | 13,442 | " , superstructure, at Rs. 17 per $100 \ldots$ | $2,285 \cdot 14$ |
| " | 15,692 | "" " at Rs. 20 per 100... | 3,138-40 |
| " | 9,660 | Arch-work, heavy, at Rs. 32 per 100 ... ... | 3,091-20 |
| " | 4,410 | \#, inverts, at Rs. 26 per 100 ... ... | 1,146.60 |
| " | 4,744 | Metalling, at Rs. 6 per 100.. ... | 284.64 |
| " | 12,523 | Earth-work, at Rs. $3 \cdot 25$ per 1000 ... ${ }^{\text {a }}$.. | $40 \cdot 69$ |
|  |  | Grand Total | 12,115.58 |

ABSTRACT OF COST OF ESTIMATE No. 132 A.-BRIDGE AS IN ESTIMATE No. 132, BUT WITH WELL FOUNDATIONS.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | $\begin{gathered} 2,724 \\ \text { as Estimate } \\ \text { No. } 132 \end{gathered}$ | Kunkur masonry in foundations, at Rs. 17 per 100... <br> in Superstructure <br> Arching, heavy | $\begin{array}{r} 463 \cdot 08 \\ 5,423 \cdot 54 \\ 3,091 \cdot 20 \end{array}$ |
|  | " | Metalling ... ... ... | 284.64 |
|  | ग | Earth-work ... ... ... | $40 \cdot 69$ |
| cubic feet | 8,190 | Kunkur flooring under arches, at Rs. 8 per 100 ... | 655.20 |
| Square feet | 1,362 | Well foundations, at Rs. 301 per 100 ... ... | 4,099.62 |
|  |  | Graind Total | 14,057.97 |

ABSTRACT OF COST OF ESTIMATE No. 133.-SYPHON FOR DRAINAGE, 2 ARCHES OF $10^{\prime}$ ON 3Rd SECTION.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 11,910 | Masonry in foundations, at Rs. 17 per 100 ... | 2,024.70 |
| " | 11,866 | , in superstructure, at Rs. 17 per 100 ... | 2,017•22 |
| " | 5,256 | Arch-work, light, at Rs. 28 per 100 ... | 1,471.68 |
| " | 4,599 | \# inverts, at Rs. 26 per 100 | 1,195.74 |
| " | 1,040 | Kunkur flooring, dry, at Rs. 8 per 100 ... | 83.20 |
| " | 1,547 | Puddle, at Rs. 10 per 1,000 | 15.47 |
| " | 52,976 | Excavation, at Rs. 3:25 per 1,000 ... ... |  |
|  |  | Grand Total | 6,980 18 |

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## ABSTRACT OF COST OF ESTIMATE No. 134.-SYPHON DRAINAGE, 1 ARCH 10 (ON 3Rd SECTION.)

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cabic feet | 6,833 | Masonry in foundations, at Rs. 17 per 100 | 1,161.61 |
|  | 7,696 | \#r in superstructure, at Rs. 17 per 100 ... | 1,308.32 |
| " | 2,628 |  | 735.84 |
| " | 2,299 | " inverts, at Rs. 26 per $100 \ldots$ | $597 \cdot 74$ |
| " | 520 | Kunkur flooring, dry, at Rs. 8 per 100... | 41.60 |
| " | 833 | Puddle, at Rs. 10 per 1,000 ... | $8 \cdot 33$ |
|  | 28,000 | Excavation, at Rs. 3.25 per 1,000 | 91.00 |
|  |  |  | 3,944.44 |

ABSTRACT OF COST OF ESTIMATE No. 135.-RAJBUHA OUTSETS, $\mathbf{6}^{\prime}$ WIDE (ON 3RD SECTION.)

| Quantity. |  | Description. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Rs. |
| Cubic feet | 2,780 | Masonry in foundations, at Rs. 17 per 100 , in superstructure, at Rs. 17 per 100 |  |  | $472 \cdot 60$ |
|  | 3,560 |  |  |  | $605 \cdot 20$ |
| \#, | 196 | Arch-work, at Rs. 28 per 100 |  | ... | 54.88 |
| " | 2,148 | Puddle, at Rs, 10 per 1,000 |  | ... | 21.48 |
| " | 2,780 | Excavation, at Rs. 3.25 per 1,000 | .. | ... | 9.05 |
| Number | 1 | Gate, at Rs. 32 each . ... |  | ... | $32 \cdot 00$ |
| Lineal ${ }^{\prime \prime}$ feet | 1 | Windlass, at Rs. 20 each ... |  | $\cdot$ | 20.00 |
|  | 13 | Chain, small, at Rs. 9 per foot | ... | . | 1170 |
|  |  | Stone grooving, at Re. 1 per foot | ... | ... | 14.00 |
| " |  |  | Gran | otal | 1,240.89 |

ABSTRACT OF COST OF ESTIMATE No. 136.-4' FALL, 5 BAYS OF 15,' ROADWAY 18' (ON 4TH SECTION).

| Quantity. |  | Drscription. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 12,954 | Kunkur masonry in foundation, at Rs. 17 per $100 \ldots$ | 2,202•18 |
| " | 16,087 | ", $\quad$ in superstructure, at Rs. 17 per 100 | 2,754.79 |
| " | 20,091 | $" \ggg 0$ at Rs. 20 per 100 | 4,018•10 |
| " | 4,677 | Arch-work heavy, at Rs. 32 per 100 ... ... | 1,496.64 |
| " | 240 | ., light, at Rs. 28 per 100 ... ... | $67 \cdot 20$ |
| " | 5,397 | Brick on-edge, at Rs. 25 per 100 ... ... | 1,349.25 |
| " | 382.5 | Stone capping, at Rs. 2 per cubic foot ... ... | $765 \cdot 00$ |
| " | 17,850 | Concrete, at Rs. 14 per 100 ... | 2,499.00 |
| Lineal feet | 80 | Stone grooves, at Re. 1 per foot | $80 \cdot 00$ |
| Cubic feet | 9,220 | Kunkur flooring, at Rs. 8 per 100 | 737.60 |
| " | 2,400 | Metalling roadway, at Rs. 6 per 100 | 144.00 |
| " | 55,872 | Excavation, at Rs. $3 \cdot 25$ per 1,000 | 181.58 |
| " | 6,530 | Puddle, at Rs. 10 per 1,000 | 653.00 |
|  |  | Caried over ... | 16,928.34 |

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ABSTRACT OF COST OF ESTIMATE No. 196, \&c.,-continued.


ABSTRACT OF COST OF ESTIMATE No. 137.-BRIDGE 8 ARCHES OF 80', ROADWAY $20^{\prime}$.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 1,936 | Kunkur masonry in foundations, at Rs. 17 per 100... | $329 \cdot 12$ |
| " | 6,102 | , " in superstructure, at Rs. 17 per 100 | 1,037.34 |
| " | 10,642 | " $\quad$ " at Rs. 20 per 100 | 2,128.40 |
| " | 5,123 | Arch-work heavy, at Rs. 32 per 100 ... ... | 1,639•36 |
| ", | 2,700 | Metalling, at Rs. 6 per 100 .... ... | $162 \cdot 00$ |
| " | 8,000 | Earth-work at Rs. $3 \cdot 25$ per 1,000 .... | 26.00 |
| Squ | 5,580 | Kunkur flooring, dry, at Rs. 8 per 100 ... | 446.4 |
| Square feet | 1,227 | Well foundations, as Estimate No. 181 A ... | 3,693-27 |
|  |  | Grand Total | 9,461 $\cdot 89$ |

ABSTRACT OF COST OF ESTIMATE No. 138.-BRIDGE 3 ARCHES OF 30, ROADWAY 18'.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet <br> 29 29 28 28 28 28 | 1,840 | Kunkur masonry in foundations, at Rs. 17 per 100 | 312.80 |
|  | 5,934 | ., " in superstructure, at Rs. 17 per 100 | 1,008.78 |
|  | 10,112 | " ", at Rs. 20 per 100 | 2,022 40 |
|  | 4,677 | Arch-work, heavy, at Rs. 32 per 100 ... ... | 1,496.64 |
|  | 2,400 | Metalling, at Rs. 6 per 100 ... ... | 144.00 |
|  | 7,326 | Earth-work, at Rs. 3.25 per 100 ... ... | $23 \cdot 80$ |
|  | 5,220 | Kunkur flooring, at Rs. 8 per 100 ... ... |  |
|  |  | Well foundations, as Estimate No. 130 A | 3,530.73 |
|  |  | Grand Total | 8,956.75 |


| ABSTRACT OF COST OF ESTIMATE No. 139.-SYPHON FOR DRAINAGE, 2 ARCHES OF $10^{\circ}$ (ON 4th SECTION.) |  |  |  |
| :---: | :---: | :---: | :---: |
| Quantity. |  | Description. | Cost. |
| Cubic feet | 11,280 |  | $\xrightarrow[\text { Rs. }]{\text { 1,917.60 }}$ |
| Cub | 11,278 | in saperstructure, at Rs. 17 per 100Arch-work, at Rs. 28 per 100$\ldots$ | 1,917.26 |
| " | 4,920 |  | 1,377.60 |
| " | 4.305 | Kunkur flooring, dry, at Rer 100 per $100 \times$ | 1,119•30 |
| $\pm$ | 1,040 |  | $83 \cdot 20$ |
| " | 1,365 | Puddle, at Rs. 10 per 1,000 <br> Excavation, at Rs. $3 \cdot 25$ per 1,000 ...... | 13.65 |
| " | 50,064 |  | $162 \cdot 70$ |
|  |  | Grand Total | 6,591•31 |

> ABSTRACT OF COST OF ESTIMATE No. 140.-SYPHON FOR DRAINAGE, 1 ARCH OF $3^{\prime}$ (ON 4 TH SECTION.)

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 2,970 | Masonry in foundations, at Rs. 17 per 100 |  |
| " | 4,563 | ," in superstructure, at Rs. 17 per 100 .. | $775 \cdot 71$ |
| - | 615 | Arch-work light, at Rs. 28 per 100 ... | 172-20 |
| " | 307 | \% inverts, at Rs. 26 per 100 ... | 79.82 |
| " | 320 | Kunkur flooring, at Rs. 8 per 100 ... | 25.60 |
| " | 367 | Puddle, at Rs. 10 per 1,000 -.. ... | 3.67 |
| " | 8,928 | Excavation, at Rs. 3.25 per 1,000 ... ... | 29.01 |
|  |  | Grand Total | 1,590.91 |

ABSTRACT OF COST OF ESTIMATE No. 141.-RAJBUHA OUTLET (ON 4TH SECTION.)

| Quantity. |  | Description. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cabic feet„ | 2,780 | Masonry in foundations, at Rs. 17 per 100 ") in superstructure, at Rs. 17 per 100 |  | . | Rs. ${ }_{\text {R }}$ |
|  | 2,683196 |  |  | ... | $456 \cdot 11$ |
| " |  | Arch-work light, at Rs. 28 per 10 |  | . | 54.88 |
| Number | 1,545 | Puddle, at Rs. 10 per 1,000 | ... | . | 15.45 |
|  | 2,780 | Fxcavation, at Rs. $3 \cdot 25$ per 1,000 | ... | ... | 9.03 |
|  | 1 | Gate $6^{\prime} 5^{\prime \prime} \times 5^{\prime}$, at Rs. 32 each | ... | ... | 32.00 |
| Lineal feet | 13 | Windlass, at Rs. 20 each ... | $\cdots$ | ... | 20.00 |
|  |  | Chain small, at Re. 0-9 per foot |  | ... | $11 \cdot 70$ |
| - $\quad$ | 14. | Stone grooves, at Re. 1 per foot |  |  | 14.00 |
|  |  |  | Grand Total |  | 1,085.77 |

ABSTRACT OF COST OF ESTIMATE No. 142.-REPAIRS TO MAIN LINE, GANGES
CANAL

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cabic feet Miles | $\begin{array}{r} 3,102,912 \\ 131 \end{array}$ | Earth-work, at Rs. 2.5 per 100 <br> Repairing tow-path from Roorkee to Futtegurh branch head and from Julpoori Bridge to Nanoon Regulator, at Rs. 100 per mile <br> Grand Total | $\begin{gathered} \underset{7,757 \cdot 28}{\text { Rs. }} \\ 18,100 \cdot 00 \end{gathered}$ |
|  |  |  | 20,857-28 |

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ABSTRACT OF COST OF ESTIMATE No. 143.-ALTERATIONS TO FALLS ON PRESENT LINE, 200' WATERWAY.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Rs. |
| Cubic feet " | $\begin{array}{r} 34,880 \\ 8,705 \\ 8,720 \end{array}$ | Demolition, at Re. 1 per 100 | ... ... | 348.8 |
|  |  | Masonry, at Re. 22 per 100 | ... ... | 1,915•] |
|  |  | Brick-on-edge, at Rs. 25 per 100 | ... ... | 2,180.0 |
|  |  |  | Grand Total | 4,443.9 |

ABSTRACT OF COST OF ESTIMATE No. 144.-ALTERATIONS TO FALLS ON PRESENT LINE, $150^{\prime}$ WATERWAY.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Cubic feet <br> " <br> " | $\begin{array}{r} 26,160 \\ 6,610 \\ 6,540 \end{array}$ | Demolition, at Re. 1 per 100 Masonry, at Rs. 22 per 100 Brick-on-edge, at Rs. 25 per 100 |  | Rs. |
|  |  |  | . | $261 \cdot 6$ |
|  |  |  | . | 14,54•2 |
|  |  |  | $\cdots$ | 1,635.0 |
|  |  |  | Grand Total | 3,350•8 |

abSTRaCT COST OF ESTIMATE No. 145.-REGULATING BRIDGES, FUTTEgURH BRANCH, hEAD MAIN LINE.

| Quantity. |  | Description. |  |  | Cost. |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 102822409 | Girders, at Rs. $9 \cdot 4$ per foot Iron drop gates, at Rs. 5 per foot Wooden gates, at Re. 1.25 per foot Small windlasses, at Rs. 22 each |  |  | Rs. |
| Lineal Feet Square feet |  |  | ... | ... | 958.8 |
|  |  |  | ... | ... | $410 \cdot 0$ |
|  |  |  | ... | . | $300 \cdot 0$ |
| Number |  |  | ... | ... | 198.0 |
|  |  |  | Grand Total |  | 1,866.8 |

estimate, No. 146.-71' TOW-PATHS UNDER BRIDGES, MAIN LINE.

|  | Items of Work. | Total |
| :---: | :---: | :---: |
| $\begin{array}{r} 32 \\ 7 \\ 3 \end{array}$ |  | Rs. |
|  | Tow-paths, $18^{\prime}$ Roadway, vide Estimate No. 174, at Rs. 327.30 each | 10,473.6 |
|  | Ditto 20 ditto at , $368 \cdot 50$, | 2,579.5 |
|  | Ditto 25' ditto at ,, 426.50 ," | 1279.5 |
|  | Grand Total | 14,332.6 |

J. CROFTON, Captain, R. E.

GANGES CANAL.
NAVIGABLE LINE.
ABSTRACT OF COST OF ESTIMATE No. 147.-EXCAVATION.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Cubic feet <br> " | $\begin{array}{r} 217,460,025 \\ 50,429,030 \end{array}$ | Excavation, at Rs. 2-25 per 1,000 <br> Embankment, at Rs. 6 per 1,000 | $\cdots$ | $\begin{gathered} \text { Rs. } \\ 4,89,285 \cdot 05 \\ 3,02,574 \cdot 18 \end{gathered}$ |
|  |  |  | $\mathrm{G}_{\text {band }}$ Total | 7,91,859 23 |

ABSTRACT OF COST OF ESTIMATE No. 148.—DOUBLE LOCK CHAMBER AT EXIT FROM GANGES CANAL.


ABSTRACT OF COST OF ESTIMATE No. 149.-DOUBLE LOCK, $\mathbf{6}^{\prime}$ DROP, $18^{\prime}$ ROADWAY.


ABSTRACT OF COST OF ESTIMATE No. 149.-WITH WELL FOUNDATIONS.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cabic feet <br> Lineal feet <br> Number | $\begin{array}{r} 34,489 \\ 551 \\ 78 \\ 18 \end{array}$ | Brick-work in foundations, at Rs. 17 per 100 <br> Filling in wells, at Rs. 12 per foot <br> Undersinking, at Rs. 5 per foot <br> Curbs, at Rs. 20 each <br> Remainder as Estimate No. 149 <br> Grand Total | Rs. |
|  |  |  | 5,863.13 |
|  |  |  | 66.12 |
|  |  |  | 390.00 |
|  |  |  | $260 \cdot 00$ |
|  |  |  | 18,536-26 |
|  |  |  | 25,115.51 |
| ABSTRACT OF COST OF ESTIMATE No. 149A.-DOUBLE LOCK, $6^{\prime}$ WITHOU' BRIDGE. |  |  |  |
| Quantity. |  | Description. | Cost. |
| Cubic feet$\begin{aligned} & \text { و } \\ & \text { و } \end{aligned}$ | $\begin{array}{r} 32,043 \\ 39,794 \\ 5,712 \\ 483 \end{array}$ | Masonry in foundations, at Rs. 17 per 100 <br> " in superstructure, at Rs. 20 per 100 <br> Inverts, at Rs. 26 per $100 \ldots$ <br> Arch-work, at Rs. 28 per 100 <br> Grand Total <br> Foundations, superstructure, arches, and inverts, as Estimate No. 149 <br> Difference of cost without bridge <br> Vide Estimate No. 149 | Rs. |
|  |  |  | $\begin{array}{r} 5,447 \cdot 31 \\ 7,958 \cdot 80 \\ 1,485 \cdot 12 \\ 135 \cdot 24 \end{array}$ |
|  |  |  | 15,026•47 |
|  |  |  | 17,406.05 |
|  |  |  | 2,379.58 |
|  |  |  | 24,554.77 |
|  |  |  | 22,175-19 |

ABSTRACT OF COST OF ESTIMATE No. 150.-DOUBLE LOCK, 12' DROP, 18' ROADWAY.



ABSTRACT OF COST OF ESTIMATE No. 152.-DOUBLE LOCK, 8' DROP, $20^{\prime}$ ROADWAY.


ABSTRACT OF COST OF ESTIMATE No. 153-DOUBLE LOCK, 5 ' DROP, $10^{\prime}$ ROADWAY.

| Quantity. |  | Description. |  | Total. |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Rs. |
| Cubic feet <br> 28 <br> 29 <br> 38 | $\begin{array}{r} 34,021 \\ 39,603 \\ 1,160 \\ 6,375 \end{array}$ | Masonry in foundations, at Rs. 17 per 100 " in superstructure, at Rs. 20 per 100 Arch-work, light, at Rs. 28 per 100 ... , inverts, at Rs. 26 per 100 ... <br> Carried over | ... | 5,783.57 |
|  |  |  | $\ldots$ | 7,920.60 |
|  |  |  | ... | 324.80 |
|  |  |  | ... | 1,657.50 |
|  |  |  | ... | 15,686.47 |

## ( 118 ) <br> ABSTRACT OF COST OF ESTIMATE No. 153.-continued.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  | 1 | Rs. |
|  |  | Brought forward ... | 15,686.47 |
| Cabic feet$\prime \prime$ | 1,275 | Brick-on-edge, at Rs. 25 per 100 ... ... | 318.75 |
|  | 1,398 | Metalling roadway, at Rs. 6 per 100 ... ... | 83.88 |
| " |  | Kunkur flooring, dry, as Estimate No. 149 ... | 92-16 |
| " | 7,827 | Puddle, at Rs. 10 per 1,000 ... ... | 78.27 |
| " | 47,242 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... ... | 153.53 |
| Square feet Lineal feet Number | $572 \cdot 4$ | Lock gates, at Rs. 6.5 per foot ... ... | 3,720.60 |
|  | 188 | Chain, at Re. 1 per foot . . ... ... | 188.00 |
|  | 8 | Pulleys, at Rs. 5 each ... ... ... | 40.00 |
| Number | 8 | Crabs, at Rs. 135 each ... .... ... | 1,080.00 |
| " | 8 | Sluices, at Rs. 125 each ... ... $\ldots$ | 1,000.00 |
|  |  | Additional for well foundations, as Estimate No. 149 A | 560.74 |
|  |  | Grand Total | 23,002-40 |

ABSTRACT OF COST OF ESTIMATE No. 154.-DOUBLE LOCK, 9•73 FALL, 10' ROADWAY.

| Quantity. |  | Description. |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Rs. |
| Cubic feet <br> " | $\begin{aligned} & 34,504 \\ & 53,256 \end{aligned}$ | Masonry in foundations, at Rs. 17 per 100 in superstructure, at Rs. 20 per 100 Brick-on-edge, as Estimate No. 153 Arch-work, light, as Estimate No. 153 ... inverts, as Estimate No. 153 Metalling roadway, as Estimate No. 153 Kunkur flooring, dry |  |  | 5,865.68 |
|  |  |  |  | - | 10,651•20 |
|  |  |  |  | ... | 318.75 |
|  |  |  |  | $\ldots$ | $324 \cdot 80$ |
|  |  |  |  | ... | 1,657.50 |
|  |  |  |  | - | 83.88 |
|  |  |  |  | ... | 92.16 |
| " | 9,783 | Puddle, at Rs. 10 per 1,000 | , | ... | 97.83 |
|  |  | Excavations, as Estimate No. 153 | .. | ... | 153.50 |
|  | $\begin{aligned} & 752 \cdot 89 \\ & 196 \end{aligned}$ | Lock gates, at Re. 6.5 per foot | ... | ... | 4,893.78 |
|  |  | Chain, at Re. 1 per foot | ... | ... | 196.00 |
| Lineal feet Number | 196 | Pulleys, at Rs. 5 each ... | ... | ... | $40 \cdot 00$ |
|  | 8 | Crabs, at Rs. 135 each ... | ... | - | 1,080.00 |
| " | 8 | Sluices, at Rs. 125 each ... |  | $\ldots$ | 1,000.00 |
|  |  | Additional for well foundations |  | ... | $560 \cdot 74$ |
|  |  |  | Grand | tal | 27,015•82 |

ABSTRACT OF COST OF ESTIMATE No. 155.-AQUEDUCT OVER KALEE NUDDEE, UPPER CROSSING, 2 ARCHES OF 40' SPAN.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 18,469 | Brick-work in well foundations, at Rs. 20 per 100 ... | 3,693.80 |
| Lineal feet | 96 | Sinking wells, large, at Rs. 7 per foot ... | 672.00 |
|  | 264 |  | 1,320.00 |
| Cubic feet | 8,278 | Filling in wells, at Rs. 12 per 100 ... | $993 \cdot 36$ |
| " | 91,076 | Brick-work in superstructure, at Rs. 20 per 100 ... | 18,215•20 |
| " | 10,395 | Arch-work, heavy, at Rs. 32 per 100 ... | 3,326•40 |
| " | 1,627 | Brick-on-edge, at Rs. 25 per 100 ... | 406.75 |
| " | 13,350 | Puddle, at Rs. 10 per 1,000 $\ldots$ | $133 \cdot 50$ |
|  | 15,760 | Kunkur flooring, dry, at Rs. 8 per 1,000 | 1,260•80 |
| Number | -8 | Curbs, large at Rs. 90 each... | 720.00 |
| " | 22 | , small, at Rs. 20 each | $440 \cdot 00$ |
|  |  | Grand Total | 31,181•81 |

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( 119 )
ABSTRACT OF COST OF ESTIMATE No. 156.-AQUEDUCT OVER KALEE NUDDEE, LOWER CROSSING, 3 ARCHES OF 40' SPAN.
```

| Quantity. |  | Description. | Totial. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. . |
| Cubic feet | 21,871 | Brick-work in well foundations, at Rs. 20 per $100 .$. | 4,374.20 |
| Lineal feet | 96 | Sinking wells, large, at Rs. 7 per foot ... | $672 \cdot 00$ |
|  | 336 | ",". small, at Rs. 5 per foot ... | 1,680.00 |
| Cubic feet | 9,456 | Filling in wells, at Rs. 12 per 100 ... | 1,134.72 |
| , | 143,947 | Brick-work in superstructure, at Rs. 20 per 100 ... | 28,789•80 |
| " | 19,305 | Arch-work, heavy, at, Rs. 32 per 100 ... | 6,177.60 |
| " | 2,450 | Brick-on-edge, at Rs. 25 per 100 ... ... | 612.50 |
| " | 23,120 | Kunkur flooring, dry, at Rs. 8 per 100... ... | 1,849.60 |
| '" | 18,174 | Puddle, at Rs. 10 per 1,000 ... | 181.74 |
| Number | 8 | Curbs, large, at Rs. 90 each ... | 720.00 |
| " | 28 | " small, at Rs. 20 each | $560 \cdot 00$ |
|  |  | Grand Total | 46,752•16 |

## AbSTRACT OF COST OF ESTIMATE No. 157.-AQUEDUCT OVER CHOYA NULLA, 3 ARCHES OF 30' SPAN

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 81,994 | Well foundation and sinking, as Estimate No. 156 ... Masonry in superstructure, at Rs. 20 per 100 | Rs. |
|  |  |  | 7,860.92 |
|  |  |  | 16,398.80 |
| ",", 12,994 <br> 1,925  |  | Arch-work, heavy, at Rs. 32 per $100 \ldots . \quad \ldots$ | 4,158.08 |
|  |  | Brick-on-edge, at Rs. 25 per 100 ... ... | $4 \times 1 \cdot 25$ |
| ", 18,820 |  | Kunkur flooring, dry, at Rs. 8 per 100... ... | 1,505•60 |
| " | 10,885 | Puddle, at Rs. 10 per 1,000 ... | 108.85 |
|  |  | Grand Total | 30,513•50 |

## ABSTRACT OF COST OF ESTIMATE No. 158.—BRIDGE ON WELL FOUNDATIONS, 10 ' KOADWAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 4,447 | Brick-work in foundations, at Rs. 17 per 100 | 755.99 |
| " | 16,263 | ." in superstructure, at Rs. 20 per 100 | 3,252.60 |
| , | 260 | Brick-on-edge, at Rs. 25 per 100 ... | $65 \cdot 00$ |
| " | 1,345 | Arching, heavy, at Rs. 32 per 100 | $430 \cdot 40$ |
| " | 104 | A." light, at Rs. 26 per 100 ... | 27.04 |
| " | 1,357 | Filling in, at Rs. 12 per 100 ... | 162.84 |
| " | 1,005 | Puddle, at Rs. 10 per 1,000 ... | 10.05 |
| " | 1,356 | Metalling, at Rs. 6 per 100 ... | $81 \cdot 36$ |
| " | 4,315 | Excavation, at Rs. $3 \cdot 25$ per 1,000 $\ldots .$. | 14.02 |
|  |  | Undersinking wells, as per Estimate No. 159A ... | $240 \cdot 00$ |
|  |  | Number of curbs ... ... ... | $240 \cdot 00$ |
|  |  | Grand Total | 5,279•30 |

## ABSTRACT OF COST OF ESTIMATE No. 159.-BRIDGE, $18^{\prime}$ ROADWAY.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 4,686 | Masonry in foundations, at Rs. 17 per 100 ... | 796.62 |
| " | 18,295 | in superstructure, at Rs. 20 per 100 | 3,659.00 |
| " | 468 | Brick-on-edge, at Rs. 25 per 100 ... ... | $117 \cdot 00$ |
| " | 2,173 | Arch-work, heavy, at Rs. 32 per $100 \ldots$... ${ }^{\text {a }}$ | $695 \cdot 36$ |
| " | 1,270 | " invert and light, at Rs. 26 per 100 | $330 \cdot 20$ |
| " | 2,712 | Metalling roadway, at Rs. 6 per 100 ... | 162.72 |
| " | 1,125 | Puddle, at Rs. 10 per $1,000 \ldots \ldots$ | 11.25 |
| " | 5,320 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... | 17-29 |
|  |  | Grand Total | 5,789•44 |

## ABSTRACT OF COST OF ESTIMATE No. 159 A.-BRIDGE, $18^{\prime}$ ROADWAY WITH WELL FOUNDATIONS.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 4,708 | Brick-work in foundations, at Rs. 17 per 100 ... | $800 \cdot 36$ |
|  | 168 | Arching, light, at Rs. 26 per 100 ... | $43 \cdot 68$ |
| " | 2,173 | ". heavy, at Rs. 32 per 100 | 695.36 |
|  |  | Remainder, as Estimate No. 159 | 3,967.26 |
|  | 1,357 | Filling in wells, at Rs. 12 per 100 ... | 162.84 |
| Lineal" feet | 48 | Undersinking wells, at Rs. 5 per foot ... | $240 \cdot 00$ |
| Namber | 8 | Curbs, at Rs. 30 each ... ... ... | $240 \cdot 00$ |
|  |  | Grand Total | 6,149•50 |

ABSTRACT OF COST OF ESTIMATE No. 160.—BRIDGE, 20' ROADWAY.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 4,900 | Masonry in foundations, at Rs. 17 per 100 | 833.00 |
| " | 18,803 | ,, in superstructure, at Rs. 20 per 100 ... | 3,760.60 |
| " | 508 | Brick-on-edge, at Rs. 25 per 100 ... | 127.00 |
| ," | 2,380 | Arch-work, heavy, at Rs. 32 per $100 \ldots$ | $761 \cdot 60$ |
| " | 1,391 | ", inverts and light, at Rs. 26 per 100 | 361.66 |
| ", | 3,051 | Metalling roadway, at Rs. 6 per 100 ... ... | 183.06 |
| " | 1,155 | Puddle, at Rs. 10 per 1,000 ... ... | 11.55 |
| " | 5,670 | Excavation, at Rs, $3 \cdot 25$ per 1,000 ... ... | 18.43 |
|  |  | Grand Total | 6,056.90 |

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A BSTRACT OF COST OF ESTIMATE No. 160A.-BRIDGE, 20' ROADWAY WITH WELL FOUNDATIONS.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet <br> 23 <br> " | $\begin{array}{r} 4,772 \\ 184 \\ 2,380 \end{array}$ | Brick-work in foundations, at Rs. 17 per 100 <br> Arching, light, at Rs. 26 per 100 <br> heavy, at Rs. 32 per 100 <br> Remainder, as Estimate No. 160 <br> Undersinking, \&c., as Estimate No. 159A | 811-24 |
|  |  |  | $47 \cdot 84$ |
|  |  |  | 761.60 |
|  |  |  | 4,100.64 |
|  |  |  | 642.84 |
|  |  | Grand Total | 6,364•16 |

ABSTRACT OF COST OF ESTIMATE No. 161.—BRIDGE, 25' ROADWAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 5,4.35 | Masonry in foundations, at Rs. 17 per 100 | 923.95 |
| , | 20,073 | ,, in superstructure, at Rs. 20 per 100 ... | 4,014.60 |
| " | 608 | Brick-on-edge, at Rs. 25 per 100 ... | $152 \cdot 00$ |
| " | 2,898 | Arch-work, heavy, at Rs. 32 per $100 \ldots . \quad$... | $927 \cdot 36$ |
| " | 1,694 | , inverts and light, $\mathrm{a}^{\dagger}$ R Rs. 26 per 100 ... | 440.44 |
| " | 3,898 | Metalling roadway, at Rs. 6 per 100 ... ... | 23.88 |
| " | 1,230 | Puddle, at Rs. 10 per 1,000 ... ... | 12.30 |
| " | 6,545 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... $\ldots$... | 21-27 |
|  |  | Grand Total | 6,725 80 |

ABSTRACT OF COST OF ESTIMATE No. 161A.-BRIDGE, $25^{\prime}$ ROAD WAY, BUT WITH WELL FOUNDATIONS.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cabic feet | 5,535 | Brick-work in foundations, at Rs. 17 per 100 ... | 940.95 |
| " | 224 | Arching, light, at Rs. 26 per 100 ... | 58.24 |
| " | 2,898 | \# heavy, at Rs. 32 per 100 ... | $927 \cdot 36$ |
|  |  | Remainder, as Estimate No. 161 ... | 4,434.05 |
|  | 1,696 | Filling in wells, at Rs. 12 per 100 ... ... | $203 \cdot 52$ |
| Lineal feet | 60 | Under-sinking wells, at Rs. 5 per foot ... | $300 \cdot 00$ |
| Number | 10 | Curbs, at Rs. 30 each ... ... ... | $300 \cdot 00$ |
|  |  | Grand Total | 7,164-12 |

ABSTRACT OF COST OF ESTIMATE No. 162.-CULVERT FOR DRAINAGE, 2 ARCHES OF $20^{\prime}$ SPAN.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 17,568 | Masonry in foundations, at Rs. 17 per 100 | 2,996.56 |
|  | 32,180 | \% superstructure, at Rs. 20 per 100 ... | 6,436.00 |
| " | 7,596 | Arch-work, light, at Rs. 28 per 100 ... ... | 2,126:88 |
| ", | 6,645 | ", invert, at Rs. 26 per $100 \ldots$ | 1,727.96 |
| " | 2,880 | Kunkur flooring, dry, at Rs. 8 per 100 | $230 \cdot 40$ |
|  | 1,456 | Puddle, at Rs. 10 per 1,000 ... ... | 14.56 |
| " | 23,898 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... ... | $77 \cdot 67$ |
|  |  | Grand Total | 13,600.03 |

ABSTRACT OF COST OF ESTIMATE No. 163-CULVERT FOR DRAINAGE, 2 ARCHES OF $10^{\prime}$ SPAN.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 7,283 | Masonry in foundation, at Rs. 17 per 100 | 1,238•11 |
| " | 22,300 | , in superstructure, at Rs. 20 per 100 | 4,460.00 |
| " | 2,532 | Arch-work, light, at Rs. 28 per 100 ... | 708.96 |
| " | 2,215 | , inverts, at Rs. 26 per 100 ... | $575 \cdot 90$ |
| " | 1,680 | Kunkur flooring, dry, at Rs. 8 per 100 ... | 134.40 |
| " | 728 | Puddle, at Rs. 10 per 1,000 ... ... | $7 \cdot 28$ |
| " | 8,385 | Excavation, at Rs. $3 \cdot 25$ per 1,000 | $27 \cdot 25$ |
|  |  | Grand Total | 7,151-90 |

ABSTRACT OF COST OF ESTIMATE No. 164.-SYPHONS FOR DRAINAGE, 1 ARCH OF $10^{\prime}$ SPAN.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 3,996 | Masonry in foundations, at Rs. 17 per 100 ... | 679.32 |
| " | 4,802 | , in superstructure, at Rs. 20 per 100 | $960 \cdot 40$ |
| \% | 1,266 | Arch-work, light, at Rs. 28 per 100 ... | 354.48 |
| " | 1,108 | " invert, at Rs. 26 per $100 \ldots$ | 288.08 |
| " | 520 | Kunkur flooring, dry, at Rs. 8 per 100... | $41 \cdot 60$ |
| " | 322 | Puddle, at Rs. 10 per 1,000 | $3 \cdot 22$ |
| " | 15,288 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... ... | $49 \cdot 68$ |
|  |  | Grand Total | 2,376.78 |

ABSTRACT OF COST OF ESTIMATE No. 165.-SYPHON FOR DRAINAGE, 1 ARCH OF $6^{\prime}$ SPAN.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 3,192 | Masonry in foundations, at Rs. 17 per 100 | 542.64 |
|  | 4,264 | , in superstructure, at Rs. 20 per 100 ... | $852 \cdot 80$ |
| , | 738 | Arch-work, light, at Rs. 28 per 100 ... ... | 206.64 |
| " | 686 | inverts, at Rs. 26 per 100 | $178 \cdot 36$ |
| , | 360 | Kunkur flooring, dry, at Rs. 8 per $100 \ldots$ | 28.80 |
| " | ${ }_{2} 230$ | Puddle, at Rs. 10 per 1,000 | $2 \cdot 30$ |
| " | 11,171 | Excavation, at Rs. $3 \cdot 25$ per 1,000 $\ldots$.. $\ldots$ | 36.30 |
|  |  | Grand Total | 1,847 $\cdot 84$ |

ABSTRACT OF COST OF ESTIMATE No. 166.-SYPHON FOR DRAINAGE, 1 ARCH OF 3' SPAN.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 2,075 | Masonry in foundations, at Rs. 17 per 100 | 352.75 |
| " | 3,220 | , in superstructure, at Rs. 20 per 100 | 644.00 |
| " | 369 | Arch-work, light, at Rs. 28 per 100 ... | 103.32 |
| " | 158 | " invert, at Rs. 26 per 100 ... | 41:08 |
| " | 320 | Kunkur masonry, at Rs. 8 per 100 ... | 25.60 |
| 3 | 161 | Puddle, at Rs. 10 per 1,000 ... ... | $1 \cdot 61$ |
| " | 5,794 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... ... | 18.83 |
|  |  | Grand Total | 1,187•19 |

## ABSTRACT OF COST OF ESTIMATE No. 167.-SYPHON FOR DRAINAGE, 2 ARCHES OF $6^{\prime}$ SPAN.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 5,006 | Masonry in foundations, at Rs. 17 per 100 | 851.02 |
| " | 6,022 | " in superstructure at Rs. 20 per 100 | 1,204.40 |
| \% | 1,477 | Arch-work, light, at Rs. 28 per 100 ... | 413.56 |
| " | 1,372 | " inverts, at Rs. 26 per $100 \ldots$ | 356.72 |
| " | 520 | Kunkur flooring, dry, at Rs. 8 per $100 . .$. | 41.60 |
| " | 414 | Puddle, at Rs. 10 per 1,000 | $4 \cdot 14$ |
| " | 18,638 | Excavations, at Rs. $3 \cdot 25$ per 1,000 ... ... | 60.57 |
|  |  | Grand Total | 2,932.01 |

ABSTRACT OF COST OF ESTIMATE No. 168.-SYPHON FOR DRAINAGE, 2 ARCHES OF $10^{\prime}$ SPAN.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 6,802 | Masonry in foundations, at Rs. 17 per 100 ... | 1,156.34 |
| , | 7,098 | , in superstructure, at Rs. 20 per 100 | 1,419.60 |
| " | 2,552 | Arch-work, light, at Rs. 28 per 100 ... | 708.96 |
| " | 2,216 | , inverts, at Rs. 26 per $100 \ldots$... | $576 \cdot 16$ |
| , | 1,040 | Kunkur flooring, dry, at Rs. 8 per 100... | 83.20 |
| " | 598 | Puddle, at Rs. 10 per 1,000 ... ... | 5.98 |
| " | 29,368 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... ... | $95 \cdot 44$ |
|  |  | Grand Total | 4,045.68 |

ABSTRACT OF COST OF ESTIMATE No. 169.-RAJBUHA SYPHON, 1 ARCH OF $6^{\prime}$ SPAN.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic fect | 2,517 | Masonry in foundations, at Rs. 17 per 100 ... | $427 \cdot 89$ |
| " | 2,891 | , in superstructure, at Rs. 20 per 100 | $578 \cdot 20$ |
| \% | 997 | Arch-work, light, at Rs. 28 per 100 ... | 274•12 |
| " | 200 | Kunkur flooring, dry, at Re. 8 per 100 | 16.00 |
| " | 230 | Puddle, at Rs. 10 per 1,000 . ... ... | $2 \cdot 30$ |
| " | 8,904 | Excavation, at Rs. $3 \cdot 25$ per 1,000 ... ... | 28.93 |
|  |  | Grand Total | 1,327•44 |

## ABSTRACT OF COST OF ESTIMATE No. 170.-WATERCOURSE SYPHON, <br> 1 ARCH OF $3^{\prime}$ SPAN.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 2,030 | Masonry in foundations, at Rs. 17 per 100 ... | $345 \cdot 10$ |
| " | 2,468 | , insuperstructure, at Rs. 20 per 100 ... | $493 \cdot 60$ |
| " | 489 | Arch-work, light, at Rs. 28 per 100 ... ... | $136 \cdot 92$ |
| " | 140 | Kunkur flooring, dry, at Rs. 8 per 100 ... | 11.20 |
| " | 161 | Puddle, at Rs. 10 per 1,000 ... ... | $1 \cdot 61$ |
| " | 6,231 | Excavation, at Rs. 3.25 per 1,000 ... ... | 20.25 |
|  |  | Grand Total | 1,008.68 |

J. CROFTON, Captain, R. E.

## GANGES CANAL.

MISCELLANEOUS ESTIMATES.

## ESTIMATE No. 171.-GIRDER BRIDGE 55' SPAN, 18' ROADWAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | $\begin{array}{r} 1,045 \\ 784 \end{array}$ |  | Rs. |
|  |  | Longitudinal and cross girders, rail, and fixing | 4,000.00 |
|  |  | Iron plates between girders $55 \times 19=1,045$, at Rs. 875 per foot ... <br> Kunkur, $55 \times 19 \times 75=783 \cdot 75$, at Rs. 6 per 100 | $\begin{array}{r} 914 \cdot 37 \\ 47 \cdot 04 \end{array}$ |
| " |  | $4,961 \cdot 41+55=90 \cdot 21$ per foot. Grand Total | 4,961 41 |

$20^{\circ}$ ROADWAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cabic feet <br> " | $\begin{array}{r} 1,155 \\ 866 \end{array}$ | Longitudinal and cross girders, rail, and fixing... <br> Iron plates between girders, $55 \times 21=1,155$, at Rs. 875 per foot <br> Kunkur, $55 \times 21 \times 75=866$, at Rs. 6 per $100 \ldots$ <br> Grand Total $5,062 \cdot 58 \div 55=92 \cdot 65 \text { per foot. }$ | Rs. |
|  |  |  | 4,000.00 |
|  |  |  | $1,010 \cdot 62$ 51.96 |
|  |  |  | 5,062 58 |

25' ROADWAY.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cabic feet <br> 28 | $\begin{aligned} & 1,430 \\ & 1,072 \end{aligned}$ | Longitudinal and cross girders, rail, and fixing... Iron plates between girders, $26 \times 55=1,430$, at Rs. $\cdot 875$ per foot ... <br> Kunkur, $55 \times 26 \times 75=1,072$, at Rs. 6 per 100 | Rs. |
|  |  |  | 4,000.00 |
|  |  |  | $1,251 \cdot 25$ 64.32 |
|  |  | Grand Total | 5,315•57 |
|  |  | $$ |  |
|  |  |  |  |
|  |  |  |  |
|  |  |  |  |

$$
\begin{gathered}
(126) \\
\text { ESTIMATE No. 172.-GIRDER BRIDGE } 30^{\prime} \text { SPAN, } \\
18^{\prime} \text { ROADWAY. }
\end{gathered}
$$

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet <br> 2 | $\begin{array}{r} 570 \\ 427 \end{array}$ |  | Rs. |
|  |  | Longitudinal and cross girders, rail, and fixing $\ldots$ | 1,700.00 |
|  |  | Iron plates between girders, $30 \times 19=570$, at Rs. 875 per foot ... | $498.75$ |
|  |  | Grand Total | 2,224•37 |
|  |  | $2,224 \cdot 37 \div 30=74 \cdot 14$ per foot. |  |

$20^{\prime}$ ROADWAY.


25' ROADWAY.


ESTIMATE No. 173.-TOWING.PATHS UNDER BRIDGES $12^{\prime}$ WIDE. ROADWAY $18^{\prime}$ WIDE.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Number <br> Lineal feet Square feet |  |  | Rs. |
|  | 4 | Iron uprights, at Rs. 25 each ... ... ... ... | $100 \cdot 0$ |
|  | 4 | Girders, at Rs. 30 each ... ... ... ... ... | $120 \cdot 0$ |
|  | 40 | T Iron, at Rs. 0.75 per foot ... ... ... ... | $30 \cdot 0$ |
|  | 432 | Wooden flooring, $36 \times 12$, at Rs. 0.8 per foot ... ... | $345 \cdot 6$ |
|  |  | Grand Total | $595 \cdot 6$ |

ROADWAY $20^{\prime}$ WIDE.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Number <br> Lineal feet Square feet | 5 | Iron uprights, at Rs. 25 each ... ... ... ... | $125 \cdot 0$ |
|  | 5 | Girders, at Rs. 30 each ... ... ... ... ... | ]50.0 |
|  | 40 | T Iron, at Rs. 0.75 per foot ... ... ... ... | $30 \cdot 0$ |
|  | 456 | Wooden flooring, $38 \times 12$, at Rs. 0.8 per foot ... ... | 364.8 |
|  |  | Grand Total | $669 \cdot 8$ |

ROADWAY $25^{\circ}$ WIDE.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Number <br> Lineal feet Square feet |  |  | Rs. |
|  | 6 | Iron uprights, at Rs. 25 each ... ... ... ... | $150 \cdot 0$ |
|  | 6 | Girders, at Rs. 30 each ... ... ... ... ... | $180 \cdot 0$ |
|  | 40 | T Iron, at Rs. 0.76 per foot ... ... ... ... | $30 \cdot 0$ |
|  | 516 | Wooden flooring, $43 \times 12$, at Rs. 0.8 per foot ... ... | 412.8 |
|  |  | Grand Total | $772 \cdot 8$ |

$$
\text { ( } 128 \text { ) }
$$

ESTIMATE No. 174.-TOWING-PATH UNDER BRIDGES $7^{\prime}$ WIDE. - ROADWAY 18' WIDE.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Number <br> Lineal feet Square feet | 4 | Iron uprights, at Rs. 25 each ... ... ... | $100 \cdot 0$ |
|  | 4 | Girders, at Rs. 17.5 each ... ... ... ... | 70.0 |
|  | 30 | T Iron, at Rs. 0.75 per foot .. ... ... ... | 22.5 |
|  | 231 | Wooden flooring, $33 \times 7$, at Rs. 0.8 per foot ... ... | $184 \cdot 8$ |
|  |  | Grand Total | 377-3 |

ROADWAY $20^{\prime}$ WIDE.

| Quantity. |  | Description. |  |  |  |  |  |  |  |
| :---: | ---: | :--- | :--- | :--- | :--- | ---: | :---: | :---: | :---: |

ROADWAY $25^{\prime}$ WIDE.

| Quantity. |  | Description. | Cosr. |
| :---: | :---: | :---: | :---: |
| Number <br> Lineal feet Square feet |  |  | Rs. |
|  | 6 | Iron uprights, at Rs. 25 each ... ... ... | $150 \cdot 0$ |
|  | 6 | Girders, at Rs. 17.5 each ... ... ... ... | 105.0 |
|  | 30 | T Iron, at Rs. 0.75 per foot ... ... .... ... | 22.5 |
|  | 280 | Wooden flooring, $40 \times 7$, at Rs. 0.80 per foot ... | 224.0 |
|  |  | Grand Total | 501.5 |

ESTIMATE No. 175.-TOWING-PATH ON CAWNPOOR BRANCH.


$$
\text { ( } 129 \text { ) }
$$

ESTIMATE No. 176.-MUMHOODPOOR FALLS, WITH WOOD AND IRON GRATING.

| Description. | No. | L. | B. | D. | Quantity. | Total. | Gravd Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Masonry, videEstimate No. 17 | $\cdots$ | $\ldots$ | $\cdots$ | . | $\ldots$ | $\ldots$ | 29,061 |
| Arch-work ... ... ... | $\cdots$ | $\cdots$ | $\cdots$ | ... | .... | $\ldots$ | 2,520 |

ABSTRACT OF COST OF ESTIMATE No. 176.-WITH WOOD AND IRON GRATING.

| Quantity. |  | Description. |  | Cost. |
| :---: | :---: | :---: | :---: | :---: |
| Cubic feet <br> " | $\begin{array}{r} 29,061 \\ 2,520 \end{array}$ | Masonry, at Rs. 22 per 100 Arch-work, at Rs. 28 per 100 Grating, \&c., vide Estimate No. 17 |  | $\begin{gathered} \text { Rs. } \\ 6,393 \cdot 42 \\ 705 \cdot 60 \\ 8,332 \cdot 80 \end{gathered}$ |
|  |  |  | Grand Total | 15,431 82 |

ESTIMATE No. 176.-WITH STONE GRATING.

abstract of cost of estimate No. 176.-WITH Masonry grating.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet$"$$"$ | 26,441 10,067 2,563 | Masonry, at Rs. 22 per 100 <br> Arch-work, at Rs. 28 per 100 <br> Stone-work, at Rs. 2 per foot | $\begin{gathered} \text { Rs. } \\ 5,817 \cdot 02 \\ 2,818 \cdot 76 \\ 5,136 \cdot 00 \end{gathered}$ |
|  |  | Total cost with masonry grating Total cost with grating of wood and iron | $\begin{aligned} & 13,771 \cdot 78 \\ & 15,431 \cdot 82 \end{aligned}$ |
|  |  | Difference of Cost | 1,660.04 |

ESTIMATE No. 177.-FALLS ON 1st SECTION ALTERNATIVE LINE, 11 BAYS OF $15^{\prime}$ ROADWAY, 18 ' WITHOUT LOCKS.

| Description. | No. | L. | B. | D. | Quantity. | Total | Grand Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Foundations. |  |  |  |  |  |  |  |
| Water-wings, up-stream ... | 2 | 42 | 5 | $8 \cdot 5$ | 3,570.0 |  |  |
| Drop wall of fall .. | ... | 190 | $5 \cdot 5$ | 11.5 | 12,017.5 |  |  |
| Curtain wall, down-stream .. | ... | 190 | $4 \cdot 5$ | $5 \cdot 5$ | $4,072 \cdot 5$ |  |  |
| "'. ${ }^{\text {e }}$ " |  | 190 | 4 | 4.5 | 3,420.0 |  |  |
| Flooring of fall ${ }^{\text {\% }}$ | $\cdots$ | 190 | 62 | 1.5 | 17,670.0 |  |  |
| Revetments below fall ... | 2 | 158 | 5 | 10 | 15,800.0 |  |  |
| Foundations of 1 bridge, 4 of $51-18^{\prime}$ roadway | \} Fid | Estima | No. 1 |  | 16,160.0 | 73,340 |  |
| Deduct- |  |  |  |  |  |  |  |
| Hollows, up-stream waterwings | 2 | 42 | 2 | 4 | 672 |  |  |
| Hollows, down-stream revetments ... | 2 | 156 | 2 | $6 \cdot 5$ | 4,108 |  |  |
| Hollows, up-stream waterwings of bridge | 2 | 52 | 4 | 8 | 3328 | 8,108 |  |
| Superstructure. |  |  |  |  |  |  |  |
| Water-wings, up-stream ... | 2 | 42 | $3 \cdot 25$ | 12 | 3,276.0 |  |  |
| Side walls of fall ... | 2 | 35 | 5 | 11 | 3,850.0 |  |  |
|  | - | 35 | $3 \cdot 5$ | $12 \cdot 5$ | 3,062.5 |  |  |
| Sloping portion of wall ... | 2 | 12 | 2 | 3 | 144.0 |  |  |
| Side walls of fall ... | 2 | 35 | ${ }_{3}$ | 5 | 1,750.0 |  |  |
| Pie ${ }^{\prime \prime}$ ".. | 2 | 35 | $3 \cdot 5$ | $12 \cdot 5$ | 3,062.5 |  |  |
| Piers of fall ... | 10 | 31 | $2 \cdot 5$ | 22 | 17,050.0 |  |  |
|  | 10 | 3 | 2.5 | 12 | $900 \cdot 0$ |  |  |
| Backing of arches over fall | 22 | $7 \cdot 5$ | 3 | 1.5 | $742 \cdot 5$ |  |  |
| Counterforts of foot-path bridge | 2 | 3 | $3 \cdot 5$ | 7 | 147.0 |  |  |
| Retaining walls of foot-path bridge | 2 | 15 | 8 | $2 \cdot 5$ | $600 \cdot 0$ |  |  |
| Retaining walls below fall... | 2 | 158 | 3 | 12 | 11,376.0 |  |  |
| Counterforts of revetments | 10 | 2 | 2 | 10 | $400 \cdot 0$ |  |  |
| Superstructure of 1 bridge, 4 of $51-18^{\prime}$ roadway ... | \} Tid | stim | No. |  | 58,503.37 |  |  |
| Up-stream water-wings of bridge ... | 2 | 52 | $2 \cdot 5$ | 12 | 3,120.0 |  |  |
| Brick-on-edge $5^{n \prime}$ thick ... | $\ldots$ | 190 | 3 | $\cdot 5$ | 285.0 |  |  |
| Dif ${ }^{\prime} \mathrm{l}^{\prime}$ ", ... | ... |  |  |  | 12,920.0 |  |  |
| Difference be- Parapets... | ... | 14 | 75 |  | 21.0 |  |  |
| tween bridge $\quad$, $\ldots$ | ... | 14 | $1 \cdot 5$ | $1 \cdot 5$ | $31 \cdot 5$ |  |  |
|  |  | 14 | 2 | 1 | $28 \cdot 0$ |  |  |
| out $\quad$ Piers ... | 3 | 21 | $6 \cdot 5$ | 3 | 1,228.5 |  |  |
| Tow-path, \&c. $\begin{gathered}\text { Tow-path } \\ \text { walls ... }\end{gathered}$ | 2 | 21 | $2 \cdot 5$ | 12 | $\underline{\text { 1,260.0 }}$ | 18,894 |  |
| Carried over ... | $\ldots$ | ... | $\cdots$ | $\ldots$ | ... | ... | 151,291.87 |

## ( 131 )

ESTIMATE No. 177.-FALLS ON 1st SECTION ALTERNATIVE LINE, 11 BAYS OF 15', ROADWAY 18 ' WITHOUT LOCKS,-continuer.

| Description. | No. | L. | B. | D. | Quantity. | Total | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Brought forward Arching. | $\cdots$ | $\cdots$ | $\cdots$ | $\cdots$ | $\cdots$ | $\ldots$ | 151,291•87 |
| Over fall ... .. ... | 11 | 16 | 3 | 1 | 528 | 528 | , |
| As Estimate No. 107, inverts | $\cdots$ | 14 | 21 |  | 6,426 441 |  |  |
| Deduct $\dddot{\text { As Estimate }}$ No. $107 \%$ bridge | $\cdots$ | 14 | 21 | $1 \cdot 5$ | 441 |  |  |
| Arches ... ... ... | $\ldots$ |  |  |  | 13,734 | 5,985 |  |
| Deduct ... | $\ldots$ | 14 | 21 | 3 | 882 |  |  |
| Brick-on-edge 3' bridge ... | $\ldots$ | 190 | 3 | . 5 | 285 | $\begin{array}{r} 13,205 \\ 799 \end{array}$ | 19,365 |
| Flooring of fall $\mathbf{1}^{\prime}$ thick ... | $\cdots$ | 190 | 68 | 1 | 12,920 |  |  |
| timate No. 101 | $\cdots$ | $\cdots$ | $\ldots$ | $\cdots$ | $\cdots$ |  | 12,406 |
|  |  |  |  | Grand Total, Brick-work |  |  | 183,062.87 |
| Concrete bed | $\cdots$ | 190 | 70 | 3 | 39,900 |  | 39,900 |
| Kunkur flooring ... ... | $\ldots$ | 190 | 20 | 2 | 7,600 | 7,600 |  |
| Puddle. |  |  |  |  |  |  | 7,600 |
| Abotments ${ }^{\text {Drop- }}$ | 2 | 70 | $20 \cdot 5$ | $1 \cdot 5$ | 4,305 | 19,125 |  |
| Drop-wall of fall .. ... | $\ldots$ | 190 | 20 | 9.5 | 7,600 |  | 19,125 |
| " ... | $\cdots$ | 190 | 4 | $9 \cdot 5$ | 7,220 |  |  |
| Basin of fall | $\cdots$ | 190 | 65 | 4 | 49,400 | 122,740 |  |
| Foundations ... ... | $\cdots$ | .. | .. | ... | 73,340 |  |  |
| Metalling .. ... ... | ... | $\ldots$ | ... | $\cdots$ | 5,400 | ... | 122,740 |
|  |  |  |  |  |  |  | 5,400 |

ABSTRACT OF COST OF ESTIMATE No. 177.

| Quantity. |  | Description. | Cost. |
| :---: | :---: | :---: | :---: |
| Cubic feet | 65,232 | Brick-work in foundations, at Rs. 20 per 100 ... | $\underset{13,046 \cdot 40}{\text { Rs. }}$ |
| " | 47,140 | ", in superstructure, at Rs. 20 per 100 ... | 9,428.00 |
| " | 38,920 | " ", at Rs. 24 per 100 ... | 9,340•80 |
| " | 12,852 | Arch-work, heavy, at Rs. 32 per 100 ... | 4,112.64 |
| ", | 528 | " light, at Rs. 28 per 100 ... | $147 \cdot 84$ |
| " | 5,985 | \# inverts, at Rs. 26 per $100 \ldots$ | 1,556.10 |
| " | 12,406 | Brick-on-edge, at Rs. 25 per 100 ... | 3,101-50 |
| Lineal feet | 799 | Stone capping, as Estimate No. 101, at Rs. 2 per foot | 1,598.00 |
| Cubic feet | 39,900 | Concrete, at Rs. 14 per 100 .. ... | 5,586.00 |
| Lineal feet | 264 | Stone grooving, as Estimate No. 101, at Re. 1 per foot | 264.00 |
| Cubic feet | 7,600 | Kunkur flooring, dry, at Rs. 8 per $100 \ldots$... | 608.00 |
| n | 5,400 | Metalling, at Rs. 6 per 100 | 324.00 |
| ," | 19,125 | Puddle, at Rs. 10 per 1,000 | $191 \cdot 25$ |
| " | 122,740 |  | 39890 |
| \% |  | Gratings, \&c., as Estimate No. 101 | 9,534 87 |
|  |  | Grand Total | 59,238.30 |

ESTIMATE No. 178.-LOCK ON MUHOODPOOR FALLS.


ABSTRACT OF ESTIMATE No. 178.

| Quantity. |  | Description. |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Rs. |
| Cubic feet | 2,250 | Demolition, at Re. 1 per $100 \ldots$ | $\ldots$ |  | 22.50 |
|  | 53,060 | Masonry, at Rs. 22 per 100 ... |  |  | 11,673.20 |
| Square feet | 1,113 | Lock gates, at Rs. 6.5 per foot | ... | ... | 7,234-50 |
| Number | 4 | Crabs, at Rs. 135 each .. ... |  |  | $54.0 \cdot 00$ |
| ", | 4 | Sluices, at Rs. 125 each |  |  | $500 \cdot 00$ |
| rim | 4. | Pulleys, at Rs. 5 each ... ... | ... |  | 20.00 |
| Lineal feet | 104 | Chain, at Re. 1 per foot ... |  |  | 104.00 |
| " | 36 | Draw-bridges, at Rs. 60 per foot |  |  | 2,160.00 |
|  |  |  |  | nd T | 22,254 20 |

$$
(133)
$$

ESTIMATE No __(?) $\qquad$ (?)


ABSTRACT OF ESTIMATE No.——(?)

| Quantity. |  | Description. |  | Total. |
| :---: | :---: | :---: | :---: | :---: |
|  | . |  |  | Rs. |
| Cubic feet | 966,490 | Excavation, at Rs. $2 \cdot 25$ per 100 | $\ldots$ | 2,174.60 |
|  | 3,773 | Masonry, at Rs. 22 per 100 | ... ... | $830 \cdot 06$ |
| Square feet | 1,188 | Grating, at Re. $1 \cdot 15$ per foot | ... ... | 1,366.20 |
| Lineal feet | 104 | Girders, at Rs. $4 \cdot 25$ per foot | ... ... | $442 \cdot 00$ |
|  | 44 | Wall-plate, at Rs. 6.25 per foot | ... ... | $275 \cdot 00$ |
| Cubic feet | 900 | Stone, at Rs. 2 per foot ... | ... | 1,800.00 |
|  |  |  | Grand Total | 6,887•86 |

ESTIMATE No. 179.—SOONDA BRIDGE.

| Description. | No. | L. | B. | D. | Quantity. | Total | Grand Total. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Demolition, vide Estimate No. 35 | ... | $\ldots$ | $\ldots$ | $\ldots$ | $\ldots$ | ... | 16,543 |
| Masonry. |  |  |  |  |  |  |  |
| Raising piers . ... ... | 2 | 21 | $6 \cdot 5$ | 5 | 1,365•00 |  |  |
| Filling spaces in abutments | 2 | 21 | 6 | $4 \cdot 75$ | 1,197•00 |  |  |
| Raising wings ... ... | 4 | 35 | 2.5 | 5 | 1,750.00 |  |  |
| " pillars ... ... | 4 | 4 | 4 | 5 | 320.00 |  |  |
| Backing of arches, vide Estimate No. 34 | $\ldots$ | $\cdots$ | $\cdots$ | ... | 3,024.00 |  |  |
| Spandrils, vide Estimate No. 34 ... | $\ldots$ | $\ldots$ | $\ldots$ | ... | 2,037•50 |  |  |
| Parapets ... ... ... | 2 | 235 | $1 \cdot 5$ | 2.87 | 2,023•35 |  |  |
| Pillars ... ... ... | 4 | 3 | 3 | 2.87 | $103 \cdot 32$ |  |  |
| Embankment ... ... | 2 | 450 | 35 | $2 \cdot 5$ | 78,750 |  |  |
| Lifling and relaying metal... | $\ldots$ | 500 | 15 | .75 | 5,625 |  |  |
| Arch-work ... ... ... | 3 | 54 | 21 | $2 \cdot 25$ | 7,654•5 |  |  |
|  |  |  |  |  |  | 7,654 | 7,654 |

ABSTRACT OF ESTIMATE No. 179.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet | 16,543 | Demolition, at Re. 1 per 100 ... ... ... | $165 \cdot 43$ |
|  | 11,820 | Brick-work, at Rs. 22 per 100 ... ... | 2,600.40 |
| " | 7,654 | Arch-work, at Rs. 32 per 100 ... ... | 2,449-28 |
|  |  | Tow-path, 12 feet wide ... ... ... | $595 \cdot 60$ |
|  | 78,750 | Embankment, at Rs. $2 \cdot 25$ per 1,000 ... ... | $177 \cdot 18$ |
| ", | 5,625 | Metalling, at Re. 1 per 10) ... ... ... | 56.25 |
|  |  |  | 6,044 14 |
|  |  | Amount of estimate with girders | 15,205•25 |
|  |  | Difference of cost . ... . | 9,161•11 |

$$
(135)
$$

ESTIMATE No. 180.-TEMPORARY OVER-FALL ON DIVERSION, 200' WATERWAY.


ABSTRACT OF ESTIMATE No. 180.

| Quantity. |  | Description. | Total. |
| :---: | :---: | :---: | :---: |
|  |  |  | Rs. |
| Cubic feet. <br> Number <br> Cubic feet.. | $\begin{array}{r} 3,871 \\ 11,009 \\ 40 \\ 58,000 \end{array}$ | Timber in abutments, at Rs. 0.62 per foot ... ... | 2,400.02 |
|  |  | ,", Weir, at Rs. 0.62 per foot ... ... | 6,8.25.58 |
|  |  | Cribs, at Rs. 20 each ... ... ... ... ... | $800 \cdot 00$ |
|  |  | Boulders, at Rs. 7 per 100 ... ... ... ... | 4,040.00 |
|  |  | Grand Tomal | 14,085 60 |
|  |  | Cost of Weir $=$ Rs. $11,685 \cdot 58+200=58.85$ per fo | t, say Rs. 59. |

## ( 136 )

ESTIMATE NO. 181.—DIVERSIONS OF MAIN CHANNEL ABOYR FUTTEGURH BRANCH HEAD.

| Description. |  | No. | L. | B. | D. | Contents. | Total. | Grand <br> TutaL. |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Excavation and filling | $\ldots$ | $\ldots$ | 500 | 150 | 10 | 750,000 | 750,000 | 750,000 |
| Bridge-of-boats | $\ldots$ | $\ldots$ |  |  |  |  |  |  |

ABSTRACT OF COST OF ESTIMATE No. 181.


No.-(?)-FROM FUTTEGURH BRANCH HEAD TO NANOON.

| Description. | No. | L. | B. | D. | Contents. | Total. | Gramp <br> Total. |  |
| ---: | ---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Excavation and filling | $\ldots$ | $\ldots$ | 500 | 115 | 10 | 575,000 |  |  |
| Bridge-of-boats | $\ldots$ | $\ldots$ |  |  |  |  |  | 575,000 |

ABSTRACT OF COST OF ESTIMATE No.-(?)

| Quantity. |  | Description. | Cosr. |
| :---: | :---: | :---: | :---: |
|  |  |  | Re. |
| Cubic feet <br> Lineal feet | $\begin{array}{r} 575,000 \\ \quad 150 \end{array}$ | Excavation and filling, at Rs. 4 per 1,000 | 2,300 |
|  |  |  | 750 |
|  |  | Piling to protect slopes, at Rs. 5 per foot <br> Bridge-of-boats | . 800 |
|  |  | Grand Total | 3,850 |

## R E P OR T

of the

## gavges canal committer,

CONYENED BY ORDER OF HIS EXCELLENCY THE GOVERNOR GENERAL OF INDIA IN COUNCLL ( N . 20,410C. OF GOVERNMENT OF INDIA, PUBLIC WORKS DEPARTMENT, DATED 24TH FEBRUARY, 1866), TO' DECIDE UPON THE PROPRIIETY OF PROCEEDNG, AS PREVIOUSLY DETERMINED WITH

MAJOR CROFTON'S PROJECT FOR REMODELLING THE GANGES CANAL,

or
OF STOPPING ITS PROGRESS, PENDING THE PREPARATION OF A DETALLED PROJECT

## ACCORDING TO THE VIEWS OF MAJOR-GENL. SIR ARTHUR COTTON, R.E., K.S.I.

WITH A COMPARISON OF THE COST AND ADVANTAGES OF THE TWO PLANS.

ROORKEE:
PRINTED AT THE THOMASON CIVIL ENGINEERING COLLEGE PRESS. mdCcclevi.

JAMES JOHNSTON, SUPERINTENDENT.

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> Report of a Committee convened by Order of His Excellency the Governor General of India in Council (No. $20,410 \mathrm{c}$., of Government of India, Public Works Department, dated 24th February, 1866), to decide upon the propriety of proceeding, as previously determined, with Major Crofton's project for remodelling the Ganges Canal, or of stopping its progress, pending the preparation of a detailed project according to the views of Major-General Sir Arthur Cotton, R.E., with a comparison of the cost and advantages of the two plans.

President.<br>Colonel Commandant Edward Lawrord, R.E.

Members.

Lt.-Col. J. C. Anderson, R.E. George Sibley, C.E.<br>Lt.-Col. J. G. Fife, R.E. $\quad$ Huge Leonard, M.I.C.E., F.G.S.

The Committee having met at Cawnpore on the 31st March, proceeded, accompanied by Major J. Crofton, R.E., to examine the Canal Works from the river Ganges up to near the Dubowli Falls, and also the river itself in the vicinity of the town. They then went to Agra and examined the river Jumna at tlat place, and for a few miles lower down.

The Ganges was examined near Rajghat, opposite Allygurh, and the Committee proceeded for about eight miles along the canal above Nanoon, after inspecting the Regulating Works at that place. The river Jumna was examined at the confluence of the Hindun, and afterwards at Alee, a small village near the ruins of Toghlukabad, about ten miles below Delhi.

The Committee then returned to the Ganges at Gurmuktesur, and from thence marched along the edge of the high land, to the con. fluence of the Solani torrent with the Ganges, examining its junction with the strip of low ground along the margin of the river nearly the whole way, and more carefully in the vicinity of the Solani. Crossing the high land to the canal, the Committee then proceeded along its banks from the Jaoli Falls to the head at Hurdwar, examining each important work as closely as circumstances permitted, and termina-
ting their inspections with the temporary bunds in the river and the head works of the canal.

The Committee also saw some of the places near Hurdwar from whence stone has been obtained for the canal works.
Conclusions antived After much discussion and careful consideration of every point at. bearing on the subject, the Committee have arrived at the following conclusions, on grounds explained in the subsequent detailed reports.

Weir across the
Ganges near the So-
lani. £1,128,631
I. That the construction of a weir across the Ganges below the confinence of the Solani with other necessary works for supplying water to the canal, at an estimated cost of Rs. 1,12,86,314 cannot be recommended.
Weir and canal at II. That the project for opening an additional canal head, in-
Rajghat. £1,180,417. cluding the construction of a weir on the Ganges at Rajghat, or other point in that part of the river, at a cost of Rs. 1,13,04,170, for bringing under irrigation lands not now watered by the canal is feasible, but should be held in abeyance until the probable returns appear more proportionate to the outlay than at present.
Weir across the
Jumna, and canal for III. That the construction of a weir across the Jumna at Togh-
irrigation.
£354,570. lukabad with a canal for the irrigation of that part of the Doab below Allyghur, not under the influence of the Ganges canal, at a probable cost of Rs. 35,45,701, inclusive of branch channels, is practicable, and that the project should be further investigated; but they are of opinion that it cannot be substituted for any portion of Major Crofton's project.

Major Crofton's pro-
ject. ject.
IV. That Major Crofton's project for remodelling the Ganges canal should be proceeded with, subject to the modifications suggested in this Report.
Weir at Hurdwar.
V. That the construction of a permanent weir across the Gan- ges at Hurdwar, though not indispensable while the present reduced quantity of water is passed down the canal, will become a matter of absolute necessity in order to maintain without risk of interruption the full supply of $\mathbf{7 , 0 0 0}$ cubic feet per second.

## PROPOSED HEADS FOR THE GANGES CANAL.

## PRELIMINARY OBSERVATIONS.

1. As the Committee think that the peculiarities of the Ganges Peculiarities of are somewhat remarkable, and that a comprehension of them is indis- markable. pensable before an opinion of any value can be formed as to the relative advantages of different sites for the opening out of new canals, they will endeavour to describe them.
2. The Plains in the N. W. Provinces, unlike the Deltas on the East Coast, are far above the limit of the inundation of the Provinces, in relation rivers. In the course of ages the Ganges and the Jumna have scoop- through them. ed out their beds to a far lower level than they must have had at some former period, and this operation has completely changed the relation of the land with the rivers. The N. W. Provinces for a distance of several hundred miles from the base of the hills are thus formed of a vast plateau, which rises many feet above the highest flood level of the Ganges and Jumna. This elevated tract is termed the Bangur, in contradistinction to the Khadir, or the strip of low land in the actual valleys of the rivers, and which is more or less inundated by them during high floods. The fall of the Bangur is not exactly the same as that of the rivers; or, in other words, the plane of the surface is not parallel to the surface of the rivers. That of the Ganges is more elevated near the hills than it is at any point lower down. For instance at Sookertal, at the distance of 34 miles from the hills, it is 80 feet above the flood level of the Ganges opposite ; and at Cawnpore, 350 miles below Hurdwar, the elevation is only 38 feet. But the difference is in a nearly regular gradation, so that knowing the height of the Bangur above the river at any two points, say 40 miles apart, it may be assumed with confidence that the height at the point midway between them would be almost exactly the mean of the other two.
3. A range of low hills called the Sewaliks runs in a nearly Unbroken plain straight line across the head of the Bangur land from the Ganges Allahabad.
to the Jumna. From their base to the junction of the two rivers at Allahabad, the great plain is unbroken by a single elevation larger than a sand hillock.
4. The drainage water from the southern slopes of the Sewalik Drainage from the
range has to find its way into the Ganges and Jumna. The portion due to the latter, that is the Western section, runs for the most part through the Bangur, and united with the minor drainages form the river Hindun which joins the Jumna below Delhi. But the drainage courses on the eastern section of the Sewaliks turn off abruptly towards the Ganges, constituting a series of torrents which have completely altered the character of the portion of the Bangur traversed by them, that is along a distance of about 20 miles, extending between the well known towns of Hurdwar and Roorkee. Hurdwar is on the east flank of the Sewaliks and close to the Ganges; Roorkee is on the Bangur land beyond the influence of the hill drainage.

The country slopes along the drainage courses towards the Ganges between Hurdwar and Roorkee : great expense of car-
rying canal across the rying cana
drainage.
5. Thus the original formation of the Bangur from Hurdwar to Roorkee is broken. It has in fact been washed down by the torrents, which have thus formed a plane sloping towards the Ganges. But this plane also has a slope parallel to or along the Ganges, so that when selecting the line for a Canal to open from the Ganges at Hurdwar, the Engineer could obtain the required fall (indeed there was more than sufficient), and could at the same time work up the course of the drainages, so as after crossing the last of them, to be able to enter the Bangur land, in a moderate cutting. Thus the point selected for crossing the Solani, the last of the hill torrents, is 80 feet below the head of the canal at Hurdwar, but it is 70 feet above the level of the Ganges immediately opposite. The aqueduct across the Solani raises the sole of the canal 20 feet, and the level of the Bangur at Roorkee being 45 to 50 feet below the head of the canal; it can be entered in a cutting 10 to 15 feet in depth. Besides the advantages thus gained by opening the canal at Hurdwar the two other important objects have been attained. I. The water from the river enters the canal charged with much less silt than would have been the case had the head been lower down. II. The channel being of a comparatively permanent nature, the sluice and regulating works are secure against the action of the stream, which can be more easily controlled, and by temporary arrangements more easily diverted towards the mouth of the canal, than at any point below. On the other hand, the Hurdwar head involved an enormous expense in masonry works for the Solani and other hill torrents which the canal had to cross, and for overcoming this excessive slope of country.

Sir A. Cotton in favor of a head near Sookertal.
6. Sir A. Cotton maintains that it would have been far better to have opened the canal head in the first instance near Sookertal, below the confluence of the Solani with the Ganges; and that even now, instead of spending a large sum to improve the existing line, it would be preferable to open a new head about Sookertal.

[^2]7. The Committee refer to this point, and find that the valley of the Ganges is four miles wide. The river is only about half a mile in width during the dry season, but in high floods the whole of the valley is submerged.


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8. The Khadir, or low land forming the valley, is formed of a valletreof the Kadir or crust of alluvial soil, which may vary from the thickness of a few inches to a yard or more, overlying pure and exceedingly fine micaceous sand, which extends to an unknown depth. The Khadir everywhere possesses the same character, but is wider at one point than another ; and while in some places the dry weather channel runs nearly through the centre of it, in others it may run on one flank or under the foot of the Bangur, as for instance at Rajghat, Futtehghur, and Cawnore.
9. The Khadir is intersected by deserted arms of the river, $\begin{gathered}\text { Khadir frequently }\end{gathered}$ some of which have become choked by deposits, till there is only a the river.
depression, to mark their course. The Khadir cannot properly be termed the channel of the river though mostly inundated in high floods; and it is liable to be, and frequently is, encroached upon by the action of the stream, so that at no single point can it be said to be safe from its influence. The experience of some of the Members of the Committee on the Ganges in Bengal, the Punjab rivers, and the Indus in Scinde, could furnish proofs of the extraordinarily rapid and extensive action of the Himalayan rivers in eroding their banks and altering their courses. Instances could be adduced of villages and trees being carried away bodily, even in ordinary floods, not by direct action of the stream during inundation, but by its undermining action before it rose to the level of the bank. The sand is sucked away by the current when the river is rising or falling, and the crust of stiff alluvial soil falls in masses by its own weight. There is thus no stability in the Khadir. If embanked, the action of the river being on the sand at a low level, the embankment would have no strength to resist the stream, and no material is procurable on the spot for protecting the foot of the slope.
10. It would be irrelevant to seek for examples of the destruc-

Changes at Sooker tive and changeable character of the Himalayan rivers at points at a at on the Ganges, and distance from those which do not more or less nearly concern the ${ }_{\text {ne }}^{\text {Hindun with the Jom- }}$ distance from those which do not more or less nearly concen the ni. Ganges canal, but below Sookertal, the very point which Sir A. Cotton has selected as the most suitable head for the canal; the river instead of flowing close to the foot of the Bangur land, as it is shown in the Trigonometrical Survey sheets, has now a course more than one and a half miles to the eastward, while the confluence of the Hindun with the Jumna, about 20 miles below Delhi, which Major Crofton has similarily pointed out as a likely place for a head to the canal, is now four miles lower than it was, when the Trigonometrical Survey was made.
11. The banks of the Godavery and Kistna are formed of alluvial soil containing a large proportion of clay down to and below the ${ }_{\text {manent }}^{\text {and }}$ summer level of the water: here and there strata of sand are to be seen, but as a rule the banks are firm.
12. On the Ganges it is different. Sir A. Cotton seems for- Sir A. cotton and

Major Haig, on the merly to have thought so also, as will be seen from the following
Godavery. extract from one of his Reports on the Godavery.
Profesional Papers, "It must be observed that these changes, from the action of the Vol. III., p. 190. river on its banks are much slower on the Godavery than in the Colleroon and Cauvery, and as it seems also on the Indus and Ganges; so that there is plenty of time to take the necessary precautions; and the stone we have here at hand is a material for barriers far beyond the power of the stream to remove, even though in pieces of the size of the fist." "In comparing the present state of the river with the map constructed 30 years ago, it will be seen how very slowly the encroachments of this river proceed." Major Haig bears testimony to the same effect. The following is an extract from his Report, dated
Parliamentary Ble 3 rd February, 1862. "In the first place the Godavery differs from the Book on the Upper
Godaser, 1862 , p. 75. . . liable to erosion and subject to little change. The changes which take place on the Ganges in the course of a few weeks or months would be on this river the work of a century, or rather, it would be more correct to say that the country along its banks presents incontestible proofs that no material alteration has taken place in the position of the channel for ages. In the second place, the material of which the bed is composed is a large course grained sand, totally unlike the fine powdery sand of the Ganges, and therefore requiring a very much more powerful current to displace it."
13. The obvious conclusion to be drawn from the above facts is Works for con-that, although the Ganges for a considerable distance from the Himatrol of Ganges ree
quire to be on a more layas discharges during floods, a very much smaller body of water extensive scale than those on the Goda very. than the Godavery, yet owing to the weakness of its banks and the long continuance of its floods, it is so much more destructive than the latter, that works on a more extensive scale in proportion to its discharge would be required to control it.

## HEAD WORKS NEAR SOOKERTAL.

## FLOOD DISCHARGE AND LENGTH OF WEIR REQUIRED.

Flood discharge of
14. The Committee have no means of ascertaining with any Ganges at Sookertal high degree of accuracy the flood discharge of the Ganges. Mr. Login estimated it at between 180,000 and 190,000 cubic feet per second at Hurdwar ; not however including the quantity discharged by the Myapoor Dam and the canal, which would probably amount to 20,000 cubic feet per second more. Sir Proby Cautley again

Ganges Canal Re port; Vol. IL., p. 465. estimates the flood discharge of the Solani at 84,000 cubic feet per second, and the discharge of the other large drainages between the Solani and Hurdwar, must amount to at least as much more. $\mathbf{A}^{-}$ considerable body of water must also enter at the opposite bank of
the river. Hence it may be concluded that the flood discharge of the Ganges below the confluence of the Solani would be much greater than at Hurdwar. From the cross section which has been taken at Sookertal below the Solani, combined with the surface fall, it is found that a rise of $13 \frac{1}{2}$ feet above the ordinary low level, would give a discharge of 516,000 cubic feet per second; but this is probab'y an extraordinarily high flood which might not be experienced under an interval of 20 years. According to the Registers, which were kept at Cawnore and Futtehghur from 1843 to 1853, the highest flood at the former was 13 feet 8 inches, and at the latter 10 feet 8 inches, above low level. The highest rise at Cawnpore is probably attributable to the influx of the Ram Gunga, and 10 feet 8 inches, or say 11 feet, is more likely to represent the rise between Futtehghur and Hurdwar. Applying this to the section at Sookertal, the Committee find the discharge would be 279,000 cubic fect per second. Taking Sir A. Cotton's estimate of the flood discharge of the Godavery, at $\mathbf{1 , 5 0 0}, 000$ cubic feet per second, and that of the Ganges at $\mathbf{2 8 0 , 0 0 0}$ cubic feet; it might be assumed that if the rise of the flood were the same in both cases and the circumstances in other respects similar, a weir of less than one-fifth the length of the Godavery anicut would suffice for the Ganges at Sookertal. As the length of the former is about $\mathbf{1 2 , 0 0 0}$ feet; the length of one for the Ganges would thus be about 2,200 feet.
15. But the circumstances of the two rivers differ to a notable Low water comextent. The Ganges discharges a considerable body of water in the of $\begin{gathered}\text { pared with flood level } \\ \text { and } \\ \text { Gooda- }\end{gathered}$ dry season, with a depth in mid channel of between 9 and 10 feet, and ${ }^{\text {very. }}$ the floods rise from 10 to 14 feet above the level. The Godavery carries a comparatively small body of water in the summer. The surface is only 3 or 4 feet above the bed, while the flood rise to the height of 30 feet. The crest of the Godavery anicut is 14 feet above the bed. The passage of the flood water is therefore 16 feet deep. The length of the work being 12,000 feet, the mean velocity will be $\frac{1,500,000}{16 \times 12,000}=8$ feet per second nearly.
16. Sir A. Cotton, in one of his pamphlets, suggests a solid weir Length and height for the Ganges, 7 feet high above summer or low level. If the length $\begin{gathered}\text { of wiae } \\ \text { wir }\end{gathered}$ of the weir were to have the same proportion to the discharge as the Godavery ; and if, as Sir A. Cotton intended, there should be no perceptable afflux or heaping up of the water above the work, it would, as above-nientioned, be 2,200 feet long, so that as the rise in flood is 11 feet, the velocity would be $\frac{280,000}{2,200 \times 4}=32$ feet a second nearly, or four times as much as that of the Godavery. If the length of the weir were to be increased to 4,000 , the velocity would then be $17 \frac{1}{2}$ feet per second, or still very much greater than that of the Godavery; but if the height of the weir be cut down from 7 feet above low water to 5 feet, there would then be a velocity of $\frac{280,000}{4,000 \times 6}=11 \frac{2}{3}$ feet per second,
or nearly 8 niles per hour. The afflux of the water above the natural level at that height of flood would be less than $1 \frac{1}{2}$ foot; there would therefore be no cause for apprehending injurious flooding of the Khadir. The above velocity corresponds very closely with that of the Kistna anicut, with a depth on the crest of 17 feet. Owing to the curve formed by the water in falling over the dam during moderate floods, the depth of water on a portion of it is less than at the crest, and the velocity is then considerably higher. There would be this same thing with the overfall on a weir on the Ganges. Were the discharge during an exceptionally high flood to amount to $\mathbf{5 0 0 , 0 0 0}$ cubic feet per second, which would be the case if the river rose to $13 \frac{1}{2}$ feet above summer level, the velocity would then be $\frac{500,000}{4,000 \times 8 \cdot 5}=14$ feet 7 inches per second, or 10 miles per hour.
Length of weir proposed at 4,000 feet:
height above low wa- and its height above the bed of the river is 20 feet. What is considered
ter 5 feet.
17. The length of the $K$ istna anicut at Bezwada is 3,600 feet, ter 5 feet. suitable for the Ganges is a weir 4,000 feet long, and a height of 5 feet added to the ordinary depth of the river during the dry season, or in all 15 feet above the deep bed.
Additional height
18. For the supply of a canal the surface might be raised in of 2 or 3 feet to be gained by moveable planks. the dry season as suggested by Sir Arthur Cotton, by means of iron posts and moveable planks, to a height of 3 feet at the utmost above the crest of the weir, or 8 feet above low water level; but the Committee consider that 2 feet for the planking is as much as could be calculated on in practice, and that the weir would either have to be raised or the channel deepened one foot additional.
Discharge of Ganges at Sookertal during the dry season.
19. The discharge of the river near Sookertal was measured in April of this year, and found to be 5,300 cubic feet per second, while a supply of upwards of 5,000 cubic feet per second was entering the canal at Hurdwar, a portion of the latter having been returned to the river by the mill escapes. The river was then slightly higher than it had been during the winter, and allowing that the present supply of the canal is maintained, 5,000 cubic feet would be the most that could be counted on as available at Sookertal.

Capacity and fall of channel to carry 5,000
cubic feet per second.
20. A channel 180 feet bottom width, side slopes 2 horizontal to 1 vertical, and with a fall of $5 \frac{1}{2}$ inches per mile, would discharge that quantity with a depth of 10 feet, and velocity of $2 \frac{1}{8}$ feet per second. The latter is the highest the lighter kinds of soil could stand, while any reduction would render necessary an additional width of channel and an additional expense, besides causing an increased deposit of silt. The bed of the channel would thus be 2 feet below low water level of the river.

Most suitable site
21. The most suitable spot for the head to the canal in the for a weir at Raolee Ghât, $4 \frac{1}{2}$ miles below Sookertal.
neighbourhood of Sookertal is at the bridge of boats, $4 \frac{1}{2}$ miles below that village, known as the Raolee Ghât. The level of the surface of the river in April was 200 below the head of the Ganges canal, and
the head of the proposed new channel would therefore be 208 feet below the same datum.
22. The bed of the Ganges canal at the $116 \frac{2}{3}$ mile from Hurdwar is 242 below the head of the canal, the distance of the site of the pro- wair would be main miles posed weir to this point, measured along the line which is considered best calculated to meet Sir A. Cotton's views, is 70 miles, and the fall required to carry a supply of 5,000 cubic feet a second with a velocity of $2 \frac{1}{2}$ feet per second, and depth of 10 feet, being $5 \frac{1}{2}$ inches per mile, there will be $\frac{70 \times 5 \frac{1}{2}}{12}=32$ feet 1 inch, as the total fall from the head. Adding this to the relative level above-mentioned 208 feet, gives 240 feet 1 inch, which corresponds closely enough with the level of the present canal bed at $116 \frac{2}{3}$ mile.
23. Before entering upon an estimate of the cost of constructing a weir across the Ganges, the Committee propose to specify the cribed. approximate cost of the channel. It would be carried along the Khadir or low land for a distance in all of 29 miles. The surface of the Khadir falls much more rapidly than the proposed bed of the canal, and as the total height of the Bangur land above the Khadir is between 50 and 60 feet, it is desirable, in order to avoid a cutting of that depth, to carry the bed of the canal for some distance above the level of the Khadir, or in other words within embankments. For the last 13 miles of this section of the canal it is proposed to follow the base of the Bangur land, as the ground is considerably higher there than the general level of the Khadir. It however presents great irregularities. The edge of the Bangur is cut up by ravines, some of which extend back to the distance of about half a mile, and their beds are only slightly above the Khadir level. The section therefore presents a series of sharp undulations, which will necessitate a cutting in one place, and a heavy embankment in another. According to the arrangements proposed, the maximum height of the water level will be 24 feet above the ground, and as the soil is generally very sandy, this is considered as much as could be allowed with any degree of safety. A simple earthern embankment would not be sufficient along a portion of the channel under consideration. Puddling would be the most effectual mode of preventing leakage, but as suitable earth would have to be brought from a distance, the Committee recommend, as an addition to the ordinary soil obtainable on the spot, a rough concrete wall in the centre of the bank for ten miles out of the whole distance of thirteen miles. This would also have the effect of protecting the bank against vermin, a precaution which the Committee consider to be very necessary. Only one embankment where the channel runs close to the Bangur is allowed The water is supposed to stand back in the ravines, which would thus form silt traps, but the raising of the bed would only be a question of time, and sooner or later the drainage water would have to be carried under the channel, or a large body of silt would be brought into it; for the drainage courses, though
embracing an insignificant area, drop from the Bangur 50 feet above the level of the channel, and the erosion must evidently be considerable.

Drainage works ultimately necessary. head of the ravines, would cut off part of the drainage water, but still it is considered that the portion of the channel affected by it would always be liable to damage, and that the ultimate cost of the work is likely to be much greater than an approximate estimate based on a simple section of the ground would show. In taking the cost of this part of the channel into consideration the additional expense of constructing under-tunnels and catch-water drains should not be lost sight of.

Ohjection to high embankments.
25. Some of the Committee consider that it would be objectionable to carry an embanked channel through the Khadir, with the surface water more than 4 or 5 feet above the level of the ground: that in consequence of the.sandy character of the soil, embankments could not be guarded effectually against excessive percolation or breaches, excepting at a much heavier outlay than the present estimate provides for. They would further remark that the large body of drainage water that will be admitted into the channel, will increase the danger of breaches, and that in the event of the bank giving way in the rains, it could not in all probability be repaired till the next dry weather.

Deep cutting through Bangur land, and masonry works.
26. At the end of the 29th mile the channel would enter the Bangur in a cutting of about 35 feet, which would work out to 15 feet at the 52 nd mile. From thence to the junction with the present line of canal the cutting would be moderate. Masonry works are allowed for on the scale of Major Crofton's estimate for the 1st section of the "alternative line," proposed by Sir P. Cautley (after deducting the cost of the falls and regulator at head), which was to have the same capacity as the channel now under consideration. The rate allowed for contingences and establishment, is also the same as in Major Crofton's estimate. The masonry works would be very heavy as the channel from beginning to end is carried across the drainage of the country. No separate allowance is made for catch-water drains or for the masonry works which will eventually be required to dispose of the drainage of the Bangur where the line of channel runs at its foot.

Estimate of cost of channel.
27. The Committee estimate the cost of the channel at nearly $68 \frac{3}{4}$ lakhs of rupees. An abstract of the cost of the different portions of it is given in the Appendix.
Form of weir for
28. As regards the proposed weir across the Ganges a section has been proposed which assimilates to a considerable extent to that of the Godavery anicut. The form of overfall has however been changed. Sir A. Cotton has expressed his opinion on the subject in the following terms; after he had completed the Godavery anicut, "I must however say that here as in the Coleroon, I would now prefer Professional Papers,
Madras
Engineers, building a work with a vertical fall as safer in sandy rivers; and so it Vol. III., p. 189.
would in any place excepting one similar to the Kistna, where there


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Digitized by GOOgle
is unlimited supply of stone at very low rates, and where consequently 20 cubic yards of rough stone could be obtained at the same cost as one of cut stone. When I planned the Coleroon anicut, I considered that the great point in these rivers was to break the force of the wạter effectually, and prevent it scouring the lower channel; and what I have here seen makes me think still more of the importance of that principle. But I believe where stone can be obtained very cheaply, a large mass of rough stone with a very long slope on the lower side, will be the cheapest and safest work in a sandy river."
29. It will be observed that a depth of 15 feet has been allowed $\underset{\text { tions. }}{\text { Depth }}$ of founderfor the well foundations. This depth though considerably in excess of that allowed in the Madras works, is about half what is generally considered necessary in the N. W. Provinces in the foundation of falls, and other works subjected to a scouring action. The plans of Madras anicuts, as at first constructed, do not show the works in their present state; for example, the rough stone in rear of the Godavery anicut extended in the first instance to a width of only 75 feet, now according to a section which the Committee have received from the Superintending Engineer of the division, it extends to a width varying from 120 to 150 feet. The anicut across the Kistna has also received considerable additions since it was first built, and large masses of rough stone have been thrown in along the front of the works, which do not appear in any plan. The Committee learn from Colonel Dyas, that several dams in Rohilkund have lately given way, owing to the want of a stone apron; vitrified lumps of kiln refuse, the only material available, having proved an altogether inefficient substitute.
30. In designing a weir across a river like the Ganges, the foundations have to be secured against two distinct actions: firstly, the jected. scouring action which is produced when the stream meets with an obstruction, and which would undermine and destroy the foundations, unless they are either carried down to the full depth to which the scouring extends, or unless they are protected by a mass of stone or other suitable material; and secondly the pressure arising from the water on the up-stream side of the weir being ponded up above the level of the water on the down-stream side. Unless the pressure is counteracted by the formation of the foundation wells or filling between them into a water-tight screen, it will force the water through between them, and by carrying with it the sand under the body of the work would cause it to subside and fall to pieces.
31. An instructive example of the scouring action is furnished in $\underset{\text { ing action on the } 60}{\text { Exar- }}$ one of the Reports of the Godavery works. One of the sections of ${ }^{\text {ing avery anicut. }}$ the anicut had not been carried completely across the channel, but for a distance of 260 yards an embankment and wing wall had been sub- Madras $\begin{gathered}\text { Mrofesional Papers, } \\ \text { Engineers, }\end{gathered}$ stituted. The course of the stream being obstructed by these works, and the only outlet for the water being over the anicut, a current was formed towards it along the face of the wing wall, and that of the
anicut itself. The result was the formation of a deep channel far below the bottom of the foundations, and a reference to the sections will show that the only thing that saved them was the deposition of a great mass of rough stone.*

Example of effect of hydranlic presssure on Coleroon anicut. Professional Papers,
Madras
Engineers, Vol. L, p. 144 .
32. As an example of the effect of hydraulic pressure, the Committee quote the following from a report by Colonel D. Sim, on the Coleroon anicuts. "This anicut was completed about the end of April, and was breached the following June during one of the freshes, about 80 yards of the northern division having been entirely destroyed; various causes have been assigned for the failure, and it is difficult at this time to ascertain which is the true one. It has been supposed by some to have been caused by a tall upright stone which had been fixed in the body of the work where the breach occurred, to indicate the height of the river in freshes, having got entangled in a quantity of straw and bushes floating down the stream, the large surface exposed by which acted upon by a powerful current tore it out of the anicut, apd the water thereby getting access to the masonry which had not had sufficient time to set, easily destroyed the work. Others attributed the failure to the anicut having been undermined by the water being forced through its foundation by the heavy pressure during high freshes, and washing from under it the sand on which it rested. This appears to me the most probable cause of the failure of the lower anicut which was breached at the end of the second season, but I think the upper one could scarcely have been destroyed in that manner in the short period of a fortnight or three weeks. The water it is evident was forced under the foundation, and it was observed bubbling up in many places through the apron below, the anicuts whenever there were 5 or 6 feet standing above them, and if it passed in considerable quantities, which there is reason to believe it did, it would be very liable to wash away the sand by degrees and leave the work without support. During the last two years both anicuts have been materially strengthened by substantial aprons of cut stone in chunam - being constructed behind them, to break the overfall of water, which have been executed in a very efficient manner. Since these additions the passage of the water underneath the foundation would seem to be considerably diminished, for it now.spouts through the apron in only a few places and in small quantities, but I am not inclined to attribute this improvement so much to the aprons as to the large quantities of fine clay, and which has been collected in front of the anicut, and prevents the water being forced underneath them."
33. To apply the above facts to the case before the Committee, they have to remark that the length of the proposed weir, which for

[^3]economical reasons has been limited to 4,000 feet, is only nine-sixteenths the width of the channel of the river during high floods. It is certain, therefore, that a scouring action similar to that above described as having occurred on the Godavery anicut, would be liable to be induced along the flanks of the work, and that they would require either a great depth of foundation or have to be protected in front by a large quantity of rough stone. To what depth the Ganges is liable to scour when it encounters such an obstruction as it would then meet with, there is no means of ascertaining; but it is known that the sand of which the bed and banks are formed is of a remarkably mobile quality, partaking in fact of the nature of quicksand, and that it is not unusual for the beds of the Himalayan rivers under conditions similar to those of the Ganges at Sookertal, to be scoured to a depth exceeding 20 feet below the ordinary level, when the stream sets against an exceptionally firm piece of bank. It may therefore be reasonably concluded that the foundation of a weir on the Ganges would require to be protected to a depth of at least 30 feet below the bed. It is proposed to allow 800 feet in length of wing walls for the up and down-stream sides together, on each flank of the weir, or 1,600 running feet in all; but in addition to this, the river would either have to be trained by means of rough stone groins for a considerable distance above the weir, or if defensive measures were confined to the embanked roadway which would have to be carried through the Khadir land, a large quantity of stone would have to be thrown in along the foot of its slope. It is impossible to judge precisely to what extent this protection would be necessary, but it is indispensable that precautions should be taken to resist the action of the river at any point against which the stream would be liable to set, and as this would be the case throughout the whole width of the Khadir, stone must be provided for the protection of the whole embankment. It would not necessarily be thrown in until the embankment should be threatened, but it would have to be collected at the spot ready for use at a moment's notice. The total length of bank to be thus protected would be $4 \frac{1}{2}$ miles. For this it is proposed to provide stone of the section $30 \times 7$ for a length of one mile which would have to be divided over the whole $4 \frac{1}{2}$ miles at such intervals as should in practice be found necessary.
34. As regards the undermining action generated by the simple pressure arising from the head of the waters on the weir, it is usual to puddle with clay in front and between the foundation walls to as great a depth as possible, but it would be difficult in the Ganges to work such material through the natural bed of semifluid silt, and it would be unsafe to wait until a scouring action should deepen the bed, before arrangements should be made for covering the front of the wells with a layer of clay. The actual process could hardly be arranged beforehand, but in one form or another it would be indispensable.

The closing of the dam must also be a difficult and expensive process. Captain Orr, in one of his Reports on the Kistna anicut,

Professional Papers, Madras Engineers, Vol. IV., p. 44. mentions "the almost insuferable difficulties we met with in closing the Rallee branch of that river (Godavery), and the alarming doubt that for some time existed of our being able to so at all."

Summer discharge of Godavery and Ganges compared.
35. The discharge of the Godavery, according to Captain Haig, ranges from 3,750 cubic feet per second in February to 1,500 cubic feet in the first half of June. 'Ihe discharge of the Ganges at the site of the proposed weir is $\mathbf{5 , 0 0 0}$ cubic feet per second. It is possible that when the Godavery anicut was closed there was an exceptionally high supply in the river; but the same contingency might occur with the Ganges.

Heary contingencies in constructing a
weir across tbe Ganges.

Rates.
36. In preparing an estimate for a weir, it may therefore be assumed that the expenditure on temporary embankments and in baling out water, and the losses occasioned by interruption of the works by unseasonable floods, and by the damage that would be produced by the floods passing over an unfinished work, would be very heavy; and that 50 per cent. would not be too large an addition to make to the rates at which the work (excepting cut stone) could be carried out for on dry land, free from all the drawbacks that have been mentioned.
37. The ordinary rates are as follows :-Cut stone from Bhurtpore, Rs. 2-8 per cubic foot; it is possible however that stone of suitable quality may be obtained from the Himalayas at Rs. 2, if the road from thence to Hurdwar, 16 miles in length, is made practicable for heavily laden carts. For rough stone, boulders might be used, but as they would have to be brought from some distance above Hurdwar, and as the carriage would be very costly, it will probably be advisable to use blocks of concrete instead. Any quantity of shingle can be obtained from the bed of the river $\mathbf{2 5}$ to 30 miles above the site of the proposed weir. It cannot be delivered at a lower rate than an equal quantity of brick, and allowing that the extra quantity of lime which would be used would be nearly an equivalent to the saving of bricklayers, it may be assumed without great risk of error that the concrete would cost much the same as brickwork, that is Rs. 20 for the commonest work.
Estimate for weir 44 lakhs.
33. The estimate for the weir amounts to 44 lakhs of rupees. This may appear an inordinate amount compared with the cost of the large works of a similar kind which have been executed in the Madras Presidency, but the extra cost of the material of the Ganges weir serves in a great measure to explain the difference.

[^4]39. The anicut across the Kistna at Bezwada, cost only Rs. $6,42,000$, though a considerably larger amount of material was expended than has been provided for the Ganges weir. But at Bezwada there are inexhaustible quarries of easily worked but serviceable stone within 300 yards of the flanks of the work, and the

rough stone of which it was mainly constructed cost only 11 annas per cubic yard, or a fraction over Rs. 2-8 per 100 cubic feet. The cut stone cost Rs. 5, and the largest blocks Rs. 9 only, per cubic yard or from 3 to $5 \frac{1}{3}$ annas per foot. Rough stone or concrete for the Ganges weir could not be delivered under eight times the Bezwada rate; or suitable cut stone under Rs. 2-8 per foot.

40. Some doubt is entertained as to the possibility of laying the concrete or brick foundation of the weir to the depth shown in the $\begin{aligned} & \text { Ganges meir might } \\ & \text { have to } \\ & \text { be constroct }\end{aligned}$ section, namely 10 feet below low water level. Should it be found un- ${ }^{\text {ed on a higher level. }}$ practicable, partly by the aid of baling and partly by working in water, to attain to this depth, the only alternative, without altering the plan would be to construct the floor on a higher level. This would necessitate an extension of the rough stone apron at a corresponding additional outlay.
41. A weir on the pattern of the Bezwada anicut could be constructed more expeditiously than the one proposed by the Committee, tiously bo more expedibut it is questionable whether any saving would be effected in first bat woold not be leas construction, while the annual repairs would be very costly.
42. The construction of under sluices at one or both flanks of Great cost of mell weir must necessarily be a very expensive operation, as the whole of slanices. the floors would have to be founded on wells, and as the capacity of the sluices would not only have to be sufficient to discharge the ordinary supply of the river during the dry season, but the exceptionally high one, which is liable to be brought down in January and February.
43. In the above enquiry, the valuable experience which has Experience gained been gained in the construction of weirs in the Madras Presidency viem by Committee. has been kept in view, and the plan adopted for a weir on the Ganges, differs no further from those approved in Madras, than is considered necessary to meet the peculiarities of the Ganges, which have been previously described.
44. One of the members of the Committee not satisfied as to the stability of the wells 15 feet in depth, or the feasibility of con-member of the Comstructing the flooring from a level 10 feet below low water, has drawn out a section for a weir, on the principles which are generally followed in the N. W. Provinces, when 'a scouring action has to be provided against.
45. The cost of a weir on this pattern would be much the same weir monld cost as that of the other, the extra cost of the wells being counterbalanced $\begin{aligned} & \text { mall fond the same if } \\ & \text { mans } \\ & \text { mere }\end{aligned}$ by the saving in rough stone.
46. It is generally supposed, or by at least a large section of the Engineers in other parts of India, that the foundations of bridges and other works in the N. W. Provinces are extravagant, but in many places rough stone is so costly as to render deep well foundations the most economical procedure that could be pursued, for securing a work against the action of a powerful current.

Cost of weir and channel together, 112 lakhs.
47. The total cost of a weir across the Ganges, near Sookertal, and a channel from it to the present main line of canal to carry 5,000 cubic feet per second would thus be $112 \frac{3}{4}$ lakhs of rupees.
Compared with Ma-
48. As Major Crofton's estimate for the rectification of the whole length of canal from Hurdwar to the head of the Cawnpore Branch, including compensation and loss of water-rents, if the canal were closed for a year, amounts to about $\mathbf{4 5 \frac { 1 } { 2 }}$ lakhs, (Major Crofton's Report, p. 61,) it is evident that the Sookertal project cannot be recommended as a substitute.

Water in Ganges insufficient for full supply of canal at Hurdwar, and a separate canal at Sookertal.
49. When the additional works provided for by Major Crofton shall have been completed, the canal will be competent to carry the supply originally contemplated of 7,000 cubic feet per second. Now, as the Committee have before observed 5,000 cubic feet per second is the maximum* supply likely to be available at Sookertal, when an equal quantity is entering the canal at Hurdwar, there is not likely to be much above $\mathbf{3 , 0 0 0}$ cubic feet available when 7,000 are aùmitted at Hurdwar. In that event the great cost of a weir across the Ganges, which must be the same, whatever quantity might be taken from the river at Sookertal, would seem to render the consideration of a separate project for a head from that site superfluous.
50. The remarks however which have been made in the section ghat project. of the Committee's Report relating to the proposed head from Raj- ghat, when a supply somewhat in excess of 3,000 cubic feet per second would probably be available, are applicable with slight modifications, to a project for conveying $\mathbf{3 , 0 0 0}$ cubic feet from Sookertal.

## HEAD WORKS ON THE GANGES BELOW SOOKERTAL.

## GURMUKTESUR.

51. The examination which the Committee made of the river at and about Gurmuktesur, led them to conclude that the site is unsuited for headworks; the great extent over which the river travels, the width of its bed, the unfavorable nature of its banks, combined with the scarcity of good building material in the neighbourhood, render it a most objectionable position for any such project.

## RAJGHAT.

52. The point on the Ganges which seems best adapted for the contruction of the necessary works either for supplementing the lower part of the canal, or for supplying water for irrigating additional land on that part of the Doab, is in the neighbourhood of Raj-
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ghat, nearly opposite Allyghur. The banks of the river and the adjoining land there, seemed to be sufficiently favorable for the purpose, to warrant such a survey of the locality as would supply details for estimating the cost of the work.
53. The Ganges canal plans, and the Trignometical Survey maps $\begin{gathered}\text { Steps taken to col-- } \\ \text { lect }{ }^{\text {nntormation }} \text { for }\end{gathered}$ were first examined in order to ascertain the best trace to follow in estimating. the more detailed enquiry; the line was then levelled, a cross section and rough survey of the river made, and the discharge at low water measured. From these data the accompanying estimate has been drawn up.
54. The construction of a weir across the Ganges at this site

Difficalty of estimating cost of weir. will be attended with similar difficulties and heavy expense as at Sookertal, nor can the examples of the Madras works be fairly applied to the case, for it cannot be too clearly explained that the difficulty and cost of such undertakings does not depend so much on the heights of the floods of a few weeks duration as on their long continuance, and on the volume and steadiness of the permanent stream, the nature of the bed and banks of the river, and the facilities of procuring materials, on all which points there are essential differences between the Godavery and the Ganges. The Committee-while careful to estimate the cost of the works as correctly as possible-are aware, that some degree of uncertainty must exist, and feel it their duty to guard against the necessity of further outlay except for boná fide repairs; the weir has therefore been designed with every regard to stability, and as much economy as circumstances will allow.
55. The quantity of flood water for which provision has been

Flood discharged and dimensions of weir. made is $\mathbf{3 0 0 , 0 0 0}$ cubic feet per second, that is 20,000 feet more than has been provided for at Sookertal-see note on that project. It is proposed to build the weir 4,500 feet long, to raise the low water level 8 feet, $5 \frac{1}{2}$ by a masonry weir, and $2 \frac{1}{2}$ feet by a moveable board, and the flood level about $1 \frac{1}{2}$ feet; this height would be attained during very high floods only, and would not injure the low land to any important extent. There would be a velocity of about 12 feet per second over the weir in high floods.
56. The design for the work has been prepared with a view to using as far as possible the materials which are available in the district. It differs from the Sookertal weir so far, that the wells are designed to be sunk 30 feet below the bed of the river, and consequently, less protection of loose stone has been provided for. The most important of the local materials is the block kunker, it is found at a distance of about fourteen miles from the river; the cost in a rough state, delivered on the bank is about Rs. 10 per 100 cubic feet. Cut stone must be brought from Bhurtpore; the cost at Muthra is Rs. 1 per cubic foot, carriage from Muthra to Rajghat (a distance of about 80 miles) Rs. 1, dressing and setting, Rs. 0-8, a total of Rs. 2-8 per cubic foot.
57. The extent and cost of training and protection walls and trection malls.
embankments is very uncertain; the right bank of the river is good and would therefore require very light works, but the left bank is subject to much erosion, and consequently would require heavy works and constant attention. dry season.
58. The quantity of water passing Rajghat on the 10th April, 1866, when the Ganges was at a low level-some of the villagers said it was at its lowest for the year-was $\mathbf{5 , 6 3 0}$ cubic feet per second; while a measurement made at Cawnpore on the 5th of the same month, gave 5,438 . The quantity for which provision is to be made in the new channel is 5,000 cubic feet per second, which is considered a fair quantity to fix for the purpose of this estimate; but it is tolerably certain that so much would not be available during very dry seasons with the canal taking its full supply from Hurdwar, while if a canal were to be opened from Sookertal to take off the whole supply of the river in the dry season at that point, the supply that would then be available at Rajghat would be very small.
59. In fixing the dimensions of the channel the fall has been made as great as the nature of the soil permits, the object kept in view being to give as small a sectional area as possible through the deep cutting in the first twenty miles. By diminishing the slope and increasing the sectional area, the length of the line might be shortened; but the decrease in quantity by shortening, would be much more than counterbalanced by the additional width of excavation in the deep cutting. As the results of his enquiry, Major Crofton has adopted 2.5 feet per second as the highest velocity which the lightest of this kind of soil can stand, the same has been adopted here; consequently, the sectional area required to carry the quantity fixed upon is 2000 square feet. The depth of water to be carried has been fixed at 10 feet, and the side slopes of the channel at 2 to 1 , with berms of 12 feet wide at 3 feet above high water.

Rate for excavation and cost of masonry works.
60. The rates for works have been fixed as nearly as possible in accordance with the prices now being paid in the district, making such additions for the great depth of cutting and consequent distance to which the spoil must be carried as were considered necessary. The number of bridges over the line has been estimated on the same principles as those by which the Ganges Canal Officers were guided; the rule being, that there should be a bridge at about every third mile. Very accurate estimates of the cost of crossing the small rivers, could not be made without waiting for detailed surveys and observations, which would cause much delay ; they are, however, perhaps sufficiently accurate for a merely comparative estimate of this kind, the best information available having been collected from both Railway and Canal Officers.

Project cannot be substituted for Major Crofton's on account of cost.
61. The estimate for the work amounts to-

| Channel, Weir, | - - |  | $\ldots$ |  |  | -. | $\begin{aligned} & 67,54,550 \\ & 45,50,280 \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | al, | -• | -• | 18,04,830 |

this sum is for conveying 5,000 cubic feet of water per second from the Ganges to a point in the Cawnpore branch of the canal about 18 miles below the Nanoon regulator. The whole cost of Major Crofton's scheme for remodelling the canal to carry the full supply down to this point does not amount to half this sum, it is therefore clear that the project cannot be recommended as a substitute in any way for his scheme.
62. As an independent scheme for supplying additional water for the irrigation of new ground in the lower part of the Doab, the cost of forming distributing channels and rajbuhas must be added to the estimate quoted above. The cost of the Cawnpore and Etawah branches of the Ganges canal, which pass about 3,500 cubic feet per second, will be about Rs. $\mathbf{2 9 , 0 0 , 0 0 0}$; taking the cost of passing 5,000 in the same proportion, the amount would be $42,00,000$. The cost of the rajbuhas for the Ganges canal will be about $53,00,000$, this is for distributing 6,750 cubic feet per second; supposing the cost of distributing 5,000 to be in the same proportion, the rajbuhas for the Rajghat project would be $37,00,000$, making the total cost, say 1,92,00,000.
63. The cost of the Ganges canal water is estinated by Major Crofton at 4,740 Rs. per cubic foot per second passed through the head works; this, however, includes the cost of navigation, for which a deduction of one-tenth may be made, making the cost for irrigation alone $\mathbf{4 , 2 6 0}$. The estimated cost per foot per second of the supply from Rajghat would be $\frac{192,00,000}{5000}=3,840$ Rs. per cubic foot per second, so that the water would be a little less expensive than that of the Ganges canal.
64. The cost of clearing silt from a channel taken off from the Ganges at a point so low down as this, must be much greater than the cost of clearing a channel taken off at a point so near the hills as Hurdwar ; but, on the other hand, this channel would be much shorter than the Hurdwar line, and consequently it would require less repairs and less establishment, and would not probably on the whole be more expensive to maintain.

65: Again, supposing the quantity of land which may be irri-

Silt would be trouhlesome, but other annal expenses would be light. gated by each cubic foot per second to be 200 acres, a result now attained on the Jumna canal-and one hoped to be attained on the Ganges canal-and suppose the value of water to be Rs. 2-8 per acre, which is about the average rate now charged on the Ganges canal, and that the profits be taken at Rs. 1-8 per acre, those of the Ganges canal in 1863-64 being Rs. 1-6 out of Rs. 2-4; the result would be a profit of 8 per cent on the capital.
66. It must however be remembered that 5,000 feet per second $\begin{gathered}\text { s.000feet per recond } \\ \text { not alweys }\end{gathered}$ cannot always be relied on at Rajghat. If the Ganges canal were taking its full supply, that is about $\mathbf{3 , 0 0 0}$ feet per second more than

Project as an independent scheme.

Cost of Ganges canal water and of water from the Rajghat project compared.


Retarn on capital. not always certain.
was entering at the time when the Ganges measurements above referred to were made, two-thirds of 5,000 would be fully much as could be calculated upon, while such a limited supply would continue the profits would be reduced to about 5 per cent.

Conclusion.

River near Agra.
67. The Committee therefore conclude that the project can not be considered a reasonably profitable one to undertake at present; but it is not at all improbable that the value of water will rise, and that the quantity distributed per acre will be greatly reduced without any injury to the consumer, and if so, the project may be worthy of more careful examination at some future period.

## HEAD WORKS ON THE JUMNA NEAR AGRA.

68. The difference of level between low water at Agra and the high land is 61 feet, and the point at which water could be brought to the Etawah branch, allowing a fall on the channel of 6 inches per mile, is 116 miles from Agra, and within 27 miles of the termination of the branch. The Committee considered that on account of this great length of channel, and depth of cutting, and proximity to the termination of the present canal-beyond which are heavy drainages : further examination of that scheme was unnecessary.

## NEAR DELHI.

69. The point on the Jumna which seems best adapted for head

River near Delhisite well suited for head works.
works, and for the formation of a channel to bring the water on to the land to be irrigated, is close to the village of Alee, about ten miles south of Delhi, and just opposite to the old fort of Toghlukabad. The selection of this site involves the necessity of building weirs across both the Jumna and the Hindun, but after having examined the river at and below the point where the Hindun joins, and also at Alee, the Committee have no hesitation in recommending the latter as the best position for such works. The right bank of the river is remarkably firm and good, and the left bank is not particularly bad; but the strongest recommendative in favor of the site is the abundance of stone-both of rubble and of blocks, almost completely dressed, which can be obtained from the old fort, opposite to and within a distance of three miles of the river bank at Alee. Neither is the country on the left bank near the site unfavorable for the formation of a channel; the water could be brought into the Ganges canal below Baroli bridge, about the 115th mile from Hurdwar, or to the surface of the ground where irrigation might be commenced, without any great depth of cutting, and by a length of fifty-seven or fifty-eight miles of channel.

Flood discharge Jumna and Hindun.
70. The information available regarding the flood discharge of the Jumna is even less than in the case of the Ganges; indeed there is
none to be found that can be safely relied upon. The railway engineers provided waterway of 2,400 feet in length at Delhi ; but this has as much reference to the width of the river at that place as to the quantity of water to be passed through. An attempt has been made to calculate the discharge from a few isolated cross sections and the average fall, the results, however, have been so contradictory that they have been rejected as altogether untrustworthy, and it has been assumed that, as the heads of the rivers Ganges and Jumna are situated near each other, and almost similarly circumstanced as to sources of supply, the flood and dry weather discharges probably bear about the same proportion to each other in the two rivers. In the absence of better data, this mode of calculating has been adopted. The dry weather discharges of the Jumna is four-sevenths of that of the Ganges, the latter at Rajghat has been calculated at $\mathbf{3 0 0 , 0 0 0}$ cubic feet per second, four-sevenths of which or about $\mathbf{1 7 0 , 0 0 0}$, will be taken as the Jumna discharge. Regarding the Hindun, the railway engineers have porvided a length of 440 feet of waterway across it, and as there is no. better means of calculating the discharge it will be taken in the same proportion to that of the Jumna, as the waterway of the bridges bear to each other; this gives about 31,000 cubic feet per second as the flood discharge of the Hindun.
71. Several measurements of the low water discharge of both the Jumna and the Hindun have been made. The register of the height of the former river kept by the Executive Engineer of Canals residing at Delhi shows, that as a general rule the water is lowest in January ; hence it is in that month that discharges are generally taken. Information on this subject has been kindly furnished by the Resident Engineer of the railway, which is to the effect that the water was as low in April last as it was in December; but as his own registry shows that the river was higher in April, and as he accounts for the rise shown by his gauge by assuming merely, that the damming up of the river by the bridge works at Delhi, caused a rise of the water level equal to that shown by his gauge, the Committee think that his information cannot well be relied upon; they prefer trusting to the readings of a gauge near Delhi, regularly kept by canal officers, and removed from the influence of any changes of level caused by the bridge works.
72. The dry season discharges of the Jumna at Delhi, and of the Hindun near its junction with the Jumna, were measured, as shown

Dry weather discharge of Jumna and charge of below and were found to be-

Jumna at Delfi.

|  |  |  |  | Cubic feet per second. |
| :---: | :---: | :---: | :---: | :---: |
| 19th December, 1864. Lieut. Moncrieff, 18th January, 1865. Sergt. Caernarton, 19th December, 1865. Mr. Garbett, | - | -• | - | 935 |
|  | . | - | - | 1,613 |
|  | . | -. | . | 828 |
|  |  |  |  | 3$\lcm{38,376}$ |
| Average discharge, |  |  | - | 1,125 |

## Hindun River.


making a total average low water discharge of the two rivers of 1,554 cubic feet per second, or say 1,500 , which will be taken as the dry weather supply available for irrigation. It will be observed that this is above the observed minimum supply, but the very low discharge cannot be of long duration. . The measurement made in April last, when the river was considered to be low, gave Jumna 2,800, Hindun $\mathbf{5 0 0}=\mathbf{3 , 3 0 0}$. There are other measurements, which although not taken at Delhi, go to show that 1,500 cubic feet per second is not too low for an average minimum supply. Sir Proby Cautley, in page 43, of his Ganges Canal Report, Vol. I., makes the discharge at Agra 2,061 cubic feet per second; and Capt. Stewart made it 1,153, at the same place on the 19th January, 1865. Mr. Garbett made 5,004 at Delhi on the 19th January, 1866, but in the registry is noted "rain on the hills," which of course renders the result useless as a minimum guide.
73. The design of weir which is considered best adapted for the site is one in which the least quantity of brick-work and the greatest quantity of rough stone can be used, an abundance of the latter material being available at a very low rate. It is not considered sufficient to build the foundation of dry rough stone alone, the leakage through such a base, unless it were of very great width, would be considerable, if not even large, and the sand forming the bed of the river being very fine, would pass through with even a small leakage, the rough stone base would then sink, and the masonry floor, and probably the weir would be in great danger of breaking down. The form of section designed for Sookertal, has therefore been adopted, that is wells of moderate depth, protected by a large quantity of rough stone. The part of this report relating to Sookertal weir explains why such a section has been proposed there. It is proposed to raise the low water level 8 feet, 6 feet by a solid weir, and 2 by a moveable board. The length of the weir has been fixed at 2,500 feet, that being in about the same proportion to the length of the Rajghat weir, as the floods of the Jumna at Delhi bear to those of the Ganges at Rajghat. Such a weir would pass the calculated floods with an afflux of about one and a half feet, and with a velocity over the crest of about 11 feet per second, and without causing much injury to the low land above it.
74. As the river navigation past the weir should be provided for, and as the proposed canal would be important as a line of navigation, two locks would be necessary, one to pass the weir, which should be 24 feet wide, to allow the largest river boats to be locked, and one to pass
through the canal, which as it is probable that through the canal entrance would be the best way of passing the river, may require to be of the same size.
75. The right bank of the river is good, consisting of very sound clay and kunker, the amount of protection required to it would therefore be very small; but the left bank is of the usual sandy loamy nature, and would require nearly the same class of works, to protect it, as provided for the Rajghat project.
76. For crossing the Hindun and taking in its dry weather water, dun. Crossing the Hinworks similar to those used for passing the Rutmoo river in the Ganges canal would seem to answer very well, it is quite possible however that a more careful examination may show that great modifications would be found desirable ; the Rutmoo plan is only adopted now as the best available guide in forming the estimate. The width of waterway provided by the Railway Engineers for their bridge over the Hindun must be sufficient for the passage required on the irrigation channel ; they allow 440 feet. No lock has been provided for passing boats up, although it is not certain that one might not ultimately be required. Both banks of the river are easily eroded, and will require considerable protection.
77. As the cost of weirs across the Jumna and Hindun remains it may be well to the same, whatever quantity of water may be taken off by channels in reef crop. connexion with them, it may be more desirable to take an addition to the low water supply, when the river affords it, for the irrigation of a Khureef crop, than to trust to the dry weather supply alone; both views of the question will be briefly examined.
78. Probably the best rule to be guided by in fixing the quantity of water, above dry weather supply, which should be provided for in the new channel, is to make it as large as is consistent with keeping it navigable all the year round by the low water supply of 1,500 cubic feet per second. The lowest depth which is desirable for this purpose is about 6 feet, and the greatest depth in the channel may be limited to 10 feet, a maximum discharge of about 3,300 cubic feet per second will meet these conditions.
79. The highest velocity which the weakest of the soil through which the channels would be cut, could bear, is 2.5 feet per second, and a lower velocity than this would rapidly deposit the silt, carried by the rivers of this part of the country, $2 \cdot 5$ may therefore be taken as the velocity for the maximum discharge when the water would be the most heavily laden with silt. When the discharge falls to the dry weather supply, the velocity would be only about 2 feet per second; but then the water would be comparatively clear, and hence the decrease of velocity would be of little consequence.
80. The sectional area of channel required to discharge 3,300

Fixing the quanti-
of water to bo aken for Khurreef crops.

Velocity of water in new channel. cubic feet per second, with a velocity of 2.5 feet per second, is $\mathbf{1 , 3 2 0}$ square feet, and the fall required is about five and a half inches per mile. As in the Ganges projects, it is proposed to have the bed of
the channel 2 feet under the present low water level of the river or 10 feet under the crest of the regulating board of the weir, and following the most favorable line of country, the water in the proposed canal could thus be brought to the surface in a distance of about 58 miles from the river,-passing through the low land of the Jumna and Hindun valleys for 19 miles, then through higher land in a cutting not exceeding 26 feet deep, and coming to the surface near the village of Baroli.
Rates and cost of 81. The rates for the earthwork have been taken from Major project. Crofton's estimate for the alternative line of canal, and the estimate for the masonry works required has been based on the same information; of course great accuracy is not professed, nor could it be attained without a detailed survey. The cost of this project would then be-

| Weir and works on the Jumna and Hindun, | 17,00,000 |
| :---: | :---: |
| Channel, and all masonry works connected with it, | 34,00,000 |
| Total, | 51,00,000 |

Cost of project if for dry weather supply only.
82. If instead of providing a supply for Khureef watering, provision be made for carrying the dry weather supply of 1,500 cubic feet per second only, a channel large enough to carry it, having the same fall of $5 \frac{1}{2}$ inches per mile, and a depth of 10 feet, would give a velocity of 2.4 per second, which for the clear water of the dry weather would probably be enough; a sectional area of 50 feet bottom width, and side slopes of 2 to 1 , would carry rather more than the required quantity. The cost of bridging and crossing small rivers would also be greatly diminished, but the weir works would remain the same. The cost of this project would be

| Weir works, on the Jumna and Hindun, .. | .. | $\mathbf{1 7 , 0 0 , 0 0 0}$ |
| ---: | ---: | ---: |
| Channel, including all masonry works, \&c., | .. | $18,65,791$ |
| Total, | .. | $\mathbf{3 5 , 6 5 , 7 9 1}$ |

This project was not meant as a substitute for the sanctioned work of remodelling.
83. It does not seem to have been contemplated either by Sir A. Cotton or by Government, that any work on the Jumna or on the lower part of the Ganges should have been in any way substituted for the present line of the Ganges canal ; but it appears to the Committee that a few words may properly be introduced here on the subject before examining the Jumna work as an independent project.
Showing that it should not be substituted for the sanctioned work.
84. It is evident that aid from the Jumna, coutd only be substituted for the improvement of the Ganges canal, as sanctioned by Government, to the extent of the low water supply of that river. All the projection calculations, and all the calculations of the engineers who completed and worked the canal, are based on the supposition that it should have the full supply of 6,750 cubic feet per second during the whole dry season; an additional supply thrown in when the rivers would not be at their lowest, would not compensate in any way for the loss of water in January, when there is a large demand for the irrigation of the Rubbee crops, and when a continuous supply for them
is absolutely necessary. 1,500 feet per second is the full extent to which the Jumna at Debli could afford aid, and consequently the supply to be received through the upper part of the canal could only be diminished by that quantity. Now, there can be very little room for doubt that the difference between the cost of improving the canal to carry 6,750 feet, and that of improving it to carry 5,250 feet would be small; if the Engineers should be obliged to turn off the water to carry out one project, they would certainly be obliged to turn it off to carry out the other, the difference of cost between the two works could not exceed twenty per cent., and it is not at all likely that it would amount to even so much. Major Crofton's estimate for remodelling down to Nanoon, is about $35,00,000$, one-fifth of which, or $7,00,000$, would be the probable gain by taking 1,500 feet from the Jumna, and $\mathbf{5 , 2 5 0}$ feet, instead of $\mathbf{6 , 7 5 0}$, from Hurdwar. But the cost of taking the 1,500 feet from the Jumna has been calculated at $\mathbf{3 5 , 5 0 , 0 0 0}$ rupees, so that the question of cost alone seems to the Committee to afford the most ample grounds for adhering to the project for remodelling.
85. But there are other reasons also. Adopting the Jumna scheme would be trusting to that which was not certain for that which may be considered tested and certain,-the feasibility of building and maintaining a weir across the Jumna is yet untried, and although there is no reasonable doubt but that it could be done, there is no certainty as to the time required to complete it, nor indeed as to the cost of the work. The quantity of silt which would be brought in by a head so low down the river is also an unsettled question, although known to be a very important one. Then there would be the evil of delay,-the Jumna project could not be undertaken without a careful examination, a detailed survey, and references for sanction; so that it would most likely be a year, or perhaps more, before any works could be commenced, and three or four years before they could be finished. And, lastly, although there is no appearance of immediate failure in the works of the canal, it is not at all certain that they would remain in a safe state for even one year; indeed, the Committee have recorded their opinion in another part of this Report, that it is of great importance that the repairs should be taken in hand as soon as possible. They have no hesitation whatever therefore in coming to the conclusion, that a new head from the Jumna cannot be substituted-either as a temporary or permanent arrangement-for the sanctioned project for remodelling.
86. If the work be adopted as a means of irrigating additional land, of which there is plenty available to the south-west, the cost of making distributing channels and rajbuhas must be added to the estimate given above; but on the other hand, the water would be brought to the surface of the land to be irrigated by channels' at least 10 miles shorter than those required to bring it to the canal. Consider-

Project considereir as a work for supplying water to irrigate additional land.
ing it first as a project for taking $\mathbf{3 , 3 0 0}$ feet per second when so much can be had, the shortening of the channels would lessen the expense of the main line by about $4,00,000 \mathrm{Rs}$. Then in order to make it complete as a navigation channel, the main line should be connected with the Ganges canal, this would require 10 miles of still water navigation at, say 15,000 a mile $=1,50,000$. Making the total cost for weir and main channels $48,50,000$.

The cost of distribution channels and rajbuhas for the Cawnpore and Etawah branches of the Ganges canal, which are to carry 3,250 feet per second, is-


This estimate would give a fair idea of the cost of distributing the 3,300 feet from the Jumna, if the whole were to be used for a Rubbee supply; but as about half of it is for a Khureef supply, and as one foot per second used for Khureef, waters less than half the area which it would do if applied to Rubbee crops, it is evident that the distribution channels for a certain quantity of water used for Khureef watering, would be only about half the length of those required for the same quantity if used for Rubbee crops, and they would be only about two-thirds of the capacity. Now, as more than half the supply of the project under consideration is for Khureef watering, a deduction of about one-sixth may be made from the cost of the Cawnpore and Etawah branches, making the amount $\mathbf{3 8 , 5 0 , 0 0 0}-\mathbf{4 , 8 3 , 0 0 0 =}$ $33,67,000$; which, added to the cost of main channels, give a total of 82,17,000.
Probable returns
87. The probable return from this outlay would be,-for Rubbee and Khureef crops, 1,500 cubic feet per second, at 200 acres per foot per second $=3,00,000$ acres, at a profit of $1-8$ per acre (see note on Rajghat project) $=\mathbf{4 , 5 0 , 0 0 0}$. Again using the Cawnpore and Etawah branches of the Ganges canal as a guide, it is found that each cubic foot per second, waters under 60 acres of Khureef crop: but it' is probable that in this instance all the water is not utilized. The returns of the Eastern Jumna canals for 1863-64, give 75 acres per foot per second as the quantity watered by the Khureef supply, using this as data, 1,800 cubic feet per second would water say $1,35,000$ acres. The profit per acre would be about the same as that already calculated for Rubbee crops, giving a return of $2,00,000$, and making a total on the whole outlay of $\mathbf{6 , 5 0 , 0 0 0}$, or a little under 8 per cent., exclusive of returns from navigation, plantations, \&c.
88. If the project be confined to the use of the dry weather as proviang for a
dry weather supply
supply of
1,500
cubic feet per second only, the result would be,cost of bringing the water from the Jumna to the Ganges canal, as
estimated above, $35,50,000$; from which is to be deducted-cost of 10 miles of channel, by which the line would be shortened by delivering the water on the surface of the ground to be irrigated, $3,00,000$, less the cost of 10 miles of navigation channel required to connect the irrigation channel with the Ganges canal, $1,50,000=35,50,000-$ $(3,00,000-1,50,000)=34,00,000$. The cost of distributing $\mathbf{1 , 6 1 0}$ cubic feet per second on the Cawnpore branch of the Ganges canal is $16,00,000$, and the cost of rajbuhas for the same $5,50,000$, making a tatal of $21,50,000$. Taking the expenditure for distributing 1,500 to be in the same proportion, the cost would be $\mathbf{2 0 , 0 0 , 0 0 0}$ nearly, making the total cost $34,00,000+20,00,000=54,00,00 c$. Using the calculation given above as to the value of the water, that is 1,500 cubic feet per second $\times 200$ acres per cubic foot $\times 1 \frac{1}{2}$ Ŗs. per acre profit $=$ $4,50,000$ on an expenditure of $54,00,000$; giving a return of about $8 \frac{1}{2}$ per cent., exclusive of returns from navigation, \&cc. From this it appears, that the project for taking in the dry weather supply only would be the more profitable. But further, the quantity of silt, which would be taken in by the Khureef supply when the river would be disturbed, and consequently muddy, would be very large, and without doubt must involve heavy additional expenditure for cleaning.
89. On the above grounds, the Committee are of opinion that the project for taking, either 1,500 cubic feet per second, or 3,300 cubic feet per second from the Jumna, is well worthy of a detailed mittee examination, as an independent scheme for the irrigation of land in the lower part of the Doab.
90. They would, however, draw attention to the separate paper recorded by Mr. Sibley (Appendix A.), dissenting from the views of the majority, and advocating the formation of a canal from the Jumna at Toghlukabad, in substitution for Major Crofton's project.

## THE PRESENT CONDITION OF THE GANGES CANAL, WITH REMARKS ON ITS FAULTS AND UPON THE REMODELLING PROJECT.

91. So much has been said of the serious faults and inefficiency Present condition of this great work, that it has been a matter of some surprise to the of canal. Committee to find that it has been carrying nearly two-thirds of its full supply during the past twenty months; and that the navigation, FallsupplyatRoor-
 same period, during which the canal has not been closed for even a ${ }^{\text {bic feet per second. }}$ single day.
92. The canal has now been in operation for eleven years, and Increase of irrigathough no outlay has been incurred for additional falls to lessen $\begin{gathered}\text { tinn, burent too veoceity of }\end{gathered}$
the slope and velocity of current which have been found to be mucin too great for the soil to bear, the area of irrigation has steadily increased.
Cannot be used to its fullest capabilities till fall works are in-
creased.
93. Looked at as an irrigation work, for which it was princiof the country is not sufficiently overcome by artificial masonry falls; and, consequently, as already mentioned, the velocity of current is too great. The canal cannot therefore be used to its fullest capabilities till the fall works are increased.
Navigation impeded by velocity of current.
94. Viewed with regard to navigation the velocity of current is a great obstacle to traffic. The want of sufficient headway under bridges on the Cawnpore Branch is also a serious disadvantage, as in carrying light and bulky cargo, boats are loaded to a great height above their decks. The insufficiency of headway is the more unfortunate, as it would not have added much to the original cost of the works to have given 10 feet, as in all the large bridges on the main line.

Imperfection of arrangements in regulating passage of wate at falls.
95. The excessive velocity of current appears to have been further increased by an imperfection in the arrangement for regulating the flow of water over the falls. They were originally designed of of greater width than the earthen channel, and in several separate chambers or compartments, the entrances to which were fitted with sleepers to admit of their being closed for repairs. But when the canal was first brought into operation, and all the works were new and in good repair, the stream was permitted to pass through all the compartments, and its velocity as it approached the falls was thus accelerated. No evil would have resulted from this if the soil had been firm, but it was too soft to bear the rapid current.

[^6]96. The canal authorities do not appear to have been fully aware till too late that the current was so much too rapid, and that it was steadily working upon the bed, and causing a retrogression of levels below each set of falls. What principally engrossed attention at first was the construction of minor distributing channels, and especially the arrangements for giving the water to the cultivators by measurement, the importance of which was certainly not over estimated. The canal, moreover, had only been in operation two years, when the district which it traversed became the scene of the mutinies, and Europeans of all ranks were compelled to leave their ordinary duties. The mutinies, and the unsettled state of affairs which immediately followed, were hardly over when this same part of the country was striken with famine, and the whole attention of the canal department was given to distributing the water to tide over the calamity; minor channels were pushed on in every direction with the utmost possible rapidity, and as much water as could be distributed was thrown into the canal.
97. Thus great delay occurred in reducing the waterway at the
falls; and when at length the urgent necessity of doing so became apparent, further delay occurred in carrying the necessary arrangement into effect, from the sleepers, which are 25 feet in length, giving way. It was not till 1862 that the waterway was effectually reduced, and

The delay increased by that time so much retrogression of level had taken place that it plying a remedy. became impossible to apply means, which at an earlier stage would have been feasible. Whether at first it would have been quite safe to have completely closed several of the fall compartments, and have passed the water through the remainder with an increased depth, cannot now be decided; but there is no doubt that by delay the cushion of water on the floorings had become so much reduced that such a plan could not be attempted, and the expedient was adopted of partially closing all the compartments and distributing the water equally over them. This arrangement however in obviating one difficulty led to another. The water instead of gliding over the ogee descent in the manner intended, fell suddenly upon it from an increased height, and exposed the masonry to an action it was never designed to bear.
98. The parts of the works which suffered most from the retrogression of levels were the floorings under the falls. The water now fromererere action of fell from an increased height upon the masonry itself instead of into a cushion of water, and the action thus intensified tore up the brickwork wherever it happened not to be of the very best quality, as was the case at the Mahmudpoor, and some other falls below Roorkee. Above Roorkee, as well as in other parts of the canal, the brickwork, though requiring occasional repair, has not suffered seriously.
99. Another evil which the retrogression of level caused was the greatly increased velocity with which the water escaped from the floorings into the earthen section of the canal below. Instead of flowing off with a moderate velocity, it made a rapid shoot and scooped deep holes in the soft soil where the protective boulder crib work terminated, an effect which was increased by the direction given to the wing walls at some of the falls. Thus the whole of the masonry work was endangered. An accident was only prevented by the crib work with which the foundations had been protected, and which accommodated itself to the change in the level of the canal bed without permitting the foundations to be completely exposed.
100. Various expedients were adopted by the canal officers to mitigate the evils caused by the retrogresion of levels. Small rough tro remedy evil of resupplementary weirs were thrown across the floorings to hold up the water and thus obtain a "cushion" under the falls, and other rough weirs were carried across the canal at a distance of about 100 yards below the tails of the floorings, to head up the water and diminish the plunge over the crib work into the soil excavation. None of this work was of a permanent character, but it has enabled the canal officers to keep the canal steadily rumning with two-thirds of its full supply.

Injary to mod iridzest prevented by protecting forniatirnts wita boulders.
101. At the road bridges also the retrogression had an injurious effect, but this was remedied partly by the crib work with which their floorings had been originally protected, and partly by adding loose boulders.

Exredients adopter prevented further serious damage, but retrogression not al together arrented.
102. Since these expedients were adopted there has been no further serious damage, and the deep holes which had been formed near the masonry works have to some extent silted up; but that retrogression is still going on, was evidenced by the state of the water at different points along the canal at the time the Committee examined it. This view is concurred in by Colonel Dyas, who in reply to a question from the Committee, writes as follows :-
"As to the bed (of the canal) erosion is no doubt going on in many places, inasmuch as during the cold weather, when the water coming in from the Ganges river through the Myapoor regulator (the head of the canal) is almost quite clear, the water in the canal gradually becomes very muddy. But a comparison of cross sections of the canal bed, which 1 have lately had taken at every mile along the canal, with similar sections taken by Major Crofton in 1864, shows that no dangerous action is going on at present."
Falls still expoced to most violent action.
103. The falls, moreover, notwithstanding all the expedients that have been applied, are still exposed to a most violent action, which the brickwork was not designed to bear, and it is impossible to predicate how soon some accident may necessitate the sudden closing of the canal and the destruction of the crops dependent upon it. The Committee were anxious to have had the canal laid dry to enable them to closely examine the masonry works, but such strong objections were urged against it by the canal officers, on account of the time it would occupy and the injury to crops which required constant watering, but more especially on account of the injury which might be caused to the floorings of the falls when the depth was reduced, both when shutting off the water and re-admitting it again, that the point was not pressed. With respect to the condition of these works, Colonel Dyas replied to the Committee as follows :-
"As to the falls in the Northern division (from Myapoor to Jaoli) the extent of damage done (during the past twenty months), as far as can be ascertained by careful sounding and probing, consists of a slight displacement of crib work battens. In the Meerut division two falls have been slightly injured, but not sufficiently so to warrant a closure of the canal for the purpose of repairing them. The Chitoura fall ( 55 miles 4976 feet) has had part of the ogee in No. 3 bay ripped out, and that bay is kept closed in consequence, to prevent further damage; and the Sulawur fall ( 67 miles 2350 feet), has had two of the hammer-dressed Delhi stones in flooring of No. 3 bay, lifted out and turned over. This occurred in September 1864, and the stones have not moved since. The bridges are all secure." Colonel Dyas' report is on the whole a favorable one; but it is evident
to the Committee, as it is to all the canal officers, that the falls are still in danger, and that no time should be lost in taking steps to render them secure.
104. The navigation on the Ganges canal has always been Navigation impernecessarily imperfect from the great velocity of current which in the ${ }^{\text {fect. }}$ main canal was intended to be from $2 \frac{1}{2}$ to $2 \frac{3}{4}$ miles per hour; and in addition to this obstacle the retrogression of levels, and the consequent necessity for limiting the depth of water admitted into the canal has acted most injuriously. The lock channels, instead of having a depth of 9 or 10 feet of water in them, and a width at water line of 43 to 46 feet, have had barely enough water below the locks to float boats of light draught, and a width at water line of only about 28 feet, the width at bottom being 16 feet. This very limited capacity of channel has of course delayed the introduction of large boats, and has led to frequent interruption when silt accumulated. The shortness of water supply in the canal morever necessitated the alternate closing of the Cawnpore and Etawah branches to distribute the water for irrigation, and thus the navigation on the former which connects the main canal with the river Ganges has occasionally been completely closed. Under all these circumstances it is not surprising that the navigation has not increased satisfactorily.
105. The Committee may remark here that the present lock channels would probably work efficiently if the eroding action of the stream in the main canal were less, and if the valves at the locks were larger to admit of more effectual scouring. But where new locks have to be constructed, they think that attaching them to the falls, as in Major Crofton's project, is a great improvement.

## REMARKS ON THE REMODELLING PROJECT.

106. In correcting the great fault of the canal, or "remodelling" it as it has been termed, an expression which however is apt to lead nomy come of the to a somewhat erroneous impression of the measures required, the ${ }^{\text {expensive. }}$ Committee remark that while on a work like this, of almost a national character, they would not expect to see such rigid restriction of expenditure to works of absolute necessity as is generally practised in the construction of ordinary irrigation canals, yet at the same time, considering how important it is both for the reputation of the Ganges canal itself, as well as for the interests of other large irrigation projects in other parts of India (whose execution is supposed to be delayed on account of uncertainty as to their financial success), that economy as well as efficiency should be studied, they cannot but regard some of the changes provided in the remodelling project as unnecessarily expensive, and they would therefore suggest that the following points be carefully reconsidered:-

The present shape of the cross section need not be altered, and requisite capacity to be obtained by raising crests of falls.
107. The Excavation.-It appears that the effect of the retrogression of levels has been to deepen the canal in the reaches between each set of falls, and in the remodelling it is proposed to alter the shape of the section by widening it and filling in the bed where material is furnished from the widening, and trusting to the deep spots seft up where material for filling is not available. Now, however desirable it would be in designing a new work to adopt a shallower section, it can hardly be necessary to alter an existing work to give it that precise form. The cost of the excavation from the head of the canal to Jaoli falls is estimated at upwards of four lakhs of rupees, and it is apparent to the Committee that by slightly raising each set of falls, so as to reduce the surface slope by about one inch per mile, the present excavation will generally answer, and a considerable outlay will be saved.
Grating at Kunknl 108. Grating and lock at Kunkul bridge.-These are estimated bridye objectionable,
and lock unneessary. over Rs. 30,000, and may well be omitted. The sunken grating in almost a perpendicular position seems to be actually objectionable, and the lock with its attendant working expenses, can hardly be required to meet the contingency of a boat passing up the stream at a time when there might be only a small supply of water in the canal. It will always be necessary to maintain a large volume in the canal, and supposing a boat to arrive at Kunkul, when there might accidentally happen to be a small supply, the acceleration of the current at the bridge could be checked by partially closing the compartments of the falls three miles below, especially if instead of giving the bed of the canal a sudden drop below the bridge, the change from the small to the large section be made gradual.
New fall and lock 109. New fall and lock at Roorkee.-It appears to the Commitat
disponsed
jithe may
me-
be tee that by pitching the bed of the Solani aqueduct (earthen portion) jection to increasing depth of water on So- and securing the floorings of the bridges at Rutmoo, Peeran Kullier lani aqueduct.
Protection of canal bed. and Mahewar, the new fall and lock may be dispensed with, and'some reduction in the expense effected. But they also object to the proposed falls on other grounds. The Solani aqueduct was designed to carry a stream 10 feet in depth and if the falls are constructed this will be increased to nearly 13 feet. The Committee do not doubt that the aqueduct can be made to carry the increased depth, but sufficient provision is not made in the estimate for this, and as the expense will be heavy to render the work perfectly secure from accident, and there is already percolation through the brickwork of the masonry aqueduct which must in time do harm, they would prefer retaining the designed depth of water in place of increasing it to nearly 13 feet.
110. With respect to the reduction of the velocity of current
 Asuffnugger falls. to raise those falls slightly; but this will not be attended with any difficulty, and it will have the effect of saving a large quantity of excavation in the manner indicated in a previous paragraph.
111. Additional arch to increase the vaterway under bridges.- $\begin{gathered}\text { Additional arch to } \\ \text { bridges not abosolutely }\end{gathered}$ This does not appear to be absolutely necessary. All the bridges have ${ }^{\text {necesasary }}$ already been subjected to a very severe test from the retrogression of levels, and the boulder protection to the floorings has been increased. When the floorings are lowered to suit the altered bed level of the canal, the waterway will be increased, and the velocity of current will not, the Committee consider, be too great. If the wide section proposed in the remodelling be adopted, the omission of the additional arch will only increase the velocity from $1 \frac{4}{8}$ to $2 \frac{1}{4}$ miles per hour, and this only for a few yards. If the deeper section recommended in para. 107 of this Report, be adopted, the increase of velocity will be even less.
112. Before leaving the subject of the works provided in the remodelling estimate, the Committee would make some observations on the question of closing the canal for a long period to admit ings ander falla sugof the works being executed. In Major Crofton's report the estimate of loss from closure amounts to Rs. 13,48,213; but owing to the extension of the irrigation since that estimate was made, and to the increase in the water-rate levied, it appears that the loss from a long closure now would probably amount to upwards of twenty lakhs of rupees.

The Committee are sanguine, however, that the works can be executed without a long closure, especially if the details of the floorings of the new falls are modified, With the view of obtaining a deeper cushion of water over the floorings, Major Crofton has placed them at about 4 feet below the bed of the canal, and at this depth a covering of brick-on-edge is considered by him sufficient to resist the action of the water. The advantage of the deep cushion is probably not over-rated, but placing the flooring at so low a level will cause great delay for unwatering or forming cofferdams, and as the loss from long closure will be very heavy, the Committee would recommend that the deep cistern be omitted, and that the flooring be placed at the level of the canal bed, and covered with Bhurtpore dressed stone, 15 inches in thickness; and they would remark that, though the stone is very expensive, there is this great advantage in the arrangement that, whenever repairs may be required to a flooring placed at the level of the bed of the canal, they can be readily executed, because directly the canal's supply is shut off the floorings would be exposed to view. With the aid of the gratings which the Committee understand Colonel Dyas has contrived with complete success on the Baree Doab canal, for breaking the water as it begins to fall, the Committee consider that the modification they recommend will be found to answer well.
113. Supposing that the works can be executed during two $\begin{gathered}\text { Comparatively }\end{gathered}$ short closures of about $3 \frac{1}{8}$ months each, the loss of revenue would be cloanrea comparatively trifling. From information furnished by Colonel Dyas,
the Committee have approximately* estimated it at Rs. $\mathbf{3 5 0 , 0 0 0}$ for each closure, on the supposition that the crops which must be stopped are not replaced by others which can be watered at some other period of the year when there is an abundant supply in the canal. If other crops be substituted, the loss during each closure would only amount to about Rs. 150,000. $\dagger$
114. The Committee believe that two such closures will be found sufficient to complete all the work up to water line, and they would point to the cases of the upper and lower Coleroon anicuts, and the Gunnarum aqueduct in Southern India, $\ddagger$ each of which was commenced and completed in one dry season, as instances of what may be done in speedily executing hydraulic works.
Future improvements.
115. The Committee would now draw attention to some points affecting both the navigation and irrigation which seem to call for condideration as measures of future improvement.
Large proportion of artificial to natural
116. The large proportion of "artificial"§ to "natural flow" \| flow irrigation on the wah branches. irrigation on the Cawnpore and Etawah lranches.-There is a considerable loss of revenue on this account on the two branches, but whether this is caused by the smallness of the supply of water which keeps the surface at too low a level; by the levels of the country being unfavorable for the cross distributing channels; by a faulty alignment of those channels; or from the excavation of the canals themselves being unnecessarily deep; the Committee have not ascertained. A great part of the Cawnpore branch appears to be in light excavation. The Etawah branch $\mathbb{T}$ is generally deeper. The cross channels or rajbuhas are said to be badly laid out, and to work very imperfectly. However this may be the subject demands careful attention, and if it should be found necessary to introduce weirs at intervals to bring the water up to a higher level at certain spots, it would be desirable to make allowance for such alterations in level where the headway under the bridges is increased for navigation, otherwise it may be necessary to alter the bridges a second time. The introduction of weirs would be advantageous as affording the means of more efficiently controlling and distributing the water on

[^7]these long branch lines, and would also benefit the navigation by reducing the velocity of current. These are additional reasons for an investigation of the question by the canal officers. It may be found possible to improve the irrigation by extending the distributing channels till they command more low land.
117. Want of suffic.ent waterway at terminus of Cawnpore Cawnpore branch branch.-This branch is too narrow near its terminus to allow of the to be enlarged at its
largest class of boats passing each other, but this may easily be corrected by enlarging the canal from 20 to 27 feet bottom width from the 159 th mile, and making it still water navigation from the 164 th mile to Cawnpore, a distance of $5 \frac{1}{2}$ miles; arrangements will of course be necessary at the commencement of the still water canal to lead off for irrigation the volume of water which is brought down to that spot by the flowing canal. The silting and growth of weeds which are likely to occur in the still water must be accepted as unavoidable, but as the length is short there will be no serious difficulty in maintaining the canal in good order.
118. The Etawah branch.-The headway under bridges on this branch is insufficient and should be increased, the tail should be locked down to the Jumna with still water navigation, arranged as re- mended for Cawnpore commended for the Cawnpore branch.

119. Cruss canals for navigation.-At present boats have to make a long circuit to get across from one canal to another, this for navigation to conmake a long circuit to get across from one canal to another, this $\begin{gathered}\text { fect main canals with }\end{gathered}$ should be provided for by constructing short cross lines; similar lines ${ }_{i}^{\text {each other, and }}{ }_{\text {important towns. }}^{\substack{\text { in }}}$ should also be formed to connect important places like Meerut, Bulundshuhr, Coel, \&cc., with the main lines, as the requirements of trade may indicate.
120. The width of locks.-This question seems to demand some remark from the Committee, as it has been proposed to go to some feet considered suffexpense to obtain 20 feet in width. The existing locks are 16 wide, and after careful discussion the conclusion arrived at is, that this dimension is sufficient. The depth of water generally in the, canal is so great that there could be no objection to the use of screw propellers if steam navigation should appear profitable : the stern wheel also could be used. Moreover, locks of 20 feet width would still be too small for side paddle steamers except of a very small class.
121. Niew branch canal from Roorkee to Deobund.-The construction of this work has been under consideration for some years, irrigation from Roor struction of this work has been under consian fee to Deoband rebut the shortness of supply in the main canal has of course hitherto ${ }^{\text {commended }}$ kept it in abeyance. In Colonel 'Turnbull's Canal Revenue Report of $1861-62$, Mr. Login's opinion on the result of the survey is mentioned as most favorable. The Committee have also received the opinion of Mr. Williams, the Commissioner of Meerut, who strongly advocates the carrying out of the project for protecting the district through which it will pass, against famine, and for more equally distributing the benefit of canal irrigation throughout the country,
the Deobund district being most unfortunately circumstanced as regards water, which is only obtainable at present from very deep wells. The line which this branch will follow corresponds with that recommended by Sir Proby Cautley as the alternative line, and the Committee think that the time has arrived for turning attention to it.

Short periodical closure of canal roclosure of canal ro-
commended for ex-
amination and repair amination and repair of worke.
122. Periodical closure of canal.-In so large a work dependent on many masonry structures of a difficult nature, a short annual closure would be desirable to admit of everything being closely examined, and repaired if necessary. The canal officers will of course be able to decide upon the best period for such closures, as they know during which portion of the season the demand for water is least. If the plan of making a periodical closure is carried out, the cultivators will probably soon learn to store a small quantity of water near their fields to continue the irrigation till the canal is re-opened.

## PROPOSED WEIR ACROSS THE GANGES AT HURDWAR.

Importance of a permanent weir at Harmanent weir at Hur-
dwar generally redwar gen
123. The importance of gaining a control over the Ganges at Hurdwar by means of a masonry weir across the channel has been generally recognized. There is no difficulty however in throwing in the supply at present required by the canal, namely 5,000 cubic feet per second, and the expense has not increased to the extent that was anticipated by the late Colonel Turnbull. During the last four years the outlay incurred on the shingle bunds, or temporary dams, for diverting the supply from the river to the canal has amounted to 60,702, or 15,175 rupees per annum.
Not however necessary for the present supply of the canal.
124. If 5,000 cubic feet per second were to be the full supply of the canal, the Committee would not advocate the construction of a permanent weir across the Ganges. The expense of keeping such a work in repair would not certainly be less than 5,000 rupees per annum; 10,175 rupees would therefore be saved in maintenance, and allowing even that the expense of raising the bunds should increase so as to make the saving 15,000 rupees per annum, this would represent a capital of Rs. $\mathbf{3 0 0 , 0 0 0}$, and a permanent weir across the Ganges could not be built for that amount.
Temporary bunds rarely necessary dur-
125. It is true that although there is no difficulty in throwing ing monsoon months. 5,000 cubic feet per second into the canal during the dry season, there is the possibility of the river falling so low during some of the rainy months, as to render it necessary to have recourse to bunds even at that season; which was the case during the famine year of 1860-61, and the inefficiency of the temporary arrangements during the monsoon months, has afforded the strongest argument that has hitherte been brought forward for the construction of permanent works.
126. But the Executive Officers have now a much more ex-

Experience of Exe entive Officers equal tended experience of the management of the river, than they had up to scch a contingency. to the famine year; and the Committee consider that with the good management which may be expected, there ought to be no difficulty in feeding the canal during the monsoon. They believe that the failure during the famine year has been exaggerated. For a great part of that year, the whole supply of the river was turned into the canal, and the revenue returns were more than double those of the previous year; but the fact of $\mathbf{7 3}$ acres only having been irrigated per cubic foot of water admitted into the canal, while at present upwards of 140 are irrigated by the same quantity, indicates that the great want of the famine year, so far as the canal was concerned, was the means of distributing the water. The rajbuhas were then in their infancy, and without a proper proportion of them, it would have been useless to increase the supply from the river.
127. When the full supply of 7,000 cubic feet per second has to be thrown into the canal, the construction of a weir becomes more in canal of considerable necessary than with a supply of 5,000 cubic feet. Sir Proby Cautley's age through tempo estimate of the minimum discharge of the Ganges is over-rated; at sonry weir necessary times it falls considerably below $\mathbf{7 , 0 0 0}$ cubic feet per second. With shingle bunds there must always be considerable leakage, and at such times, the loss thereby occasioned might seriously affect the efficiency of the canal. The Committee look upon this as the most important argument in favor of a masonry weir, but they may add that it is very desirable that both Europeans and Natives should be saved from the exposure and sickness which they have to undergo in the construction of the temporary bunds during the most unhealthy time of the year.
128. Several projects for a weir have already been before Government, but none of them have met with acceptance. Colonel Dyas was about to enter upon the subject, and was having detailed surveys prepared, when the Committee was ordered to assemble; but not knowing that the question of a weir at Hurdwar would occupy their attention, he removed the officer who was surveying there to employ him on a survey of the Khadir land between Sookertal and Gurmuktesur; some further measurements are therefore required before Colonel Dyas can submit a plan and estimate for a weir to Government. The Committee are not prepared to furnish a detailed plan, which would require a more comprehensive knowledge of the locality than they have had the means of acquiring.


#### Abstract

Observations on matters of secondary importance. 129. Having in the foregoing Reports stated their views and opinions on the principal points connected with the subject referred to them, the Committee proceed to offer some observations on matters, which, though of secondary importance, will not, they trust, be considered irrelevant to the complete elucidation of the state and prospects of the Ganges canal.


## STONE FROM THE SEWALIK HILLS AND HIMALAYAS FOR THE CANAL WORKS.

130. With respect to the use of stone from the Sewalik hills ferent places in the ferent places in the
Sewalik hills, and Sewalik hills, and
the search for stone the search for stone therto unsatisfactory. obtained in small quantities scattered over the hills, yet there is apparently no single spot where quarries can be opened with the prospect of an abundant supply being met with, and that as regards the main Himalayas, they find that the search for stone has hitherto proved equally unsatisfactory in its result.
Further and more 131. The Committee are however convinced that the canal offiextended search rer cers have used their best endeavors to obtain stone of suitable
commended. quality for the works, and beg to refer to the following replies from the Chief Engineer of Irrigation to queries put by them on the subject. They however think that the search on the main Hinalayas should be extended when the weather will admit of its being done more carefully than it could be at this season; and also that a quarry on a more extended scale than any which the Committee have seen should be opened at Hurdwar.
Reply to Commit-
131. Replies to questions in Colonel Lawford's letter of yestee's enquiry. terday.

Question 1.-" What was the result of the search made for stane at Hurdwar and in the Sewaliks in the neighbourhood, stating cost of
"Quarrying,
" Dressing,
"Leading to canals,
" Making roads,
"Superintendence."
Reply.-The report made in para. 13 of my letter 4,293, dated 23rd November, 1864, which was printed with Captain Crofton's Report on the Ganges canal is correct, viz.: "The quantity of serviceable stone to be had at any one place is very small, and there are not very
many places within easy reach of the works or of water carriage where it can be had.
"The places where fair stone was found were soon worked out, and operations had to be closed on the 31st August, 1865, when 51,496 cubic feet of stone had been landed, at a cost of 51,455 rupees, say one cubic foot per rupee.
" The following is a tolerably accurate approximation to the details of cost :-

"The stone is of very unequal quality and very uncertain; thus, part of the same block may be good hard stone and the rest so friable that it can be rubbed into sand with the fingers. The particles of sand are held together merely by infiltration of lime which partially dissolves in water, so that, after 60 hours' immersion the stone will bear only half the crushing weight which it will bear when dry.
"Dividing the stone landed into three classes-fair, middling and ${ }^{\text {" }}$ bad-I find that the crushing weight of each in tbs. per square inch, was as follows:-

|  | Class. |  |  |  |  | Dry. | Wet. |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Fair, | .. | .. | .. | .. | .. | 5,311 | 2,764 |
| Middling, | . | .. | .. | .. | .. | $\mathbf{3 , 3 5 6}$ | 1,879 |
| Bad, | .. | ... | .. | .. | .. | $\mathbf{3 , 0 0 3}$ | 1,447 |

"'The following is the result of similar experiments on our bricks.

|  | Class. |  |  | Dry. |  | Wet. |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  |  |  |  |  |  |

"The crushing weight for jamah, wet or dry, is 6,180 , and for Dehli quartzite, 16,480."

I have already shown the details of these experiments, and the crushed specimens themselves to (I think) all the Members of the Committee.

Question 2.-" Has any search been made for stone in the main Himalayas in the neighbourbood of Luchmun Jhoola, or elsewhere, and if so the result ?"

Reply.-" There are now no European officers here who can give information concerning matters of ancient date, nor have I been able to find in the records of this office any report of explorations made beyond the Sewaliks. But I have ascertained from the old mistrees employed on the works that the country up to and beyond Luchmun Jhoola was often in vain explored for stone. In 1862, Professor H. B. Medlicott was employed in exploring, with Mr. Login, but I can find no other report of his than may be gathered from the printed memoirs of the Geological Survey of India, Vol. III., Part 2.
"Lately, at my request, Mr. Kelly has examined the valley of the Ganges and the streams which run into it on both sides as far as the stream above Tupobun, but he has not found anything worth quarrying. He reports that the sandstone about Rikheekes is much the same as the sandstone of the Sewaliks, that the black shale which underlies the limestone is in thin layers, that the limestone itself flies to pieces under the hammer, and that the slate is only to be had in small thin pieces.
" On Mr. Kelly's return with this report I requested him to offer a liberal reward to any one who would point out a place where good hard stone was to be had in silu, and he has men employed in exploring. He is now in all probability before the Committee, as I requested him to call on them and to communicate all his information personally.
"I have now the honor to inform you, that Mr. Kelly has made a further examination of the ground, this time on the left bank of the Ganges, up the Lal Nuddee, opposite Ghora Ghât; his men having brought in a good specimen of limestone from that locality.
"I regret to say that Mr. Kelly has ascertained that the sample stone was a boulder merely carried down possibly from some other locality. He describes the rock in situ as a 'very hard tough stone, well suited for coarse rubble masonry, but not for dressing, as the beds are very irregular and the joints run in every direction.' He adds, that 'possibly an odd stone might turn out in quarrying down a cliff,' and that 'there are lots of shapeless masses of stone in the ravine hard enough for anything, but all intersected with false joints, so that they cannot be split up straight, and being in scattered blocks, the carriage would be very expensive.' He does not think that a stone of 4 cubic feet when dressed could be found there; and his men, whom he has out on search, with the incentive of Rs. 100 reward before them, all say that at and above Rikheekes nothing capable of being dressed has been met with.
" I may add, that although I considered Mr. Kelly's report
to be conclusive as regards the localities examined by him; and although the prospect of finding good stone within a moderate distance of Hurdwar appears to be remote; yet, the search for it shall not be abandoned."

Question 3.-" The Committee are anxious to have a copy of Professor Medlicott's Report, if it can be found."

Reply.-" Memoirs of the Geological Survey of India, Vol. III., Part 2, sent by to-day's post."

Professor Medlicott states (page 176) "that stone fitted for ornamental or monumental purposes might be found among the thick bedded hard limestone of the "Krol group." Of this group the belt of limestone and slate which crosses the Ganges between Rikheekes and the stream beyond Tupobun, would appear to be the continuation; it seems strange though, and it is most disappointing that no vein of good hard stone should be found in such a locality. But as Mr. Kelly has remarked, if there were any veins of hard stone they would probably have contributed good hard boulders of fair size to the Ganges deposits, whereas it is notorious that the limestone boulders are small, mere shingle."
P.S.-The best stone to be had in the neighbourhood of Hurdwar is the conglomerate, of which the caps of the Myapoor dam spiers are made. It is hard and durable under running water. That one was not experimented on because from the nature of it, experiment on a small specimen would be useless. Unfortunately, the quantity of this stone, which is to be had within a moderate distance from Hurdwar, is very small. The visible supply has been almost worked out.

(Signed) J. H. Dyas, Lieut.-Colonel, Chief Engineer, Irrigation Works, N. W. Provinces.

Roorkee, 31st May, 1866. $\}$

## EXTENSION OF THE CANAL FROM CAWNPORE TO ALLAHABAD.

133. In the concluding paragraph of his first letter to the Government of India, Sir. Arthur Cotton has strongly urged the necessity of carrying the canal as a line of navigation to Allahabad, as the

Sir Arthur Cotton's plan for extending the canal navigation to Allahabad. point "which is essential to its effective operation;" and the Committee have accordingly given the subject their best consideration, especially as it appears to have been Sir Proby Cautley's original intention to adopt the same terminal point.
134. The length of that part of the Doab is 120 miles, and its Area of the lower average breadth under 25 miles; it is bounded on each side by a $a$ itsmeansof communi-
cation very favorable navigable river, and intersected throughout by the East India railappears to render the
extension not neces- way, and the Grand Trunk road, and it appears to the Committee sary at present. that no part of India is better provided in proportion to its area of 3,000 square miles with the means of transport, either for its own produce in favorable seasons, or for that of other districts for its relief in times of scarcity, and they therefore fail to perceive the necessity of an expenditure of at least 15 lakhs, in carrying a navigable canal through this narrow tract.
The irrigation of the extremity of the Doab not required so in the apper districts. less frequent there, than further northward, but without accepting

Doab, the Committee have been informed that the failure of rain is this as an ascertained fact, they may remark that it will be equally advantageous and more economical, to extend irrigation in the upper districts, as the great length of the canal has unquestionably been one material cause of its heavy cost, and disproportionate financial returns.

The extension to Allahabad not recommended.
136. While, therefore, there are other objects of so much more urgent importance in connection with the canal, demanding attention, the Committee cannot do otherwise than advise that the Allahabad extension should for the present remain in abeyance.

## FINANCIAL STATE OF CANAL.

Nett receipts from canal do not cover interest on capital.
137. The annual statement of remunerative works in the $\mathrm{N} . \mathrm{W}$. Provinces, exhibits the Ganges canal in an apparently hopeless state of indebtedness. The total expenditure on new work is charged with interest at the rate of 5 per cent., and the expenditure up to the end of 1863-64, having amounted to upwards of 2 millions, the charge for interest is over 10 lakhs of rupees. The total income for that year was Rs. $7,73,390$, and the expenditure in maintainance and for establishments was Rs. $6,09,711$, leaving only Rs. $1,63,679$ to meet the charge of 10 lakhs. About Rs. 8,40,000 thus appear at the end of the year as a balance of charges against the canal. This operation which has been going on from the commencement has caused the accumulated charges up to the end of the year 1863-64, to amount to upwards of 83 lakhs of rupees.
Remunerative statement not a fair criterion of the merits of the canal.
138. If the above procedure be accepted as correct, it is evident that the canal must be pronounced a failure as a commercial speculation, until the nett receipts shall exceed the annual charge for interest. The Committee do not however consider that the annual statements of remunerative works, from which the foregoing particulars are taken, are at all a fair criterion of the merits of the canal.

Policy of Government in levying low water-rates.
139. The statement to the following effect of Colonel Colvin, which Major Crofton has quoted in his Report, well deserves attention; " that the object of Government was not so much to form a productive source of revenue from the actual price paid for the water, as to gain
a sufficient control over its expenditure and to prevent its being wasted, and that they looked to the general improvement of the country as the source from which they should derive a return adequate to the outlay." The Committee believe that until very recently the Home Government continued to hold the same views, and it is to this policy the extraordinarily low water-rate which has been charged on the Ganges canal up to the end of $1864-65$ is to be attributed.
140. The Committee are not aware to what extent per acre the land revenue of the country watered by the Ganges canal has been enhanced by irrigation; but they are of opinion that whatever it may be, it should appear as an item of canal revenue, and if it cannot be separated from the ordinary land revenue, and incorporated with the proceeds from water-rates, it should at least be exhibited in the annual returns to the credit of the canal. On this point the Committee entirely agree with Colonel Dyas, who has expressed his views in the following terms in his report for 1863-64. 141. "Nothing appears clearer to me than that all proceeds

Increase of land revenue due to this canal should be exhibited in remunertive statement. resulting from the construction of a work of irrigation should be quoted. clearly shown, whether they happen to be on the debtor or on the creditor side, whether profit or loss, for if they are not shown, how is it possible for Government to know whether it is advisable to construct any new work. It matters little under what names or heads the proceeds are shown, whether as land revenue, enhancement of land revenue, diminution of land revenue (a negative proceed) or water-rate; the one thing needful is to show them all clearly and not to allow any of them to be muddled up with the land revenue proper, that is, with the land revenue assessable, had the canal no existence."
142. As an example of the effect of a canal in the N. W. Provinces in improving the land revenue, we quote as follows from a

Increase of land revenue due to irrigation on Eastern Jumna canal. report also by Colonel Dyas, with reference to the Meerut district, watered by the Eastern Jumna canal.
"Of that district at the time of the settlement, 71,920 acres were irrigated, and the land revenue from that land in 1848 was Rs. $1,35,195$; hence the rate of land revenue for land irrigated at the time of the settlement is Rs. 188 per 100 acres.
« Of the same district, $4,58,896$ were (in 1848) unirrigated, and the land revenue secured amounted to Rs. $6,35,893$, hence the rate of land revenue for unirrigated land is Rs. 138 per 100 acres.
"The increase therefore due to the Eastern Jumna canal is Rs. 50 per 100 acres, or eight annas per acre."
143. As regards the Ganges canal, it would appear that it has not heretofore received credit for any increase of land revenue, and on Gangear canal, and it was impossible that the low water-rate that was charged up to the ${ }^{\text {ent to }}$ y yied a profft beginning of 1865-66, should alone suffice to return a profit on the capital expended.

Probable increase due to Ganges canal assumed at 8 annas per acre of irrigated area.
144. Should however an increase of land revenue equal to that which as above has accrued to the irrigated land under the Eastern Jumna canal, be credited to the Ganges canal when the new settlement comes into operation, the annual receipts will be increased by 8 annas per acre, or about 583,000 acres, or nearly 3 lakhs of rupees. From the returns obtained by Colonel Dyas from the Collectors, it would appear that in two districts alone, the enhancement due to irrigation amounts to Rs. 97,863, and the above estimate is more likely to be below than above the mark.

Water-rate increased from 1st May, 1865 .
145. Fortunately for the future prospects of the canal, the Government have lately sanctioned the introduction of increased waterrates. From 1st May, 1865, they will be equal to about double what they were before. The average yield for last year will be about Rs. 2-4 per acre. Should the area of land irrigated by flow of water or gravitation, as compared with that irrigated by machinery, be raised. as is certain to be the case when the full supply is admitted into the canal, the profit per acre will be considerably increased, and for future calculations, Rs. 2-8 is more likely than Rs. 2-4 to be the average rate.
146. The irrigation returns for 1865-66, have been received by

Revenue from canal for 1865-66. the Committee. The receipts will be as follows :-

| nts-Khureef, .. . .. .. .. .. Rs. 4,88,353 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Rubbee, .. | $\cdots$ | . |  |  | " | 7,43,806 |
| Miscellaneous receipts from sale of produce,.. .. \# 63,226 |  |  |  |  |  |  |
| Receipts from navigation, | .. | . |  |  | " | 43,662 |
|  | Total, Rupees, |  |  |  | - | 13,39,047 |

Charges and nett revenue.
147. The charges for the year for repairs and establishment which the Committee have obtained from the Controllor, P. W. Accounts, N. W. Provinces, amount to Rs. 5,98,531, the nett receipts will therefore be Rs. 7,40,516.
Retarn on capital 148. The total expenditure on new works on the canal, includoutlay. ing rajbuhas, up to the end of $1864-65$, was Rs. $2,08,14,654$, the interest at 5 per cent., $10,40,732$; the nett receipts only yield a return of fully $3 \frac{1}{2}$ per cent. ; but if the enhancement of land revenue due to the canal, which is estimated approximately at upwards of Rs. $2,90,000$, be credited in the receipts, the return for the year would be 5 per cent. on the outlay.

Failure of canal
hough in an unfinished state, of a modified kind.

Progressive improvement certain.
149. However, if only the canal revenue proper is taken into account, the above figures show that the financial failure of the canal, even in its present half developed state, is of a very modified kind.
150. An increase of irrigation may be confidently anticipated, and as the charges for establishment and maintainance do not increase in the same proportion, the nett receipts will go on steadily increasing year by year. While for the five years, from 1860-61 to 1864-65, the quantity of water discharged by the canal remained nearly uniform,
the irrigated area increased during that period from 3,42,909 acres to $5,66,514$ acres, the revenue (at the old water-rates) from Rs. 6,45,111 to Rs. $\mathbf{9 , 9 0}, \mathbf{8 6 6}$, and the area irrigated by each foot of discharge from 73 to 141 acres.
151. It is found on the old established canals from the Jumna, that each cubic foot of discharge irrigates from 190 to 290 acres; the average may be taken at about 220 ; and if the Ganges canal attains to this standard, its full supply of $\mathbf{6 , 7 5 0}$ cubic feet per second will irrigate $14,85,000$ acres. But allowing only 200 acres per cubic foot, the area would be $13,50,000$ acres, which at Rs. $2-8$ per acre, will yield $33,75,000$ rupees ; to which may be added, at the lowest computation, $\mathbf{2 , 2 5 , 0 0 0}$ for miscellaneous revenue receipts and for navigation tolls or in all $36,00,000$, exclusive of enhancement of land revenue.
152. Major Crofton estimates the total cost of the project, in- Charge on capital cluding all works in the original design, except the extension to Allahabad, at Rs. $\mathbf{3 , 1 0 , 0 0 , 0 0 0}$. If the additional works the Committee have recommended, raise the cost to Rs. $3,25,00,000$, the annual charge at 5 per cent. will be $16 \frac{1}{4}$ lakhs of rupees.
153. The maintenance eventually will, it is confidently expected, be on a far lower scale that it has been of late years, seeing that there have been unusually heavy repairs to masonry works, and extensive clearances due, not to the silt admitted from the river, but to the erosion of the bed and sides of the channel. Probably 7 lakhs per annum will suffice to cover all expenses. The nett returns would then be 29 lakhs, which would not only suffice to cover the charge of 5 per cent., but to pay off the accumulated interest of former years, and that once effected, to yield a clear profit of 8 per cent. per annum exclusive of the enhancement of land revenue.
154. Considering therefore all the circumstances noticed above, which have hitherto tended so materially to frustrate the success of shom Gand not dispearage conal the Ganges canal in a financial point of view, the Committee are of ${ }_{\text {workse }}^{\text {project }}$ for irigation opinion that its comparative failure up to the present time affords no ground for doubt of a fair and reasonable return from other irrigation projects, constructed with the express object of yielding a direct profit.
155. In concluding their enquiry into the financial prospects of

Ulimate duty of
 to be met.
yield by only 2 per cent., it would be to his pecuniary interest so wastefully to apply it.

Area irrigated per cubic foot per second on Ganges canal.
158. The number of acres irrigated on the Ganges canal by each 1 foot per second of discharge has been increased during the last five years from 73 to 141 in 1864-65.

Area irrigated on Eastern Jumna canal.

Capable of further increase.
159. The quantity irrigated by each foot per second of discharge on the Eastern Jumna in 1864-65 was 220 acres.
160. But this is far below the theoretical maximum; and, as in the latter canal the rajbuhas are now doubtless fully developed, indicates some defect in the system of disposal
Quantity of water required for a wheat crop. Mr. Login's evidence.

Opinion of cultivators.
161. Mr. T. Login, an officer of great experience in these sub-

Daty which should be performed per cubic foot of discharge. jects, reports that from careful experiment he found that four waterings of $2 \frac{1}{2}$ inches, or a total of 10 inches in depth of water, was amply sufficient for a wheat crop in these soils.
162. A common concord of opinion amongst cultivators is that one foot of water suffices for a wheat crop.
163. Allowing 20 per cent. of the whole discharge for absorption and evaporation, this would make the total required 15 inches in depth; or one foot per second of discharge should theoretically irrigate 580 acres of crops, consuming equal amounts to wheat crops, assuming that the water were fully utilised throughout the year.
Four times the quantity required for a wheat crop expended at present.
164. The quantity of land actually irrigated by the Ganges canal in 1864-65 was 141 acres per 1 foot of discharge per second, and the depth of water consumed was $5 \cdot 17$ feet, or deducting 20 per cent. $=4 \cdot 65$, or nearly four times the quantity actually said to be required for wheat crops.

Theoretical duty per cubic foot Khureef per cubic foot Khureef
and Rabbee combined.
165. But of the whole area irrigated, 30 per cent. consists of Khureef crops ; which, on the average, as at present composed, consumes nearly the same total quantity of water as the Rubbee crops. Assuming the present proportion of Rubbee and Khureef, and the present composition of Khureef crops to remain the same, and assuming that the water on the Khureef is equally utilised, then the area irrigated of average crops, both Rubbee and Khureef, should be 414 acres per cubic foot of discharge.

Economy of water not of importance heretofore, but will when canal is co pleted.
166. The Committee are aware that the subject of the disposal of water by cubic measurement has from time to time engaged the attention of the canal officers, but as owing to the imperfect development of the rajbuhas and to the demand for water for extension of irrigation being of gradual and slow growth, the necessity for economy of water has not been so urgent as it will be when the canal shall have been completed.
167. The Committee would thersfore recommend that the experiments on water- periments now being made at Roorkee should include a comprehenmeters. sive enquiry into the feasibility of bringing into use a cheap and effective water-meter, and they are of opinion that the great importance of the subject would warrant a liberal outlay.

## COMPARISON OF FINANCIAL RESULTS OF MADRAS IRRIGATION WORKS WITH THOSE OF THE GANGES CANAL.

168. The financial results of the Great Irrigation Works in $\begin{gathered}\text { Works in Northern } \\ \text { and } \operatorname{Southerr} \text { India }\end{gathered}$ Southern India have been so prominently adduced as examples of carried out and fer indef what the Ganges canal, if constructed on sound principles, ought to have yielded, that the Committee think it necessary to remark briefly on the very different circumstances under which the works in the Northern and Southern Provinces appear to have been carried out, which are explained more fully in separate papers,* recorded by two of their number.
169. It appears that in Madras nearly all the large works under- $\begin{gathered}\text { Madras } \text { Delta }^{\text {chiefly }} \text { dears } \\ \text { deigned } \\ \text { to }\end{gathered}$ taken by the British Government have been to an important extent for chiefly designed to the purpose of restoring and developing irrigation systems which, in a dion. more or less imperfect state, had existed for ages. In Tanjore especially extensive channels from the Cauvery covered the Delta, and needed only proper regulation to render them thoroughly effective. In the Godavery and Kistna Deltas also, it is stated, that numerous old channels, some of which were of considerable size, had served to distribute, though imperfectly, a supply of water from those rivers. They were incorporated with the Delta systems dependent on the anicuts, and formed serviceable feeders of the new canals, thus saping a considerable cost in excavation, and expediting the distribution of the improved supply of water.

1\%0. In other districts, extensive reservoirs of ancient construc-
Ancient reservoirs improved by the contion, required only the powerful aid of weirs across the feeding rivers, striction of weirs. and in some cases connecting lines of channel, to restore and develope their storage capacity to its fullest extent.
171. Further, the rapid extension of irrigation in the South has been materially promoted by the fact of the people being accustomed to that mode of cultivation, and by their readiness to avail themselves of the large tracts of waste land fit for the cultivation of their great staple, rice, which can be raised only by artificial flooding in all the irrigated districts.
172. The Ganges canal on the other hand is an entirely new pro-

Rapid extension of irrigation in Madras.

 old systems of irriga-
$\qquad$

[^8]$\square$
$\qquad$ ject from which no return could be derived, till the main works had Ganges canal. been completed, and the results of which must necessarily depend on the gradual development of an entirely new system of branches. Wheat, the great staple produce, can be raised in favorable seasons at moderate expense by well irrigation, and the proportion of available waste land is but small, added to which, although the water-rate has been fixed very low, the sale of water depends wholly on the demand

[^9]on the part of the landholders, which it is understood is not the case in Madras, while canal irrigation in these provinces is of but recent introduction ; and not, as in the South, a system practised fot centuries.

Difference of relative levels to the land between the Delta rivers and those of the Northern Doab.
173. The Committee consider the foregoing causes sufficient to account for a great and apparently disadvantageous contrast between the North and South; but when it is also borne in mind that owing to the essential difference between the relative levels of the Delta rivers to their field of irrigation, and those of the Northern Doab to the adjacent land, a vast additional expense must be incurred in the latter case, in bringing the water to the surface, whether by taking off a canal from above or below the rapid slope of the bed following the debouchment of the river from the hills; they cannot but feel that no just comparison can be made between the results of works so very different in character as those referred to.

## ACKNOWLEDG $\ddagger$ MENTS.

174. With reference to para. 10 of the order of Government, No. 20,410, the Committee desire to acknowledge the assistance and information they have received from Colonel Dyas and the Executive Engineers under his orders.

They were also favored with the valuable evidence of Mr. T. Login, late Superintendent of the Northern Division of the canal, as to its past history.

They regret that owing to the urgent pressure of his duties in Cuttack, Colonel Rundall was unable to join them in their inspection of the Ganges, and although that officer offered to reply to any written questions referred to him, they did not consider it expedient to delay their proceedings for that purpose, when personal examination of the localities seemed essential for arriving at a correct opinion; so far however as can be judged from Colonel Rundall's unofficial communications, it would appear that his views as to the best site for the weir and line of canal near the Solani, do not differ materially from those adopted by the Committee.
175. The Committee would further record their best acknowledgements of Major Crofton's unwearied efforts to promote the objects of their enquiry, both by efficient arrangements for their movements and by full explanations of his own plans.

[^10]
## DISSENT.

1. I dissent from the above report mainly as to conclusions No. III. and No. IV., the first negativing the proposed canal'ffrom the Jumna near Delhi (bringing 3,300 feet per second to complement the present 4,400 now brought down by the main canal), as a substitute for the remodelling above the junction; the second recommending that the remodelling, according to Major Crofton's plans (with certain modifications), be at once proceeded with.
2. In reference to the latter's conclusion, I dissent from paras. No. 140 and 142 of the report, which state that the main difficulty in the remodelling, as proposed by Major Crofton, lies in the depth of the cisterns below the new falls, and which further state that if these cisterns be modified, the work up to water line many be executed during two closures of the canal, each of $3 \frac{1}{2}$ months duration.
3. I dissent from this latter conclusion, because Major Crofton has, after careful investigation, expressed his opinion, in para. 102 of his report, that to execute the work as designed by him, the necessity for shutting off the canal supply for at least one irrigating season is inevitable, and that an entire year would not be too large an allowance ; because Colonel Dyas, the Superintendent, Irrigation Works, N. .W. Provinces, has concurred generally in that opinion; because in my judgment the period assigned is rather understated to carry out the works as designed; and because I do not regard the deep cisterns?as the most troublesome part of new falls, nor nearly so likely ${ }^{\text {to }}$ to cause uncertainty and delay as the well foundations; and I decidedly do not think that the proposed modification of the cisterns would render it probable that the work up to water line could be executed (except at entire disregard of cost) during two periods of $3 \frac{1}{2}$ months stoppage of the canal.
4. Further, I am not satisfied with the estimate of probable loss from such a closure for $3 \frac{1}{2}$ months, Colonel Dyas having been asked for a statement of such loss and having not yet given it because he had not sufficient information.
5. In respect to conclusion No. IV., I dissent because I hold that it is only as a last resort that the serious evil of stopping the canal supply for a whole year, if not longer, should be incurred, and because the proposed canal from the Jumna offers a mode (if the 3,300 feet per second discharge of the Jumna and Hindun rivers can be depended upon, and for which conclusion there appears to me to be strong primad facia ground) of making up with the 4,400 feet per second now brought down the main canal, an amount of water supply in excess of that provided under the remodelling scheme, at a less, or certainly not greater cost, and without incurring the serious evil of stopping the canal supply.
6. For the further elucidation of this subject, I refer to the Memo. in that project, printed in Appendix A., and I recommend that during the next season, a complete survey should be made of that project, and that, in addition to daily record of water level, the discharges of the rivers Jumna and Hindun should be taken on the 15th of each month, from October to May inclusive, and that if the result should show that a quantity approximating 3,300 feet per second can generally be relied upon during those months, then the canal from the Jumna should be made in supersession of the remodelling above the junction, if it be determined not to execute both, or in priority to the remodelling if it be determined that both should be carried out.
7. I further recommend that, meanwhile, stone and other material should be collected at existing falls, sufficient for their complete repair should failure occur, but that no further steps should be taken towards the remodeling above Nanoon junction, until the question of general minimum discharge of the Jumna and Hindun be settled.
8. I dissent generally from paras. No. 70 to 89 of the report, treating of the Jumna project.
9. In reference to para. 71 of the report, in which it is stated that the water gauge kept at the railway bridge at Delhi, was not a safe guide, because it was affected by the damming up of the bridge spans for girder erection purposes. I would point out that this damming gradually increased in extent from the beginning to the end of the dry season; and, consequently, so far as it affected the water level, would have the effect of making it appear relatively higher in April than in December.
10. I dissent from para. 73, which decides to adopt the same section for weir at Toghlukabad on the Jumna as at Sookertal on the Ganges. At Toghlukabad there is a practicably unlimited supply of stone from the old fort, much of it dressed, and any further quantity of rough stone can be quarried within 3 miles of the work.
11. At Sookertal there is no stone within 50 miles, and probably no sufficient supply of fit stone within 66 miles, and this last has not yet been explored.
12. These conditions are so essentially dissimilar as, in my judgment, to demand different treatment.
13. For the above reasons $I$ dissent from the estimate of weir across Jumna given in para. 81.
14. In reference to para. No. 101, I do not concur in ascribing the deep holes below the falls, chiefly to the increased velocity with which the water escaped from the floorings.
15. I think that the most serious part of that scouring is due to the wings which curve inward, thus reducing the width of the channel, and throwing a current at right angles to the direction of the stream, by which a rotary motion is produced, and deep scouring naturally followe.
16. Had the scouring been chiefly due to the velocity of the water over the crib work, the greatest depth of scouring would naturally have been looked for immediately close to the crib work, where the light soil joins the protective work; but the deepest scour is not found there, but at a considerable distance lower down, and just where the conflicting currents are converted into a rotary motion.
17. At Nirgajnee falls (next below the Mahmudpoor falls) which is without these inward-curved wings, but little scour has taken place.
18. I would recommend that these inward-curving wings be altered at once, the work could be done without interrupting the canal.
19. On the subject of the supply of stone referred to in paras No. 130 and 132, it should be noted that Colonel Dyas' experiments were made on the Sewalik sandstone lately brought in, and are not necessarily to be regarded as representing the crushing weight of the sandstone seen in the old buildings at Hurdwar.
20. A more important point on the subject of stone is that the report does not, in my opinion, lay nearly sufficient emphasis on the importance of examining the apparently unexplored field in the main Himalayas in the neighbourhood of Luchmun Joola and Tupobun, where, at a distance of about 17 miles from Hurdwar, the hills for miles in extent appear to be formed of a hard ferruginous clay slate, and rather farther up the valley of the Ganges, of evenly stratified whin-stone.
21. Should a good building stone be discovered there, it would reduce the estimate for the weir at Sookertal by upwards of 4 lakhs, and would reduce the cost of ashlar stonework executed on the upper portion of the canal by from 30 to 50 per cent.

GEORGE SIBLEY.

## APPENDIX A.


#### Abstract

Memo. on a projec! for making a canal from the Jumna at Toghlukabad, about 10 miles below Delhi, to the main canal near Birowlee bridge (or to the Simra falls), about 20 miles above Nanoon junction, the head of the Cawnpore and Etawah branches.


1. In considering the relative merits of the various schemes for obtaining the desired result, of bringing down the full amount of water which the canal was originally designed to carry, there should be kept constantly in view the vital importance of not closing the canal and stopping irrigation, except as a last resort.
2. In para. 97 of Major Crofton's Report he writes, " the absolute " necessity of maintaining uninterruptedly the present volume of water " to supply existing irrigation, unless there be very cogent reasons to " the contrary, is assumed by all."
3. In para. 102, he states his conclusion that for executing the work proposed by him in remodelling, "the necessity of shutting off " the canal supply for at least one irrigating season, or half a year, " appears inevitable, and to do the work satisfactorily and give time " for the larger masses of masonry to harden sufficiently, an entire year " would, in my opinion, not be too large an allowance."
4. This opinion is concurred in generally by Colonel Dyas, Superintendent Irrigation Works, N. W. Provinces.
5. The Committee of Engineer Officers summarised the advantages of a duplicate channel to complement the supply, as presenting -

1st.-Facility of construction, and of forming reliable estimate of time and cost of construction.
$2 n d$.-The very great convenience that may be anticipated from the possession of a duplicate line of main channel, which would admit of a considerable amount being sent down to the lower parts of the canal, while either of the main lines was closed for repairs.
6. Sir P. Cautley (in page 75 of the Discussion) writes, "the " remedy however appears to be rather in the division of the great body " of water and thereby in diminishing the effect of its action, than in " the continuance of the existing channel as a single line:" again (in page 76) he writes, "I am inuch in favor of reducing the present volume
" of water in the main canal, with so many falls and so large a body of
' water passing over them, perpetual repairs and interruption will in-
" evitably occur, let the slope be reduced to any extent. By the divi-
" sion of the waters this will be avoided, and the evil of accident ou one
" line will (as far as supplies for irrigation to the South are concerned)
" be neutralised by the existence of an alternative line."
7. Sir P. Cautley also, in his Memo., printed in Appendix A. to Major Crofton's Report, insists on the value of having an alternative line for the security of the irrigation of the Southern districts; in this view, and also to avoid the serious evil of stopping the irrigation for an indefinite period Sir P. Cautley proposed a duplicate channel, leaving the main channel below Roorkee.
8. The Government of India in reviewing Major Crofton's Report rejected this project for an alternative channel--1stly, because it appeared from Major Crofton's figures that it would be more costly than the remodelling by some 30 lakhs; and secondly, and chiefly, because it appeared that the really serious objection to the remodelling, the necessity for closing the canal for a while, was found to be equally applicable to that scheme.
9. The Government of India at the sametime remarked that, had there been a prospect of the scheme being practicable without the necessity for closing the canal, it might have been thought desirable to incur additional outlay to avoid such serious interference with the convenience of the agricultural community, though it might have been doubtful whether even on that ground so large a charge as 30 or 40 lakhs should be incurred.
10. Major Crofton's comparison of the cost of the remodelling with Sir P. Cautley's alternative line, was based on the supposition that the present channel could carry with safety only 1,870 feet per second below Roorkee, leaving the alternative channel to carry 5,000 feet per second, or nearly three-fourths of the whole quantity.
11. Now, it appears that, since the last closure in August, 1864, (a period of 21 months) the canal has carried regularly 4,400 feet, being $2 \frac{1}{3}$ times the estimated safe quantity; that the action in the bed, falls and bridges, has been carefully watched,. and that no injurious action worth noting has been ascertained to have occurred.
12. This fact, as to the comparatively favorable condition of the works, if well established, would lead to the conclusion that only 2,500 feet instead of 5,000 feet need have been provided for in the new channel, and would so far modify the comparison of cost between the alternative channel and the remodelling scheme; as, however, the alternative channel was rejected, not so much on the ground of extra cost alone, as on the ground that the serious evil of closing the canal appertained to that scheme also, it does not appear to be necessary to go into the question of how far the comparison between the two projects would be modified.
13. Without having the canal laid dry, it is of course impossible to state with absolute certainty whether any seriously injurious action has taken place, but the concord of evidence from surface observation leads to a contrary presumption.
14. The Committee made a formal request to Colonel Dyas, the Superintendent of Irrigation Works, N. W. Provinces, to have the waters shut off for a few days, so as to enable them to satisfy themselves, by actual inspection, as to the condition of some of the more important and previously threatened works, but this request was met by objections so numerous and so strongly urged as almost to amount to a protest, and under these circumstances the Committee decided not to press their request.
15. Colonel Dyas reports as to the condition of the works since the date of Major Crofton's Report, as follows:-
" As to the bed, erosion is no doubt going on in many places, in" asmuch as during the cold weather, when the water coming in from " the Ganges river through the Myapoor regulators (the head of the "canal) is almost quite clear, the water in the canal gradually becomes " very muddy ; but a comparison of the cross sections of the canal bed, " which I have lately had taken at-every mile along the canal, with " similar sections taken by Major Crofton in 1864, shows that no dan"gerous action is going on at present.
"As to falls in the Northern division (from Myapoor to Jaolee) " the extent of damage done, as far as can be ascertained by careful " sounding and probing, consists of a slight displacement of crib work " battens. In the Meerut division two falls have been slightly injured, " but not sufficiently as to warrant a closure of the canal for the pur" pose of repairing them. The Chitoura fall, 55 miles 4,976 feet, has had " part of the ogee in No. 3 bay ripped out, and that bay is kept close " in consequence to prevent further damage ; and the Sulawur fall (67 " miles 2,350 feet) has had two of the hammer-dressed Delhi stones in " flooring of No. 3 bay turned over : this occurred in September, 1864, " and the stones have not moved since. The bridges are all secure."
16. The result of the last 21 months' working appears therefore nearly conclusively to show that for a-lengthened period this quantity of $\mathbf{4 , 4 0 0}$ may be safely sent down the present channel, so that if a quantity equal to that which is required for the irrigation below Birowlee bridge, on the Simra falls, can be brought in from any independent source, the whole irrigation below that point may be pelmanently secured ; and if by repairs, such as may be effected during the ordinary periods of closure (the cost of which Major Crofton estimates at Rs. $1,06,424$ ); the old channel may be made securely to carry permanently the present 4,400 feet per second, the result would be that a total quantity of $\mathbf{7 , 7 0 0}$ feet per second would be available for irrigation (as compared with the 6,742 feet of the remodelling scheme), and this result would be obtained without stopping the irrigation.

## ( iv )

17. It appears from an examination of the country and of the levels, that such a complementary channel could be made at moderate cost by leading the water from the Jumna, opposite Toghlukabad, about 10 miles below Delhi, crossing the Hindun, and taking its waters also, and joining the main line at or near Birowlee bridge, near the 158th mile from Hurdwar.
18. The minimum discharge of the Jumna for this year, as appears from sections taken by the Resident Engineer, of the Jumna bridge, on the 9th April, 1866,

$$
\begin{aligned}
& \text { Being } \quad \ddot{\quad . .} \quad . \quad . \quad . . \\
& \text { And that of the Hindun, taken by an Assistant } \\
& \text { Engineer of the Railway at Gazeeabad, } \\
& \text { Gives a total, }
\end{aligned}
$$

feet per second, which is very nearly the full amount required below the Birowlee bridge, on Simra falls, according to Major Crofton's scheme.
19. This discharge is corroborated by the discharge taken at Agra by Mr. Dodd, Executive Engineer, on the 5th April, 1866, which showed 3,544 feet per second, and which he reports to have been the lowest for the season.*
20. The Resident Engineer of the Delhi Railway bridge, who took the discharge on the 9th April, and who is on the river daily, reports however, that he is perfectly confident that the river was at its minimum for the season on the 9th April, that there was only 9 inches difference between water level on that date and on the 19th December, a difference that was fully accounted for by the channels having been bunded for the purpose of girder erection.
21. There appears to be at least sufficient primâ facie evidence of the general correctness of the minimum discharge taken, or $\mathbf{2 , 8 2 0}$ feet per second, to demand further investigation.
22. Toghlukabad is a very favorable position for the building of a weir across the Jumna-firstly, because the river bank, opposite the village of Alee, is of a very firm character (kunker and clay); and secondly, because there is a practically unlimited supply of stone admirably suited for such purposes, a very large amount of rough stone and a very considerable quantity of dressed stone (Delhi quartzite); being available from the ruins of the old fort of Toghlukabad, which is also built on rock.
23. A channel 130 feet wide at base, 10 feet deep, with slopes 2 to 1 , and fall of $4 \frac{1}{2}$ inches per mile, giving a velocity of $2 \cdot 20$ feet per second, would discharge 3,300 feet per second, and would fall into the main canal at about the 158th mile from Hurdwar, or about 22 miles

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(\mathrm{v})
$$
above Nanoon junction, the head of the Cawnpore and Etawah branches.*
24. An approximate estimate of such a channel and weir shows that they could be executed for about 39 lakhs ( $£ 390,000$ ) including superintendence.

The amount is composed as follows :-


Land, 57 miles at 64 acres per mile, at Rs.

$$
24 \text { per acre, .. .. .. .. 87,552 }
$$

$$
\text { Total, Rupees, } \quad . \quad \overline{39,16,965}
$$

25. In the above estimate earthwork cutting, depth average 10 feet, has been taken at Rs. 3 per 1,000 ; and average depth 20 feet, Rs. 4 per 1,000. In the estimate for weir at Jumna, the rough rubble stone is taken at Rs. 10 in place, and the dressed masonry at Rs. 30 per 100 feet; probably the whole of the material required being obtainable from the old fort of Toghlukabad; or if further rubble stone be required, it can be quarried within 2 miles of the weir.
26. It is considered that with the mass of rubble stone available, and with a good puddle clay on the spot, wells would not be needed under the weir wall; if a double row of wells be added, they would increase the estimate by about Rs. $\mathbf{2 , 0 0 , 0 0 0}$.
27. In the estimate for Hindun river weir, the actual rates of work in the Hindun railway bridge have been adopted.
28. The percentage for contingencies and for establishment have been taken the same as in Major Crofton's alternative line.
29. To the total, .. .. .. .. .. 39,16,965

As above, there is to be added-
Remodelling between Birowlee bridge and
Nanoon, .. .. .. .. 3,58,458

Repairs above Birowlee, .. .. .. 1,06,424
Re-adjusting rajbuha heads above junction, $\quad 1,00,000$
Total, .. .. 44,81,847
$\left.\begin{array}{c}\text { Remodelling Cawnpore branch, as per Ma- } \\ \text { jor Crofton's estimate, }\end{array}\right\} \quad$.. $\quad \mathbf{4 , 2 6 , 8 4 2}$
Giving as expenditure, Grand Total, Rupees, .. $\overline{49,08,689}$

[^12]$$
(\mathrm{vi})
$$
30. Major Crofton's estimate for remodelling the old canal-including the Cawnpore branch is, .. .. 39,19,850
Uf which is debited for navigation above Birowlee,. . $9,05,000$
Leaving debitable to irrigation, Rupees, .. 30,14,850
Now, though the above amount ( $9,05,000$ ) may as a part of a whole be fairly debitable to navigation, it is doubtful whether the work due to irrigation alone could be executed for the balance.
31. Assuming however the above figures, to that balance, would have to be added the difference of cost between that of a permanent dam at Hurdwar, without which the full supply could not be relied on, and the capitalisation of cost of maintaining present bunds which suffice for present supply, this difference will probably not be less than Rs. 5,00,000.
32. Again is to be added the amount of loss and compensation due to the stoppage of the canal. In Major Crofton's estimate, this is put down at Rs. 13,48,213, but in this the canal revenue is taken at only Rs. 7,73,390, since the date of that estimate the rates have been raised, and it is expected that the canal revenue will, before the work can be undertaken have reached 15 lakhs.

|  | Rs. | Rs. |
| :---: | :---: | :---: |
| To the above, Will therefore have to be added, |  | 13,48,213 |
|  | 15,00,000 |  |
|  | 7,73,3.0 | 7,26,610 |
| Giving a revised total, |  | 20,74,823 |

33. The cost of the remodelling scheme then, exclusive of navigation above Birowlee, becomes-

RS.

| Works, as per Major Crofton's estimates, . . | 30,14,850 |
| :---: | :---: |
| Permanent head works (difference in cost), | 5,00,000 |
|  | 35,14,850 |
| Losses and compensation, | 20,74,823 |
| Grand Total, | 55,89,673 |
| The cost of the duplicate channel scheme from the Jumna and remodelling below junction being, | 48,08,689 |
| Shows a difference of, | 6,80,984 |

34. The rates for the duplicate channel, executed in the dry, have been taken at the same as in the remodelling; but it may be safely predicted that in work executed under pressure for time, with a "working bee," as proposed, the rates would rule far higher in the remodelling than in the duplicate channel.
35. As previously noted, the Government of India remarked, that it would be desirable to incur additional outlay to avoid such a serious interference with the convenience of the agricultural community, as would be caused by the closure of the canal; it appears however from the above figures that the duplicate line would cost absolutely less than the scheme, which involves the stoppage of the canal.
36. The disadvantages of the duplicate channel may be stated to be as follows :-

1st. That it does not improve the navigation above the junction near Birowlee to the extent contemplated in the remodelling scheme; leaving, as it does, a velocity of 4 feet per second where only 3 feet would be given by that scheme. The minimum headway is left nearly the same in both schemes by the water level being kept 3 feet 6 inchcs lower in the one than in the other. It is to be remembered, moreover, that the cost of such improvement is also omitted from the calculations; the improvement of the navigation below Nanoon (the really defective portion) is equally provided for in both events.
2ndly.-The asserted extra cost of maintaining and working the extra lengths. This is doubtful, the certain amount of extra establishment will probably be more than counterbalanced by the smaller bodies of water to be dealt with, and by the fact that $\mathbf{3 , 3 0 0}$ feet is carried only 57 miles in lieu of 158 miles.
3rdly.-The loss of head in the rajbuhas owing to the full depth originally intended not being maintained in the canal. Should it be deemed advisable in any particular case, the additional head could be obtained by taking the rajbuha from the fall above the present outlet-a sum of Rs. $1,00,000$ has been provided for that purpose in the estimate. It is to be remembered that in any event many of the rajbuhas will have to be remodelled, owing to them being below the surface.
4thly. - The extra loss by absorption and evaporation. As regards the first, the 3,300 will have to travel only 57 miles before being utilised in lieu of 158 miles, and the latter is scarcely worth taking into consideration, as assuming an evaporation equal to 60 inches per annum, the total loss on the channel, 57 miles long, would be equal to a discharge of only 8 feet per second.
5thly.-The injury to navigation in the Jumna by abstraction of the water for the new canal. The whole of the water is at present abstracted from the Jumna at the canal heads, and yet there is nearly 3,000 feet flowing at Delhi, 140 miles lower down. Agra is 140 miles below the new proposed
weir; and practically there is in the cold weather no navigation above Muttra, but little above Agra, and but imperfect above the junction of the Chumbul river.
37. Among the advantages of the duplicate channel (from the Jumna scheme) are-

1st.-The maintaining irrigation permanently over, at least, the lower half of the main canal, and most probably over the whole, without any break from closing.
2nd.-Facility of construction, and of forming reliable estimate of time and cost of construction in the dry.
$3 r d$.-The great convenience of a duplicate line of main canal.
4th.-The provision without extra cost of an additional 57 miles of first class navigation, without a lock in the whole length.
38. The balance of advantages appears to be so clearly in favor of the duplicate channel that, if the quantity of 3,300 feet per second can be generally relied on from the Jumna and Hindun-I have no hesitation in recommending the duplicate channel from the Jumna near Toghlukabad in preference to, and in supersession of the remodelling of the main canal above the point of junction.
39. As a matter of course no works should be commenced until that point, as to the discharge, is established; and I would recommend that, during the ensuing season, a complete survey should be made of this project ; and that, in addition to the daily record of water level, the discharges of the rivers should be taken on the 15th of each month, from October to May inclusive.
40. I would also recommend that stone and other materials be collected at the existing falls, sufficient for their complete repair should failure occur; but that no further steps be taken towards the remodelling above Nanoon junction, until the question of general minimum discharge of Jumna and Hindun be settled.
41. Having regard further to the facts that the above total of 6,742 feet per second is calculated, at a standard 40 per cent. in excess of present duty, to irrigate only) $1 \frac{1}{4}$ million acres (or if 7,700 feet per second, about $1 \frac{1}{2}$ million acres), and though this higher standard of duty will doubtless be much further raised when the system of measurement by volume shall be generally introduced; yet, seeing that the culturable area of the Doab above Cawnpore is upwards of nine million acres, of which only about $\mathbf{2 , 2 5 , 0 0 0}$, are irrigated by the Eastern Jumna canal, it would appear worthy of consideration whether, when the duplicate channel shall have been completed, and the irrigation of the lowest half of the canal secured, the remodelling of the main channel above the junction might not then be profitably carried out, generally as proposed by Major Crofton, but with modifications (except as to suggested shallower foundations for cisterns of new falls), as recommended by the Committee, when remarking on that project, and with such further modifications if possible (whether by use of

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(\mathrm{ix})
$$

iron-piling or otherwise) as may avoid the necessity for any lengthened closure of the canal.

Estimate of the cost a canal from the Jumna near Toghlukabad, 10 miles below Delhi, and the main line near Birowlee bridge, on Simra falls, a length of 57 miles, to carry 3,300 feet per second.


GEORGE SIBLEY.

## APPENDIX B.

## Notes on the Comparative Financial Results of Irrigation Works in the Madras Presidency and the N. W. Provinces.

The financial re sults of irrigation works in Madras ove works in $\mathbf{M}$.

1. I have long been of opinion that the financial results of great irrigation works in the South of India have been over estimated, and that fallacious ideas of what may be effected elsewhere have been formed upon erroneous data, but it has not hitherto been any part of my duty to investigate the subject.

Other canses of increased revenue.
2. That the general prosperity and revenue of the districts in which such works have been carried out have been enhanced to a degree unknown in less favored provinces is an unquestionable fact, but as all of them are on the seaboard, it is certain that the great increase of foreign trade, and of the salt revenue, which has taken place in the last 20 years, must affect this result, and credit cannot I think be taken for the entire increase as due to the works of irrigation.

The works not entirely new.
3. Moreover (as the Government of India have recently suggested) the returns in many cases are the effect, not merely of British science and expenditure, but of an extensive system of hydraulic works which had been in operation for ages before the country came under our rule, but which had suffered deterioration, partly from from natural causes, and partly from neglect, and have only recently been restored to efficiency.
4. It is well known that the works in Tanjore are the prototype been carried on from time immemorial under the river Cauvery, the channels from which covered the province, and made it the "Garden of Southern India." In 1842, Major H. C. Cotton wrote as follows: "The main branches of the Cauvery with the branch which retains the parent name, the minor channels separating from these, and the innumerable ramifications threading the surface of the Delta, are so disposed that one can scarcely believe it was a gradual work, each portion the accidental result of what had been done before, but would rather suppose it the work of consummate skill and seience, planning the whole system at once and establishing it. Changes have taken place during the last 40 or 50 years, disordering in some measure

## (' $\mathbf{x i}$ )

the irrigation of certain tracts, and remedies have of late years been applied, but these have been almost entirely confined to restoring these tracts to the condition they had been in at some former period. The deterioration which had occurred was due to the tendency of one of the arms of the river to absorb a continually increasing proportion of the whole of the supply, and thus to injure the irrigation dependent on the other arm. The remedy applied was the construction of weirs or anicuts across the river, and various other works, for the regulation of the supply to different channels." These works were in the highest degree successful, and the revenue which had been falling off for a number of years, not only regained its former maximum but rose far above it, and produced returns exceeding 100 per cent. on the amounts actually laid out in the works above described. It is, however, clear that such profits were due not to these works alone, but to the $\begin{gathered}\text { The to } \\ \text { profts not } \\ \text { note }\end{gathered}$ improvement they caused in the pre-existing canals by which their alone. benefits were conveyed to every part of the Delta, and which had cost the British Government nothing.
5. So also the construction of weirs across the Vellaur, Palaur, Pennair, Ponnyaur, Poiney, and other secondary rivers on the Eastern coast, was expressly designed for the better supply of large reservoirs ment. of ancient formation; which at the best had received but a precarious supply and had yielded an uncertain revenue, and which had gradually become inefficient from deposits of silt and general decay; but it cannot be fairly said that the returns in the shape of increased revenue are owing to the weirs alone, independently of the vast embankments by which the increased supply of water was stored, and which likewise were heir-looms from former rulers, by whom their advantages were fully appreciated.
6. To institute a comparison between the remunerative returns from such works as those of Southern India, and the financial results of an extensive and costly project like the Ganges canal, involving gea canal fallacios an entirely new system of hydraulic works over a tract of country hitherto destitute of all such artificial aids to cultivation, and in which the Engineers themselves were almost as much concerned in teaching the people the value of canal irrigation, as in constructing the works, is obviously fallacious, and calculated to lead to unfounded expectations and disappointment.
7. The greater part of the Southern Presidency depends mainly for prosperity on its ancient system of irrigation by channels and reservoirs, without which it would be almost a desert, while the $N$. W. Provinces are favored with a fine climate and general fertility of soil, capable in ordinary seasons, of raising a great variety of produce without artificial irrigation, the introduction of which by the British Government was avowedly intended, not as a commercial investment, but as a means of mitigating the effects of those disastrous seasons with which Northern India is sometimes visited; and it is on

The Madras Presidency dependent on works, but the N. W. Provinces not so, and the canal constructed as a preventive of occasional famine.
such occasions that the profits derived from the canal are highest, because the demand for water is greatest.

Difference of revenue systems.
8. The great difference between the Madras revenue system, under which irrigated land bears a permanent increase of assessment, and the voluntary and annual purchase of water by the Zemindars of the N. W. Provinces, is another material cause of the discrepancy upon which so much stress has been laid. The Madras Ryot pays the same amount to Government, so long as his fields are irrigated whatever the season may be, but the Zemindar of Cawnpore or Meerut looks to the clouds, and purchases canal water or not, as he may judge the prospect of rain to be favorable or otherwise.
E. LAWFORD.

## APPENDIX C.

1. I am quite willing to accept Sir A. Cotton's estimate of the benefits derived from the Godavery and Kistna works. But I am of opinion that these works were carried out under exceptionally favorable circumstances; and that there are few, if any, non-Delta formations in India where the same results could be attained at the same ${ }^{\text {ecnted. }}$ proportional expense. Not only would Government be disappointed were they to take the returns from the Madras Deltas, as the standard which properly managed irrigation works ought generally to yield, but private companies might be led, through the same mistake, to embark in schemes which a real knowledge of the Delta works would have shown them, could not be of an equally profitable character. In the belief that a description of the peculiar advantages under which the Godavery and Kistna works were executed will not only be useful in clearing away much mis-apprehension that prevails regarding them, but will help the Government to understand better than they seem to do at present, the cause of their yielding a much higher rate of profit than the irrigation works in the North of India, I proceed to impart such information as I possess on the subject.
2. In the Godavery and Kistna Deltas irrigation from old chan-

The high profits deived from Godavery and Kistna works attribatable to the exceptionally favorable circumstances under which they were ex-
 were commenced. In the reports on the Godavery, frequent allusion is made to the old channels. They are described as being very imper- Papera $\begin{gathered}\text { Vrofesional }\end{gathered}$ fect, as they opened from the river bank on a high level, which ren- gineers, Vol. III. dered then liable either to get an insufficient supply when the river ${ }^{0}{ }^{0}$ rs s Report, No. 184,
 (ton's Report, No. 13. the channels however was not only considerable, but they afforded the the means of at once distributing the water from the new main channels, and they possessed an agricultural class ready to use it as soon as it was offered them. The new works were thus enabled to return a profit much earlier than they could have done, had an entirely new system of distribution channels formed a part of the project.
3. That the old channels in question must have been valuable valuable accessenadjuncts to the new works, is shown very clearly in following extract ${ }^{\text {ries to new works. }}$ from Captain Orr's Report. "By what has been shown as the benefit Professional Prderivable from the anicut by means of the channels immediately ${ }_{\text {neers, }}^{\text {pers, Mal }}$, Midras ${ }_{\mathrm{p}}$ affected by it, it will be seen, that with an expenditure of 9 lakhs, ${ }^{11}$ an annual increase (calculated on the lowest data) of Rs. 1,00,451 would be obtained, a result of itself sufficient to justify the construction of the anicut.

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Old tanks and channels in Kistns Deita. Former sapply defective.
4. In the Kistna Delta there were not only irrigation channels of considerable size, but a large number of tanks, both of which have been of invaluable service to the new works. Of the channels I may particularize the Pullairoo in the northern section of the Delta. Though now of moderate size, about 50 feet near the head, it is evidently an ancient arm of the river, running on a ridge like the Kistna itself, and admirably adapted for distributing the water for irrigation. It sufficed by means of numerous small branches to irrigate a large proportion of the Delta; that is when it had water. Before the construction of the anicut at Bezwada, it was liable like the channels in the Godavery, to receive either an insufficient or excessive supply, according as the freshes might be below or above the average, and like them and the channels in Tanjore, it only wanted a regular supply to secure the revenue due to the whole of the land under it. This want the anicut combined with a new head 15 miles in length, supplied, and the desired result was at once attained. The importance of this channel and its value to the new system may be understood from the fact, that when 65,000 rupees had been expended in the course of about eight years subsequently to the admission of water from the anicut, solely for clearance and repairs of the channel and its branches, 6,500 rupees only had been expended during the same period in new works and improvements. The cost of repairs to the Delta channels is under 8 annas per acre of irrigated area, and the water-rent was until lately 3 rupees per acre, or six times as much. The large expenditure on repairs therefore represents a large irrigated area.

Profit derived by throwing in an improved supply by means of new heads.
5. Another channel existed in the southern section of the Delta and conveyed water to a number of important tanks to supplement the supply from the local rain-fed streams. .A cut of 12 miles in length connected it with the anicut, and changed its supply from a variable and uncertain quantity to a certain and uniform quantity. There were 17 tanks under this channel, and the average revenue derived from them, from 1851 to 1855, that is for four years prior to the introduction of a supply from the anicut, amounted to Rs. 52,929 , the minimum being Rs. 31,458, and the maximum Rs. 70,092. The revenue derived from the same tanks in 1863, the last year of which I possess the accounts, was Rs. 1,39,323 showing an increase over the former average of Rs. $\mathbf{8 6 , 3 9 4}$. The fluctuation of the revenue before the admission of water from the anicut was very remarkable; thus in the course of four years only, four of the tanks yielded respectively a minimum revenue of Rs. 0, Rs. 10, Rs. 323, and Rs. 123, and a maximum of Rs. 3,321, Rs. 2,908, Rs. 3,663, and Rs. 6,327.
Precarions cultivation done way with; revenue secured and increased.
6. The supply of the tanks was formerly very precarious, and the above examples testify to what extent both Government and the Ryots were liable to suffer. The immediate effect of the more certain supply from the anicut was to give confidence to the Ryots, to secure

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the revenue at the highest figure to which it could have risen had the tanks received a good supply from rain, and by doing away with all risk of loss to the cultivators, to induce an extension of the cultivation, and a further increase of revenue. But without the aid of the tanks and the channels leading to them, which were, as I may say, superadded gratis to the anicut works, the same increase of revenue could not have been attained, excepting by a large additional expenditure on new channels or tanks, and a delay of several years.
7. Besides the above, there are another series of tanks in the Kistna Delta which were fed by a number of small channels from the river. A short branch from the new main channel into a cutting which had been formed to make an embankment along the river, fed these tanks with a regular, instead of their former precarious supply, and a large increase of revenue was the result.
8. It will thus be seen that the Godavery, and still more the Kistna Delta works, started in possession of some advantages over an entirely new system of works like the Ganges canal, where not a single village channel existed along the length and breadth of the country to be irrigated, and where the cultivators were unused to any other mode of irrigation but that by means of wells. The Godavery and Kistna works have other advantages in regard to the alignment of new channels, which alone would render a comparison between them and the canals in the N. W. Provinces, altogether unfair.
9. The anicuts which have been constructed across the Go- Depth of cutting davery and Kistna are about 14 and 19 feet, respectively, above the $\begin{gathered}\text { nols in } \\ \text { Godarery and }\end{gathered}$ bed, and the ground along the banks may be from 13 to 17 feet above the crest of the anicuts. The heads of the main channels are between 5 and 6 feet lower than the crest, consequently the depth of cutting will be from 18 to 22 or 23 feet.
10. If this depth of cutting had to be maintained for any consi- Limit of irrigation derable distance, the expense of conveying the large body of water ting several miles in required for the irrigation of the Delta would be very great. But the fact of the country to be irrigated being liable to periodical inundation by the river from a remote period, implies that the deposits during a series of years have raised the land along the banks to a higher level than that at a distance from them, so that the deep cutting at the head of the main channels, works out into a moderate and inexpensive cutting in the course of a few miles.
11. Sir A. Cotton in one of his early reports on the Godavery, Slope of the country thus describes the peculiarities I have mentioned:-"Besides the foom ther band of thesribed slope of the land towards the sea in a Delta, it has another slope, ${ }^{\text {by }}$ Professionaton ${ }^{\text {Papers }}$ Mad viz., a fall from the river in a direction perpendicular to its course, Vol. Il., p. 20 . and the fall is much more rapid than that towards the sea. In the present case it has been ascertained to be, near the head of the Delta, 16 feet in two miles from the west, and 7 feet in two and a half miles on the east side. Thirteen miles lower down, that is twenty-

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five niles from the sea, the fall is 9 feet in two and a half miles on the west side of the Godavery. Thus the river banks form a ridge from 18 to $\mathbf{7}$ feet above the level of the land, at the distance of from three quarters to two and a half miles distant on either side, providing most remarkably for the leading out of the water upon the lands.
" The apparently formidable operation of bringing the water from the bed of the river is upon an examination of the level reduced to this, that the highest part of the Delta is only 8 or 10 feet above the bed of the river in its immediate neighbourhood, that is within two miles of it, and that if an anicut be built 11 feet high above the deep channel of the river, the deepest excavation for the irrigating channels will be 18 feet, and within two miles, the country on the west side would be below the level of the top of the anicut. On the east side the lands would be on the same level within about four miles. The apparent objection arising from the great depth of the river is thus completely disposed of."

Ganges canal had to be carried across a number of torrents, or in a very long cut-
ting, before the irriting, before the irrigation li
reached.
12. On the Ganges canal the water before reaching the tract of country requiring irrigation, had to be carried across a series of formidable torrents, which required a vast expenditure of time and money. Had the canal been opened from the river below the point where the last of the torrents joins it, it would have had to traverse a distance of 50 or 60 miles before the irrigation limit could be attained. In either case heavy expenditure was necessarily entailed before the water could be turned to any use.

Heads of main channels in the Deltas, simple and inexpensive works, and the
length of distribution length of distribution
channels 80 or 40 miles on the average. may be distributed to every field in advance, to near the sea, and the distribution channels have not to extend beyond an average distance of $\mathbf{3 0}$ or 40 miles.
Ganges canal necessarily of great length.
14. On the Ganges canal the water is conveyed over much more unfavorable ground to a distance of 350 miles from the head. Sir A. Cotton considers this fact, as one of the errors of the original project. The practicability of forming separate heads between Roorkee and Cawnpore in order to reduce the distance to which water is conveyed, without being utilized, forms the subject of separate enquiry. 1 may remark in this place, that the principal object of the Ganges canal was to ameliorate a famine, and with this object the
Vide Sir P. Cautley's Report on Central Doab Canal, 1840. of country than was economically necessary. Had it not been for the restriction thus laid on the projector, he could have utilized the whole of the available supply of water in a canal of one-half or one.third the length to which it has been actually carried, and would have had the opportunity of effecting a large saving in the cost of the work.
15. There are several other facts which serve to explain in

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some measure the high and quick returns yielded by the Godavery and Kistna works. There is an enormous extent of waste land in the Deltas: the great mass of it is either sandy or more or less swamp, but large tracts not far removed from the sea and recently inundated by it are unfit for cultivation until the soil is improved. Both this and the sandy soil, however, become as valuable as any other land in the Deltas, after several floodings by the river water, loaded as it is with mud of the most fertilizing character.
16. Large tracts are thus rendered productive, which in their natural state were absolutely useless. A further extent of country is brought within the influence of the Delta channels by embanking or draining swamps. It is a common occurrence for 1,000 or even 2,000 acres of such waste land to be taken up in one plot for rice cultivation in a single season; and there is one instance in the Kistna Delta, of the Ryots of a number of villages uniting to present an agreement to take up in one block $\mathbf{1 5 , 0 0 0}$ acres of waste land as soon as certain drainage and irrigation channels should be completed, and to pay Government rent for it at the rate of Rs. 6 per acre, or Rs. 90,000 in all per annum.
17. This is no exaggeration, as the land was actually taken up on those terms as fast as the drainage and irrigation works progressed. The canals in the N. W. Provinces have no such advantages. Not only is the area of waste cultivable land in the Doab between the Ganges and Jumna of comparatively small extent, but the revenue settlement extends over a period of 30 years, and the cultivator has to pay no more for irrigating waste land, than the small water-rate which he has to pay for land already under cultivation.
18. But on the other hand, high as the returns have been from the Delta works, they would have been far higher had the works ex- ${ }^{\text {tas }}$ retarded ${ }^{\text {by }}$ inperienced a tithe of the liberality with which the Ganges canal has of funds to complete been treated from first to last. Notwithstanding the Government have received incontestible proofs of great and manifold advantages having accrued, both to themselves and to the country, from the extension of irrigation in the Madras presidency, the works which above all others may be taken as the type of what can be accomplished, when a supply of water can be cheaply distributed, are only half finished.
19. The Government readily sanction the estimates for the various new works and extensions that are submitted to them, but the money to carry them out is not forthcoming. Not even the modest demands of the local officers for a fixed and regular annual allotment of 5 lakhs of rupees per annum for new works on the Godavery and Kistna united, until the Delta system shall have been fully developed, has been complied with. Channels which may have been in progress in one year, are summarily stopped the next, or if the main channels are completed, the funds required to carry out the minor works, and to turn the others to profitable account may not be granted,
though the works themselves have received the complete approval of Government. Numerous instances could be adduced in which the delay that has thus arisen in utilizing the supply of water, has occa-. sioned a large loss of revenue.
A notice of the grants for expenditure necessary to complete the comparison between Delta
works and Ganges works and Ganges canal.
20. I have described the advantages which the Delia works had at starting over the Ganges canal, and to render the comparison a fair one, I think myself bound to take conspicuous notice of this one great disadvantage which they have had to contend against for a long succession of years.
Drainage channels used for irrigation in
21. Many of the drainage channels in the Godavery and Kistna Deltas, but not in have been used for carrying the water for irrigation. In the coun-
Ganges canal. try affected by the Ganges canal the drainage courses are deep, and nothing would be gained by using them as irrigation channels. Had they been shallow, the local Engineers would probably still have avoided them, and would have preferred to go to the expense of excavating new channels rather than interfere with the proper function of the drainages. Allowing that there are serious disadvantages in using channels for both purposes, there can be no doubt, that the Engineers in the Godavery and Kistna have secured a large additional revenue by being content to use imperfect channels, when time and money would have been required for the excavation of new ones.
Swamping in Deltrs not unhealthy, as it is in the Doab.
22. The slight fall of many portions of the Deltas, combined with the system of using the natural channels for purposes of irrigation, serves to produce extensive swamping; notwithstanding this, it is a remarkable fact, that the Deltas are more healthy than other parts of the district. Fever especially is far less prevalent in the Deltas than in the country immediately beyond it, where there is but little rice cultivation and no swamps. The cause is unknown to me. It can hardly be the influence of the sea air, bccause the formation of the east coast of India closely resembles that of other countries which are notoriously unhealthy. It is more likely I think to be in the geological formation of the soil. However that may be, it would be useless to attempt to prove that ill-drained rice cultivation in the N. W. Provinces should be healthy because it is healthy in the Madras coast districts.
Few bridges across the Delta charnels. canals. In most cases bridges are built over the locks, but on several of the channels there are no bridges for 30 miles and upwards. On the Ganges canal the bridges are built at every two or three miles apart. They have not been constructed in such profusion simply because the Engineers thought them necessary or wished to construct them, but because the Local Government, acting as they would act towards a private company, insisted on having them where communications were intersected.
small grants of 24. Sir A. Cotton's argument in favor of the procedure which

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has been followed in Madrus seems a sound one. Doubtless, in some money considered to parts of the Deltas, considerable inconvenience is occasioned by the tending irigation. want of bridges, but if only a limited sum was available for expenditure, it was best that it should be used to extend the irrigation.
25. The actual want of bridges is not so great in the Deltas as

Bridges required in in the N. W. Provinces, for the nature of the soil and absence of suita- Diaat.
ble material are almost prohibitory to the formation of roads, which should be passable in the rains. Indeed, there are no metalled roads in the Deltas, but the numerous navigable canals supply their place along the principal lines of traffic, and any other traffic is unimportant. In the Ganges and Jumna Doab, there are greater facilities for moving carts; the extent of thickly populated country is much greater; and there is a far higher proportion of important towns and villages, than are to be found in the Godavery and Kistna. Hence, more cross communication is necessary, and we may reasonably expect that bridges at short intervals will be looked upon as a necessary addition to the canals. I may add that the canals in the N. W. Provinces, are rarely closed unless for emergent repairs, when some sacrifice of revenue is likely to be entailed; and that bridges can be built at a considerably less cost in the first instance before water is admitted, than would be possible afterwards.
26. Thus it appears that a considerable expenditure on account of bridges has to be borne by the Ganges canal, while the Delta channels in Madras are relieved up to this time of any heavy charge on the same account.
27. In the Godavery and Kistna channels navigation and irrigation can be carried on together more favorably than is possible on the canals in the N. W. Provinces. The principal crop in the Deltas is rice, which requires water from July to December. There is also sugar-cane and a second crop of rice in the Godavery, but the area and quantity of water consumed by them is small compared with the requirements of the others. The channels are aligned with a slight fall, generally from 3 to 6 inches a mile, and locks are placed at such intervals as will allow of still water navigation, when the water is not required for irrigation.
28. The surface fall of the channels does not necessarily correspond with the fall of the bed. For three or four months in the year, July to October, it may be increased to 9 inches per mile. The velocity velocity, especially in the upper reaches, is then very considerable, the Doab. and boats cannot work up stream without some difficulty. But for the remaining eight months of the year, a smaller body of water is admitted from the river, and for half that period there is practically still water navigation. On the Ganges canal, on the other hand, the principal demand for water is not during the rains, when the river could supply any quantity that might be required, but during the dry season. Rice is the great staple produce in the South of Ind:a, wheat

Navigation combined with irrigation in Delta channela, more farorably than is po-
sible in N. W. Provinces.

Considerable outlay saved to Delta works.


#### Abstract

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that in the N. W. Provinces. The one is raised in the rains, the other in the dry seas.on. The wheat crop on the Jumna canals is greater than all the rain crops united. These canals have been in. operation for many years, and rice cultivation has in no way been discouraged, unless near cantonments and large towns; yet it has not extended to such a degree as to require a greater supply of water than the wheat. The following figures which are taken from the last report (for 1864-65) of the Chief Engineer, Irrigation Department, Punjab, serve to show the state of the Irrigation under the Western Jumna canal.

Total number of acres irrigated during 1864-65, 434,965.
Area in acres of the principal crops irrigated for the last five year:-

| Rice, .. | .. .. .. |  | $\begin{array}{\|c\|} \hline 1860-61 . \\ \text { (Famine year.) } \\ 44,96.5 \end{array}$ | $\begin{array}{r} 1861-62 . \\ 58,578 \end{array}$ | $\begin{array}{r} 1862-63 . \\ 57,925 \end{array}$ | 1863-64. <br> 47,353 | 1864-65.57,157 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |
| Cotton, | .. .. | -• | 43,706 | 38,558 | 25,549 | 45,882 | 77,738 |
| Sugar, <br> Wheat, | .. .. |  | 26,102 | 38,782 | 44,730 | 30,089 | 29,786 |
|  | .. .. |  | 1,81,208 | 1,48,317 | 1,11,129 | 1,45,234 | 1,63,159 |
|  |  |  | 2,95,971 | 2,74,235 | 2,39,333 | 2,68,558 | 3,27,840 |

The rice and cotton are rain, or "Khureef" crops. Wheat, dry weather crop, or "Rubbee:" the sugar is irrigated in both seasons.

Discharge of Western Jumna canal during each month.
29. The average monthly discharge of the canal was 1,784 cubic feet per second; 243 acres were therefore irrigated in 1864-65 by each cubic foot per second. The following was the discharge during the different months:-


Demand for water as great in dry season $s s$ in rains.
30. From the above it appears that the demand for water in April was as great as in July, and in December and March as in August, though July and August are months in which rice requires a plentiful supply of water.
31. Instead therefore of the demand for water being fluctuating water narigation.

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the year, an exceptionally high supply is not wanted owing to the rains; on the contrary the maximum supply required is that yielded by the river during the dry season, and the current will have to be ckept up at its maximum during the whole of the period, which on the Godavery and Kistna is available for still or nearly still water navigation.

J. C. ANDERSON, Lieut.-Colonel, Royal Engineers.

## A PPENDIX D.

Note on the deep cutting of the Ganges canal, and the supposed effects of irrigation in the salubrity of the N.W. Provinces.

Deep cutting of the canal imputed as the second fundamental mistake.

The effect considered to be exagger ated.

1. The second of the fundamental mistakes imputed by Sir Arthur Cotton to the original construction of the Ganges canal is, that the whole of it " has been cut so as to carry the water below the level of the surface, entailing a vast unnecessary expenditure, and keeping the water below the level at which it is required for irrigation; also that in cutting the canal deep in order to prevent the water keeping the neighbouring lands in a wet state, its Engineers produced the very effect they intended to prevent, because they cut through the watertight stratum, and let the water into the lands below, which carry it all through the country."
2. There can be no doubt that a considerable saving might have been effected by carrying the canal on a higher level and keeping the surface above soil, but as the heads of the rajbuhas or main distributing channels are governed by the levels of the weirs or falls, there does not appear to be any difficulty (except in the Etawah branch) in regard to the irrigation, which commences within 19 miles from the canal head.
3. With respect to the evils supposed to be caused by the percolation of the canal water through the sandy soil below the harder upper crust, it seems that this effect has been exaggerated, as no evidence of it appeared along that part of the canal inspected by the Committee; the only appearance of ooze being where the level is high nor was any information received on the subject ; but supposing it to be partly true, it cannot possibly extend "all through the country,"
Probable results of nor can it cause the formation of swamps likely to influence the saluthe spring level brity of the district. The general effect of extensive irrigation will no doubt eventually be to raise the spring level of the tract irrigated, but this will not depend on the surface level of the main canal.
4. On the other hand, there does seem to be no small risk in

Risk of carrying the canal on a high level. arrying a large body of water at a considerable elevation for a great distance between embankments, composed of such poor materials as even the best soil of these districts, and as it is very doubtful if it was possible to ensure throughout an impermeable bed, it seems that the deeper cutting although more expensive was the safest plan.

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5. In the Delta of the Cauvery the annual rise of the rivers $\begin{gathered}\text { Rive of pringo in } \\ \text { the } \\ \text { Canvery } \\ \text { Delta }\end{gathered}$ above the level of the country over which they spread is invariably floods cont on the followed by a corresponding appearance of spring water on the adjacent lands, and constant vigilance is required to secure the embankments.
6. But it is remarkable that although that Province is more or less inundated for half the year, and extensive swamps are formed

Malaria unknown in Tanjore, bat com-
 along its southern coast, malaria is unknown; a fact as little capable in the N. W. Proof explanation as the existence of the evil on the rocky uplands of Coimbatore and Mysore, and especially along the banks of the same river Cauvery ; and no inference can be fairly drawn from the salubrity of the Southern Deltas and other irrigated districts, to invalidate the common belief in the malaria caused by irrigation in the N.W. Provinces.

## E. LAWFORD.

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## APPENDIX E.

Note on the section of the proposed weirs across the Ganges and Jumna.

This section being avowedly prepared more in conformity with that of the Godavery anicut, than those of weirs in Tanjore and other Southern districts of Madras, I am desirous of explaining my own views on the principal points in which the proposed work differs from the latter.

1st. The depth of the wells. The Madras wells are sunk only 10 feet below the bed of the river, not merely because they are as firm at that depth as if they were carried lower, provided that no scour removes the sand, but on account of the great difficulty and expense of increasing their depth. Experience has however shown that without an almost unlimited supply of rough stone for their protection, it is not safe to rely on this depth, even in the firm coarse sands of the Southern rivers; much less then would it be prudent to do so in the beds of the Ganges and Jumna, consisting of remarkably fine micaceous sand, totally unlike the former, and sinking when wetted under the slightest pressure.
$2 n d$. The breadth of the cut stone apron. This has been fixed at 50 feet, for a dam 9 feet, or with its upper board 12 feet high, and may seem excessive compared to those of the South, which seldom do if ever exceed three or four times the height of the weir. I, however, concur in the proposed plan, because I believe it will often be found impossible to remove the board in time, and then the height of overfall will closely approximate to the same proportion to the apron as in the Southern rivers. This difficulty has been felt at the Godavery anicut. . Further, as there is no doubt that the beds of the rivers will become raised even above the height of the weirs, the overfall will in time be extended beyond its first distance, and may act on the rough apron with too great force.

3 rd . The rough concrete apron is carried out 120 feet, with 7 feet of depth at the inner, and 3 feet at the outer, edge. With a cut stone apron of 50 feet, I cannot think such a broad mass of rough material beyond will at first be necessary, but considering the nature of the sand already described, and the inferior kind of material of which this part of the work must necessarily be formed, I have no doubt that the aggregate quantity will be required in a year or two, and it ought certainly to be provided for in the estimates.
E. LAWFORD.

## A PPENDIX F.

ESTIMATE, No. 1, of the probable expense of a channel from the Ganges at Raoli Ghat to join the Ganges Canal, at $116_{3}^{2}$ miles from Hurdwar.

Width, 180 feet, side slopes, 2 to 1 ; depth of water, 10 feet; embankments, 12 feet wide at crest; 5 feet high above water surface; side slopes, 2 to 1 ; fall, $5 \frac{1}{9}$ inches per mile; total length, 70 miles.

1. 1 to 9 th mile. Depth of cutting varying from 3.5 to 11.5 feet; contents, $81,404,400$ cubic feet, at 3 Rs. per 1000,

## Rupres.

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2,44,213•2
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2. 10 to 16 th mile. Embankment from 9.5 to 13.75 high; contents, 31,159,020, at Rs. 5 per 1000,
$1,55,799 \cdot 6$
3. 17 to $29 \frac{3}{8}$ th mile, $10 \frac{5}{8}$ miles. Embankment, average height $19 \cdot 4$ feet ; contents, $55,287,672$ cubic feet, at Rs. 5 per 1000,

2,76,438-4
Concrete wall in centre of bank, at Rs. 6 per running foot, .. $3,36,600 \cdot 0$
$2 \frac{3}{4}$ miles cutting, average depth 8.3 feet; contents, $23,693,243$, at Rs. 3 per 1000,

71,079•7
4. $29 \frac{3}{3}$ mile to 52 nd mile. Berms 12 feet wide, three above water; cutting, $10 \frac{5}{8}$ mile, from $25 \cdot 2$ to $36 \cdot 4$ feet ; contents, $418,532,664$ cubic feet, at Rs. 5 per 1000,

20,92,663•3
Cutting for 11 miles, from 15 feet to 24.3 feet; contents, $\mathbf{2 6 0 , 9 7 3 , 5 0 4}$ cubic feet, at Rs. 4 per 1000,
$10,43,894 \cdot 0$
Embankment across Kala Nuddee, width of crest 20 feet; height $14 \cdot 6$; contents $\mathbf{7 0 , 9 6 , 3 2 0}$, at Rs. 8 per 1000,
.. ..
56,770•5
5. 53 to 70th mile. Cutting from 5 to 14 feet ; contents, $172,666,560$, at Rs. 3 per 1000, . . .. .. .. .. .. 5,17,999.7

## Abstract.

| 1 to 9th mile, |  | Rupess. <br> 2,44,213.2 |
| :---: | :---: | :---: |
| 10 to 16th mile, |  | 1,55,799.6 |
| 17 to 2983 ${ }^{\text {th }}$ mile, |  | 6,84,118.1 |
| $29 \frac{3}{8}$ to 52nd mile, |  | $\left\{\begin{array}{l} 20,92,663 \cdot 3 \\ 10,43,894 \cdot 0 \end{array}\right.$ |
| Embanking Kala Nuddee |  | 56,770-5 |
| 53 to 70th mile, | .. $\cdot$ | 5,17,999.7 |
| Masonry works, 70 miles, at Rs. 15,000 per mile, |  | 47,95,458•4 |
|  |  | 10,50,000 |
|  |  | 58,45,458.4 |
| Contingencies, at 8 per cent., |  | 4,67,636.6 |
|  |  | 63,13,095.0 |
| Establishment, at 7 per cent., <br> Land, $\mathbf{7 0}$ acres per mile, for 70 miles, at Rs. 24, |  | 4,41,916.0 |
|  |  | 1,17,600.0 |
|  |  | 68,72,611.0 |

ESTIMATE, No. 2, of the probable cost of constructing a weir across the Ganges at Raoli Ghat, $4 \frac{1}{2}$ miles below Sookertal.


## II. Wing Walls.

2. Walls, $25 \times \frac{3+10}{2}=162.5$, at Rs. 30, $\quad=48.75$

Wells, $2 \times 6 \times 15=180$, at Rs. 60, $\quad=108.00$ Concrete apron, in front, $30 \times 7=210$, at Rs. $20=42.00$

Total per foot run, . . . 198.75
Total for wing walls, $198 \cdot 75 \times 1,600=3,18,000$
3. Concrete blocks along foot of embankment through Khadir,

$$
5280 \times 7 \times 30=11,08,800, \text { at Rs. } 20 \text { per } 100, \quad 2,21,760
$$

## Abstract.

Rupese.

1. Weir, .. .. .. .. .. .. .. 29,10,200
2. Wing walls, .. .. .. .. .. .. 3,18,000
3. Concrete blocks along embankment, .. 2,21,760
4. Head sluice and under sluices, .. .. 2,50,000 Embankment, .. .. .. .. .. $\mathbf{5 0 , 0 0 0}$

| Add 10 per cent. for contingences, .. | 37,49,960 |
| :---: | :---: |
|  | 3,74,996 |
|  | 41,24,956 |
| Establishment, at 7 per cent, .. | 2,88,747 |
|  | 44,13,703 |

Note.- 50 per cent. has been added to ordinary rates for work to be executed in water, to cover cost of temporary embankments, baling, losses from flood, \&c., \&c. An exceptionally high rate is allowed for well foundations, owing to the depth of their hoads below low water.

## ESTIMATE, No. 3. Weir and Works, near Rajghat. <br> Channel.



Wells $7 \times 7 \times \mathbf{7 8 5 4} \times \mathbf{3} \times \mathbf{3 0}=\mathbf{4 5 0}$, at Rs. 30 per 100 cubic feet $=$ Rs.
Sinking and curbs for ditto, Rs. .. .. .. .. $\begin{array}{r}\text { 135-0 }\end{array}$
Bhurtpore cut stone apron, $30 \times 2=60$
Weir face, $\quad . \quad 6 \times 3=18$
Coping, .. .. $5 \times 3=15$
93, at Rs. 2-8 $=232-8$
Best block kunker, dressed, $20 \times 1^{\prime} \cdot 9^{\prime \prime}=35$, at Rs. $1=35-0$
Second $\quad " \quad 10 \times 1^{\prime} \cdot 6^{\prime \prime}=15$, at Rs. $0-12=11-4$
Rubble masonry under apron, $60 \times 6=360$
, body of weir, $\quad 9 \times 8=72$

$$
432 \text { at } \frac{30}{100}=130-0
$$

Dry rubble, down-stream, $\quad 90 \times 4=360$



Total for the whole work, .. . . 1,13,04,270

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( xxix )
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ESTIMATE, No. 4. Weir and works on the Jumna, for a project to carry 3,300 cubic feet per second.

## Channel.

13,24,22,400 Excavation in 10 feet cutting, at Rs. 3, 3,97,267
$25,87,20,000 \quad$, $14 \quad$. $3,7,76,160$

28,53,66,080 " 23 , " 4, 11,45,464'
Bridges 19, at Rs. 2,000, .. .. .. .. 3,80,000

Contingencies, at 8 per cent., $\quad \cdots \frac{2,27,991}{\mathbf{3 0 , 7 7 , 8 8 2}}$
Superintendence, at 7 per cent., .. $\frac{\mathbf{2 , 1 5 , 4 5 1}}{\mathbf{3 2 , 9 3 , 3 3 3}}$

Weir, per foot run-
Cut stone apron, .. .. $50 \times 1 \frac{1}{2}=75$
Do. front of weir, .. .. $7 \times 2=14$
Do. coping, .. .. .. 6.5 $\times 2=13$
$\overline{10} \overline{2}$ at Rs. $0-12=$ 76-8
$\begin{aligned} & \text { Wells, including sinking and curbs, 8ce., } 3 \times 6 \times 15=270, \\ & \text { at Rs. } 60 \text { per } 100, \ldots\end{aligned} . . \quad . . \quad . \quad . \quad 162-0$
Rubble rough stone, down-stream, $120 \times 5=600$


Length of weir 2,500, at Rs. 347 per foot, .. 8,67,500
Moveable board and post, at Rs. 5 per foot run, . . 12,500
Sluices and regulators, .. .. .. .. 1,50,000
Wing walls and two locks, .. .. .. .. 1,50,000
Training and protecting banks, .. .. .. $\frac{50,000}{1230,000}$
Contingencies, at 10 per cent., $\begin{array}{r}\text {.. } \\ \text { Total, } 13,53,000 \\ 13,53,000\end{array}$
Superintendence, at 7 per cent., .. 94,000
Total, .. .. $\overline{14,47,000}$
$\left.\begin{array}{l}\text { Weir and passage over Hindun, including } \\ \text { training and protecting walls, .. }\end{array}\right\} \mathbf{4 4 0} \quad \mathbf{2 , 5 0 , 0 0 0} \quad 16,97,000$
Total Rupees, .. .. $\overline{\mathbf{5 0 , 7 7 , 8 8 5}}$
Say, Rs. $\mathbf{5 1 , 0 0 , 0 0 0}$.

ESTIMATE, No. 5. Weir and works on the Jumna. Estimate for project to carry 1,500 feet per second.

Channel.


Total for weir over Jumna and Hindun, as in preceding \} estimate,

Total for project, . .. 35,65,684



Sour



[^0]:    Alterations to main line 81. If the alternative line be adopted, the following addiif alternative line be adopt- tions and repairs will be necessary to the adjoining portions ed. of the existing line, and have been estimated accordingly.

    Bars of earth to be thrown across the present bed at intervals of 500 feet, wherever deep channelling has taken place; the top of each flush with the original level of bed, and 10 feet in width; the spaces between these being left to silt up gradually by the deposit of sediment from the Canal water.

    The towpath on the left bank, from Roorkee to the Futtehgurh Branch head, and again from Jutpoora Bridge to the head of the Cawnpore and Etawah branches, 131 miles in all,

[^1]:    * November 186٪.

[^2]:    Width of Ganges at Sookertal.

[^3]:    * The Committee have been informed that the following large weirs at Madras have been breached at different times, viz:-Upper Coleroon anicut, in 1836 and 1859. Lower ditto, 1837, 1862, 1863, 1864, and 1865. Pennair anicat, 1858 and 1859. Vellaur anicat, 1853. Godavery anicut-(year unknown).

[^4]:    Compared with cost of Anicat across Kistna at Bezwada.
    Professional Papers Madras Engineers, Vol. IV., p. 70.

[^5]:    * For the year 1864-65, the average sapply admitted into the canal was 4,026 crbic feet per second. An observation taken at Gurmuktesur, 50 miles below Sookertal, on 5th February, 1865, makes the discharge of the Ganges only 4,278 cubic feet per second ; total 8,304 cubic fect.

[^6]:    Delay in applying

[^7]:    * The Committce applied to Colonel Dyas for an estimate of the loss, but have been disappointed in not receiving it. Since the Committees' Report was drawn out, Colonel Dyas has written demi-officially as follows:-"I have been disappointed about the information I wrote for to enable me to estimate the loss that would be incurred by closing the canal in March, April and May, and I fear when it does come it will be too late to be of ase to the Committce, so I think you had better go upon the estimate you drew up, and which is founded on the latest information I have. I should be very mach obliged by your sending me a copy of it. I will write again if the information I have written for comes soon; bat don't wait for it."
    $\dagger$ The Committee observe, vide Revenue Report, dated 4th March, 1864, that the loss sustained daring a three months closure in 1862-63, amounted to Rs, 2,25,689. The closure was necessitated by an injary to one of the falls, and tosk place at an unfortunate period of the year, while the Rabbee irrigation was at its height.
    $\ddagger \mathrm{All}$ of these works are on well foundations.
    § Artificial, i. e., aided by machinery.
    II Natural flow, i. e., by gravitation.
    I Average depth of excavation in Cawnpore branch is 4.82 fect for the first 120 miles ; average depth in Etawah branch for the same length is 6.65 feet. The water is 6 feat deep in both cases.

[^8]:[^9]:    - Appendix B and C.

[^10]:    Edward Lawrord, Col.-Commandant, R E.
    J. C. Anderson, Lieut.-Colonel, R.E.
    J. G. Fife, Lieut.-Colonel, R.F. Hugh Lionard, C.E.

[^11]:    * A doubt has since been thrown as to whether this was lowest minimum discharge, by its baving peen stated that the discharge of the Jumna on the 19th December, 1865, was only 829 feet per socoad.

[^12]:    - It might prove advisable to bring in the new channel below the Sirma falls, 5 miles lower down, or about the 163rd mile.

