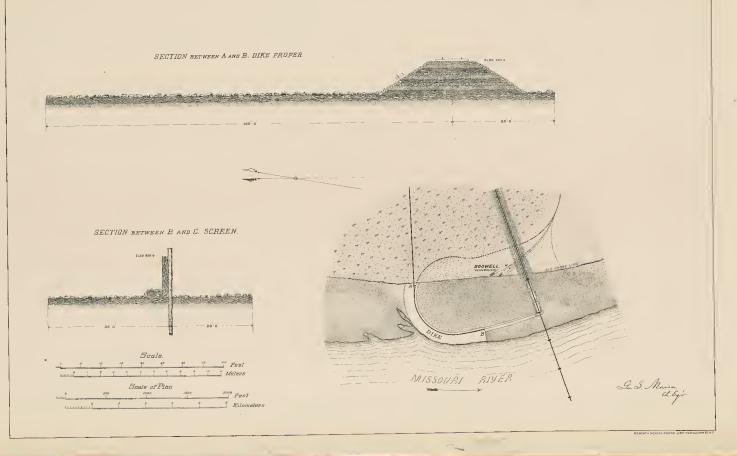
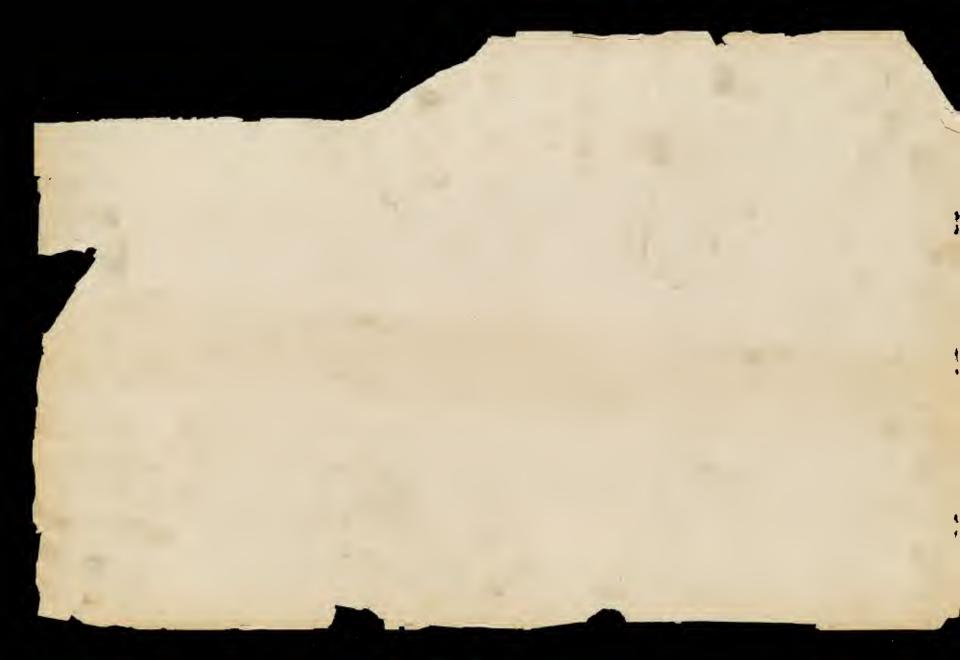
c.e.zq.r.r. RULO BRIDGE

DETAILS OF DIKE





GEO. S. MORISON,

Chief Engineer.

The Rookery Chicago.

THE RULO BRIDGE.

A REPORT

To CHARLES E. PERKINS, President of the Chicago, Burlington & Quincy Railroad,

3Y

GEORGE S. MORISON, Chief Engineer of the Rulo Bridge.

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CHICAGO, MAY 1, 1890.

CHARLES E. PERKINS, Esq.,

President Chicago, Burlington & Quincy Railroad.

Dear Sir:-

I submit the following final report in relation to the construction of the bridge across the Missouri river at Rulo, Nebraska.

Yours truly,

GEO. S. MORISON,

Chief Engineer Rulo Bridge.

THE RULO BRIDGE.

T.

PRELIMINARY NARRATIVE.

Before the completion of the Plattsmouth Bridge in 1880, the importance of a lower crossing of the Missouri to accommodate the Southern lines of your Nebraska system of railroad, became apparent, and I was instructed by you to make an examination with a view to locating such a bridge near the month of the Little Neniaha. This examination was made in the season of 1880-'81 and resulted in the selection of a location near the little village of Aspinwall about two miles below the mouth of the Little Nemaha. A bridge built here would have made a direct connection between the railroad leading westerly from Nemaha City to Beatrice and the branch on the east side of the river from Corning to Villisca.

Subsequently the acquisition of the Hannibal & St. Joseph R. R. by your company made a more southerly crossing desirable and the fact that the Atchison & Nebraska R. R. (a portion of your Nebraska System) followed down the valley of the Great Nemaha indicated that a location at Rulo near the mouth of the Great Nemaha would have decided commercial advantages over the Aspinwall location.

The season of 1883-'84 was therefore devoted to examinations and surveys in the neighborhood of the mouth of the Great Nemaha.

The beginning of the work may be fixed as September 13th, 1883, when Mr. B. L. Crosby the Resident Engineer arrived at Rnio. The location at Rulo was fixed by me and borings were actually begun one week after Mr. Crosby's arrival.

These borings showed a state of affairs quite unlike that usually found in

the Missouri River, there being no rock within any reasonable depth, but a stiff bed of blue clay, of an average thickness of about 15 feet was found under the alluvial sand, this clay resting on a bed of coarse sand and gravel of varying thickness, which itself rested on a bed of clay, the surface of which was nearly level and which from its stratified character was found to be more truly a shale than a clay. These borings showed that though the bridge when built would be of a satisfactory character, the cost of the foundations would be exceptionally large. It was evidently expedient to examine other points in the same neighborhood.

Borings were accordingly made in the spring and summer of 1884 at White Cloud to miles below Rulo and at Arago to miles above Rulo, these being the nearest points at which the general topography of the country indicated that the construction of a bridge would be feasible. The borings at White Cloud gave better results than at Rulo, as rock bottom was reached, but at Arago, on the east-side of the river, hard material was found only at a depth of 123 feet from top of sand bar, this material being a very soft sand stone. The difficulties in approaching a bridge at either of these points were so great that Rulo was selected as decidedly the best location.

The width of the river at ordinary high water stage at Rulo was about 1600 feet, the channel being next to the west shore. This width being greater than the width required to pass the river, it was determined to reduce this width to about 1100 feet by the construction of a dike above the bridge. Construction of this dike was authorized in October 1884, work was begun on the track leading to this dike October 22d; the dike itself was begun on the 4th of December and completed May 7th, 1885, and an extension in the form of a permeable screen made in the following May and June.

Authority for the construction of the bridge was obtained from the general government in 1884 by an Act which became a law June 18th, 1884. This Act is printed in full in Appendix B.

The location of the bridge had been definitely fixed at the time that Rulo was selected in preference to any other place. The character of the bridge was, however, not fixed at this time. It was evident to me from the beginning that the only proper structure was a high bridge without a draw, the western approach to which would run nearly due west connecting with the Atchison & Nebraska Railroad in the Nemaha valley. In this opinion I had the hearty support of Mr. R. J. McClure, Chief Engineer of the Chicago, Burlington & Quincy R. R. and Mr. J. F. Barnard, then General Manager of the Hannibal & St. Joseph R. R. who really had been the first to call attention to the merits of Rulo as a place for crossing the Missouri River.

There was, on the other hand, a decided demand by some of the operating officials of the company for a low bridge, the west approach to connect with the old track of the Atchison & Nebraska R. R. in front of the town of Rulo, which ran south along the Missouri bottom till it reached the Nemaha valley. The merits of a high bridge scheme was its simplicity, a less cost of maintenance of the bridge and the fact that it shortened the through distance two miles. The only advantage of the low bridge scheme was that it avoided the deep cut west of Rulo, and a careful estimate showed that a low bridge would be the more expensive of the two. The difference in opinion prevented an early determination of the plan of bridge and did much to render the cost of real estate on the west side of the river unreasonably large.

In 1885 and before the character of bridge had been determined, I asked for authority to put in the foundation of Pier I (the eastern pier) with a view of determining more fully the character of the material on which the piers would rest. This authority was granted and work was actually begun December 3, 1885, this foundation being finished in the following April.

The character of the bridge was finally determined and the plans were submitted to the Secretary of War for approval July 19, 1886. No effort was made to get this approval quickly, and it was not finally received until February 25, 1887, work, however, had meanwhile been in progress.

In May 1886 authority was given to continue the construction of the bridge in earnest and the work was prosecuted from this time forward without delay under the direct charge of Mr. B. L. Crosby as Resident Engineer.

The winter of 1886-87 was unfavorable for work, it being one in which the ice formed and broke up several times, this causing some delay and increasing the cost of the work.

The last span of the bridge was swung September 24, 1887, and on the afternoon of October 2d the first locomotive crossed the bridge, and it was opened to traffic immediately thereafter.

The great cut on the west approach to the bridge was, however, not yet completed, and for nearly two years the traffic crossing the bridge was taken over the old line between Rulo and Rulo Y.

On June 3, 1889 the excavation for the great cut was completed, though the track remained to be laid and much ditching to be done; on July 14th the first train passed through the great cut. The ballasting of the track through the great cut was completed September 8th.

On November 1, 1889, the bridge with its approaches was turned over to the operating department as a completed structure.

II.

GENERAL DESCRIPTION.

The Rulo bridge is a single track railroad bridge. It consists of three channel spans each 375 feet long between centers of end pins, resting on four piers of granite masonry (numbered from east to west), at each end of which are three 125 feet deck spans, the spans being separated by iron towers 25 feet long, making the length of the iron structure at each end of the channel spans 425 feet. The end pins are placed 4 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers II and III.

The bridge is built on a grade of 0.4 per cent. (21.12 ft. per mile) ascending westward. The clearance at the center of the east span was 53 feet above the water of April 14, 1884, and that at the center of the west span 36 feet above the same high water, this stage of high water being about six feet below the highest water observed except that of 1881; these clearances were both accepted in the approval of the plans by the Secretary of War. The actual clearances above the extraordinary flood of 1881, so far as this can be determined, are 43.5 and 46.5 feet respectively. Since the construction of this bridge, a Standard High Water has been established by the Missouri River Commission at this place (circular of April 24, 1889); the clearances above this high water are 50.8 and 47.8 feet respectively.

The east approach is 15.230 feet long from a connection with the track built by the St. Joseph & Nebraska R. R., in Section 36, T. 61 N., R. 40 W., to the end of the iron work, the maximum grade on this approach being 0.5 per cent. (26.4 ft. per mile.)

The west approach is 19,260 feet long from the west end of the iron work to the connection with the Atchison & Nebraska R. R., the maximum grades being 0.4 per cent. (21,12 ft. per mile) in each direction, excepting a short piece of one per cent. put in temporarily at the connection.

Besides the two approaches proper, a third approach, called the Atchison connection, was built connecting the west approach with the old line of the Atchison & Nebraska R. R., this connection being 1.36 miles long, with maximum grades of 1.5 per cent. (79.2 ft. per mile).

Pier IV the west pier stands on the edge of the river bank which is 530 feet distant from the base of the bluff, the intermediate ground being a piece of bottom land of about the elevation of high water, but which is composed so largely of the tough soil known as gumbo, that the action of the river upon it is very slow. The only protection required was a quantity of riprap revetment at Pier IV and extending a moderate distance above and below; this riprap must be carefully watched and may require extension sooner or later.

The only protection on the east side is the dike, which was the first work done in the construction of the bridge, and which has acted admirably.

All the levels taken during the construction of the bridge were tied to the benches established by the Missouri River Commission and referred to a tide water datum, this datum being 412.71 feet below the St. Louis City Directrix.

III.

SUBSTRUCTURE.

The substructure comprises the four granite piers which support the channel span and sixteen small cylindrical piers which support the towers which carry the deck spans. The principal piers are numbered from 1 to 1V, Pier 1 beginning at the east end. These four piers are built on pneumatic caissons of the following dimensions:—

Pier II, 53 feet long, 25 feet wide and 18 feet high.
Pier III, 55 '' a a 27 a a b 18 a b
Pier III, 55 '' a 27 a a 18 a B
Pier IV, 53 a a 25 b a a 30 a b

All caissons were built with a side batter of one in twenty-four.

The foundations were put in by the company's own men under the direction of the Resident Engineer. The masonry was built by contract by the firm of Drake & Stratton.

The caisson for Pier 1 was built in a pit excavated on the dry sand bar on the east side of the river. The caissons for Piers 11 and 111 were built in position on pile false work and lowered with screws to the bottom of the river. The caisson for Pier IV was built in a pit excavated in the shore close to the river.

The pneumatic machinery was purchased from the Missouri Valley & Blair Railway and Bridge Co., and was the same machinery that had been used in sinking the foundations of the bridge at Blair crossing. The machinery was first set up on the east side of the river near the site of Pier 1, Subsequently all of the machinery was transferred to the steamer John Bertram, which was purchased from the Sioux City & Pacific R. R., having been built to be used as a car transfer boat at Blair Crossing. This steamer arrived at Rulo May 28, 1886, and when equipped with the full outfit of pneumatic machinery, formed an admirable tool.

The caisson for Pier I was built of pine with an iron cutting edge and planked with two thicknesses of pine plank. The other caissons were built in the same way, except that the iron cutting edges were made heavier and the large sill timbers were of oak.

The caisson for Pier 1 is surmounted by 53 feet of crib work built in three sections and stepped down to 46 feet by 18 feet at the top of the upper section. The caissons for Piers II and III are surmounted by 42 feet of crib work built with the same batter as the caissons, but with the corners cut off so as to make the horizontal section that of an irregular octagon, the crib being sheathed with oak plank and the corners plated with 3-8 inch iron.

The caisson for Pier IV is surmounted by 50 feet of crib work 48 feet by 20 feet, the sides being plumb.

Both caissons and crib work were filled with Portland cement concrete.

The excavation of the pit for Pier I was begun December 3rd, 1885, the framing of the caisson December 14th and the setting up of the cutting edge January 15, 1886.

The caisson was finished February 13th and the concrete filling was beguin February 18th. Air pressure was put on February 20th and on March 19th the caisson reached the clay at clevation 792.1. After sinking about four feet into the clay a test pit was begun March 30th and sunk 19.3 ft. through the first clay into the gravel which separated the upper clay from the lower clay and which at the site of this pier was only 3.4 feet thick. No increase of air pressure was required during the sinking of this test pit until the gravel was reached, and then it became necessary to increase the pressure at once to the full amount corresponding to the actual depth. This test pit showed the upper clay to be a perfectly homogeneous layer on which it was considered safe to found Piers I and IV, while it was thought best to sink Piers II and III to the lower clay. The test pit having served its purpose the excavation was filled up and the sinking of the caisson was continued till April 5th when an elevation of 785.88 was reached. The sealing of the working chamber was begun on the

following day and completed on the 9th of April, thus finishing the first foundation.

No further work was done on the foundation till after the high water season, when the first foundation taken in hand was Pier IV.

The excavation of the pit for Pier IV was begun July 15th, 1886; erection of the cutting edge on July 23rd and the caisson was completed and lowered on the ground August 17th. Concreting was begun August 19th and air put on this caisson August 22nd. The caisson reached the upper clay at elevation 799.2 September 9th. The clay, though identical in character with that found at Pier I, showed signs of having been disturbed. A test pit was sunk in this clay and while it was being sunk a horizontal crack was observed on which the upper portion moved over the lower portion about 11/2 inches in a south easterly direction. It became perfectly evident that it was necessary to sink to lower clay, which was an expensive process. On the 19th of October a leak was discovered in the well leading to the air lock, and further trouble was experienced in the same way three days later, this defect being clearly due to bad workmanship. The caisson finally reached the lower clay on the 20th of October and on the 5th of November sinking was stopped at elevation 765.09 and scaling begun. The scaling was completed on the 8th of November and air let off on the following day.

No masonry was laid on either Pier 1 or IV until the completion of the foundation.

The first work done on Pier III was to drive a pile break water above the pier, which was begun September 1 rth, 1886, and this was immediately followed by the construction of the falsework. The erection of the caisson was begun October 9th and finished November 9th. Shallow water was obtained by sinking brush below the pile protection and thus forming a sand bar at the pier

site. Meanwhile, a winter bridge had been built by the Operating Department about one quarter of a mile below the bridge line, and this bridge caused an ice pack which extended above the bridge line and caused very serious trouble. On the 24th of November the caisson was lowered until it floated on the water. Concreting was begun on the following day, but the trouble with ice prevented rapid progress. On the 28th of November all access to the pier was cut off by ice and a gang of thirty men were imprisoned there till the following morning. These difficulties were over on December 1st and the construction of the crib was begun, on the 4th of December air pressure was put on. The crib was finished January 9, 1887, its concrete filling January 11th, and masonry was begun January 14th. On the 6th of January the caisson had reached the first clay; on February 10th it had passed through the first clay and reached the gravel. The second clay was reached at elevation 766.9 on March 12th. From the 1st of March to March 10th it became necessary to abandon the foundation while the ice went out. On March 18th the foundation was again abandoned just as it was ready for scaling, and during the high water of the next two weeks, the pier was entirely submerged. On the 12th of April air pressure was again put on and it was found that the caisson had settled 1.14 ft. into the clay. In order to fit the courses of masonry it became necessary to sink 1.07 ft. further to elevation 763.53. On the 19th of April the sealing of the working chamber began and the foundation was finished on the 26th.

The first work done on Pier II was on the 5th of January 1887, when the driving of piles for the staging was begun. The erection of the cutting edge was begun on the 14th and the caisson was finished on February 5th; it was lowered on the 10th of February, landed on the 12th and air pressure put on on the 15th, the machinery on the Bertram supplying air for both Piers II and III and

the sinking being continued in this way till the 23rd of February. Air pressure was again put on on the 18th of March; the crib and concrete filling were finished on the 8th of April, and the upper clay was reached on the 10th of April. The pier was abandoned April 12th so as to leave the steamer free to work on Pier 11I; work was resumed April 26th, and was then continued without interruption; the second clay was reached June 4th, the sealing of the working chamber began on the 10th, and this, the last foundation of the Rulo Bridge, was completed June 18th.

The sinking through clay was greatly facilitated by the use of a special air lock with an elevator arrangement attached by which the air pressure in the caisson was made to lift a bucket of clay to a lock above the masonry, which lock was worked entirely from the outside and when opened the bucket was dumped by the outside attendant. This special lock, known among the engineers as a "clay hoist" and among the workmen as a "go-devil" is shown in detail on Plate 10; the movement is precisely the same as that commonly used in hydraulic elevators, except that instead of water pressure, the air from the caisson was made available for power.

The full details of the four piers are given on Plates 4, 5,6 and 7, and of the caissons on Plate 8. The rate of progress is shown graphically on Plate 9, Full records of the progress and details of sinking these foundations were kept and are given in Appendix D. The detail cost is given in Appendix E.

The cost of the four foundations is shown in detail in the table

This cost includes all concrete and other material below the masonry.

In this statement the item of freight charges is simply what is known as "company's freight" being freight on the lines of the C. B. & Q. R. R. system.

THE RULO BRIDGE.

	Cost, exclud- ing Freight Charges.	Freight Charges.	(lost, including	Freight Charge	18,			Cost, exclud- ing Freight Charges	Freight Charges.		Dost, ancluding	Freight Char	ges.
ATION I:							FOUNDATION III:						1	1
CAISSON— Materials Labor . "	\$3,879.25 1,928.17	\$ 569.53	\$ 4,448 78 1,928.17	\$ 6,376.95			CAISSON— Materia Labor		\$3,265,21 4,077.20	\$ 983.55	\$4,248.76 4,077.20	\$ 8,325.96		
CONCRETE FILLING— Materials : Labor	4,044.22 1,536.99	1,153.02	5,197,24 1,536.99	6,734.23			FALSE WO Materia Labor	ıls	785.97 1,249.73	119.91	905.88 1,249.73	2,155.61		
CRIB— Materials Labor	3,935.11 1,264.97	1,121.05	5,056.16 1,264.97	6.321.13	\$13,111.18		CONCRETE Materis Labor	FILLING uls	4.036.01 3,317 58	911.76 —	4.947.77 3.317.58	8,265.35	\$18,746.92	
CONCRETE FILLING— Materials Labor	6,889.20 2,306.13	1,372,85	8,262.05 2,306,13	10,568.18			CRID— Materia Labor	ıls .	3,286,68 2,664,74	994.89	4,281.57 2,664.74	6,946.31	410,740.93	
CUTTING EDGE, AIR-LOCK, SHAFTS, ETC. SINKING—	2,124.96	303.44	2,428.40	-	2,426.40		Conurete Materis Labor	FILLING—	7,253.66 3,884.73	1,274 60	8_528.26 3.884.73	12,412.99		
Materials Labor	1.184.76 9,440.52	297.64	1.482.40 9.440.52		10,922,92			EDGE, AIR-LOCK, SHAFS, ETC.	4,256.63	315 89	4,572.52	_	19,359.30	
ERECTION AND REMOVAL OF MACHINERY .	40.534 13	4,817.53	1,999 85	-	1,999.85	\$45,351.66	SINKING— Malerie Labor	Is ,	2,223.83 19,697 44	537-71	2,761.54 19,697.44		22,458.98	
ATION II:							ERECTION	AND REMOVAL OF MACHINERY						
CAISSON— Materials Labor	3,314.51 3,079.51	1,053.24	4,367.75 3,079.51	7,447.26			FOUNDATION IV.	AND REMOVAL OF MACHINERY	520. 59 60,520.00	5,138.31	520,59	-	520, 59	
FALSE WORK— Materials	1,051.86 981.68	291.12	1,342.98 981.68	2,324.66			2001	ls	3,980.38 2,551.45	806.05	4,786.43 2,551.45	7,337.88		
CONCRETE FILLING— Materials	4,182.06 3,169.65	1,101.70	5,283.76 3,169.65	8,453.41			Labor	FILLING—		550.00	4,642.87 1,917.96	6,560.83		
CR18— Materials	2,648.81 2.349.19	860.87	3,509,68 2,349,19	5,858.87	18.225.33		CRIB— Materia Labor	ls	2,586 37 2,134 96	497.66	3,084.03 2,134.90	5,218.99	13,898.71	
CONCRETE FILLING— Materials	6.371.15 3,896 89	1,600.74	7,971.89 3,896.89	11,868.78			Concrete Materia Labor	FILLING—Is	7,088.43 1,986.97	1,329.03	8,417.46 1,986.97	10,404.43		
CUTTING EDGE, AIR-LOCK, SHAFTS, ETC	3,850.65	344-34	4.194.99		17,727.65 4,194.99		Curring E	DGE, AIR-LOCK, SHAFTS, ETC	3, 155.20	337-53	3,492-73	_	15,623.42 3,492.73	
SINKING— Materials Labor	2,001.54 15,827.67	557.63	3,559.17 15,827.67		18,386,84		SINKING— Materta Labor	Is	1,733-42 15,047.99	373.58	2, 107.00 15,047.99	_	17,154.99	
ERECTION AND REMOVAL OF MACHINERY	\$49.86 \$53,275.03	\$5,809.64	549.86	-	549,86	\$59,084.67	Erection ;	and Removal of Machinery	478.26 46,754.26	3.893.85	478,26	-	478.26	
							G	OST OF FOUR MAIN FOUNDATIONS						\$

It will be observed that the cost of the foundation of Pier III was \$6,574 more than that of Pier II although the quantities of material are almost identical in the two piers, the extra cost being due to the interruption and other difficulties from ice and the incidental troubles connected therewith.

The contract for the masonry was let on July 23rd, 1886 to Geo. S. Field & Co. and transferred by them on the 2nd of August with my consent to the firm of Drake & Stratton. The dimension work is of granite quarried near St. Cloud, Minn; the backing is of Anamosa limestone from Stone City, Jowa.

The specifications for the masonry are given in Appendix C.

The first stone laid was on Pier IV, the masonry of this pier being begun November 27th, 1886 and finished March 25th, 1887. The masonry of Pier III was begun January 14th, 1887 and completed May 19th, 1887. The masonry for Pier II was begun April 12th, 1887, and finished August 10th, 1887. The masonry for Pier I was begun April 19th, 1887, and completed August 9th, 1887.

The cost of masonry is shown in detail in the following table:

		PIE	R 1.			PIE	R II.			PIE	R III.			PIE	R IV.	
Estimate—																1
Masonry laid @ \$27.40 per cubic yard	cu.yds. 561.80	_	_	\$15,393.32	eu. yds. 128, 59	-	_	\$35,233.66	eu. yds, 1.317.00			\$36,084.80	eu.yds. 814.40		_	\$22,314.5
Labor—Replacing Course	_	_	_	_	_	_	_	9.10		_	_	5.30	_		_	_
Cutting and Drilling for Cramps	-	_	_		_	_	_	32.40	_		_	_	_	£ _	_	_
130 lbs. 36 in. Round Iron for Cramps	_	_	_	_	_	_	_	3.90	_	_			_		_	
Freight Charges on Stone	_	_	\$655.09	_	_	_	\$1,599.56		_		\$1,515.69	_	_	_	\$993.62	1 -
Freight Charges on Sand	_	_	126.23	-	_	_	289.93	_	_		295.91			_	182.95	_
	_		_	781.32	_	_		1,889.49	_			1,811.60	_	_		1,176.5
Eagle Cement @ \$2.55 per bbl	bbls.	\$273.75	_	- 1	bbls.	_		! _	bbls.	_	_	_	bbls.	_	_	1 -
Freight Charges	-	100.57	374-32	-	-	_	-	-	_		-	-		-	-	_
Alsen Cement @ \$2.70 per bbl	24	64.80	-	_	202	\$545.40	_	_	428	\$1,155.60	_	_	389	\$1,050.30	_	_
Freight Charges	_	10-34	75-14	-	-	100.43	- 645,83	_	_	254.76		_	_	191.77	1,242.07	9 -
Alsen Cement @ \$2.80 per bbl	26	72.80		_	187	523.60	_	_	_	_	_			_	1 -	_
Freight Charges	_	10.92	83.72	- 1	-	92.97	- 616.57	-	_	-	_	-	_	-	_	_
Alsen Cement @ \$3.35 per bbl	69	224.25		_	5	16.25		_	-	_	_	-	_	_	_	_
Freight Charges	-	9.90	234.15	-	-	.18	16.43	-	_	_	_	-	_	-	-	-
Dyckerhofi Cement @ \$2.72 per bbl	-60	163.20	_	_	233	633.76	_	_	220	598.40	-	_	52	141.44		_
Freight Charges	-	29.77	192.97	- 960 30	-	115.85	749.61	2,028 44	-	130.96	729.36	2,139.72	_	25.63	167.07	- 1,409.1
Total	-	-	-	\$17,134.94	-	_	-	\$39,196-99	_		_	\$40,042.42		_	-	\$24,900.2
Cost per Yard of Masonry	-	-	-	30.50	-	-	_	30.48	_	_	-	30.40	_	-	_	30.
Average Cost per Yard of Masonry	_	Y	_	_	_	_	_	_		_	_	_	<u>:</u>			30.

The total cost of the four piers including the foundations and masonry is given in the following table:

. — 	Cost,	excluding Frei	ght.	F	reight Charges		Cos	t, including Fre	nght,	Gross Volume.	Cost per Cubic Ft.	Cu. Ft. Sunk Area of base X feet sunk)	Cost por Cubic Pt	Vertical Ft. Sunk below Standard Low Water.	Cost per Verneal Foo
FOUNDATION I: Caisson and Filling, Cith and Filling Sinking Caisson Erection and Removal of Machinery	\$13.513.59 14.395.41 10.625.28 } 1,999.85 }	 	= -	\$2,025.99 2,493 90 297.64	= - \$4,817.53	Ξ	\$15 539 58 16,889.31 12,922.77	 \$45.351.66	=	Cubic Feet. 23,093, 16 53,568,12	Cents. 67.2 31+5	760,02	Ceuts-	\$7.36	\$220,10
FOUNDATION II: Caisson and Filling Crib and Filling Shaking Caison Exection and Removal of Machinery	19,629.92 15,266.04 17,829.21 } 549.86 }		=	2,790.40 2,461.61 557.63		Ξ	22,420,32 17,727,65 18,936.70	 59.084.67	Ξ	35,930 31 45,764.09 —	86.4 38.7 —	115.488.45	15.9	77 - 77	236.34
FOUNDATION III: Cabson and Filling Cab and Filling Sinking Calson Erection and Removal of Machinery .	20,988.33 17,089 81 21,921.27 } 520.59 }	 _ _ 60,520.00	Ξ	2,331.11 2,269.49 537.71		=	23,319 44 19,359,30 22,979.57	 65,658.31	=	25.930.31 46,015,00 —	89.9 42.1 —	. 118,369.35	19.0	79.71	381.76
FOUNDATION 1V: Caisson and Filling	15,697.86 13,796.73 16,781.41 1 478.26 §		Ξ	1,693,58 1,826,69 373-58		Ξ	17,391.44 15,623.42 17,633.25	20,648.11	=	27,368.21 49,563.36 —	63.5 31.5 —	103,548,75	16.6	78.15	219.51
COST OF FOUNDATIONS		_	\$201,083,42	- 8		\$19.659.33		-	\$220,742.75						
MASONRY PIER 1	Ē	16,192,12 36,998.07 37,845.10 23,506 30	114,541.59	Ξ	942.82 2,198.92 2,197.32 1,393.97	6,733.03	=	17,134-94 39,196.99 40,042.42 24,900.27	121,274.62						
TOTAL COST OF FOUR PIERS,	-	-	\$315,625 01	_	-	\$26,392,36			\$342,017.37						

The towers which carry the approach spans are supported on brick piers, the plans of which are given on Plate 12. An excavation was made at the site of each pier in which nine piles were driven; a further excavation was then made around the piles and filled with concrete. In this concrete was buried an annular washer from which six anchor rods extended upwards. The pier was then built up with hard brick laid in Portland cement mortar around the rods and surmounted with a wrought fron cap plate. Another annular washer was then placed on this cap plate and the rods serewed up till an initial strain of five tons was obtained in each rod; the cap plate, which was formed of a plate and a circular channel iron was then filled with Portland coment concrete; a second cap plate was then put on top and the whole riveted up. The design was based on the principle that the tension in the rods would always keep the brick work under strain and so prevent its jarring loose. These piers were built by the day by the company's own men, the total cost of the 16 piers being \$\$23,264.66 of which \$1,479.55 was for freight.

The amount of masonry and concrete in the entire bridge is as follows:

				MASONRY, Cubic Yards,	BRICK, Cubic Yards.	CONCRETE. Cubic Yards.	TOTAL.
Pier I			,	561.8		2,100 8	2,671.6
Pier II				1,285,9		1,486,1	3/272.0
Pier III .				1,317.0		1,949.0	3,266 o
Pier IV				814.4	-	2,267.0	3,081.4
16 Small Plers					263.8	266.7	530.5
Total				3.979.1	263.8	8,578 6	12,821.5

The total cost of the substructure was then as follows:

Four Main Piers .				\$342,017.3
Sixteen Small Piers				23,264.6
				\$265,282.0

The east end of the east deck span rests on a timber pier around which the embankment has now been filled; the west end of the west approach span rests upon a concrete block resting on the embankment and allowed to settle with the embankment, the end of the span being raised as settlement occurs. Both the timber pier and the concrete block have been treated in the accounts as parts of the approaches, though this distribution is not strictly correct.

IV. SUPERSTRUCTURE.

The superstructure consists of three through spans and six deck spans, here at each end,

Each through span is 375 feet long between centers of end pins, fifty feet deep and twenty-two feet between centers of trusses. Expansion is provided at the west end of every span, that is at the upper end, the bridge being on a grade.

Each deck span is 125 feet long between centers of end pins, 17 feet 6 inches deep, the trusses placed 12 feet between centers. The spans are separated by iron towers 25 feet long, thus making each set of deck spans with intermediate towers a continuous structure 425 feet long, divided into 17 equal panels of 25 feet each. The trusses are fastened rigidly to the posts which form the towers; expansion is provided at each end of the 425 feet, the expansion of the central span being taken out by the spring of the towers.

Proposals were invited from a number of prominent bridge builders and on the 2d of August 1886 the contract for the superstructure was closed with the Edgemoor Iron Co., by whom the entire work was manufactured. With my approval the Edgemoor Iron Co. sub-let the erection to the firm of Baird Bros., and it was done under the immediate direction of Mr. Andrew Baird.

The through spans are of the double system Whipple type, the trusses being divided into 15 panels of 25 feet each. The top chord, end posts, eye bars, floor beams, rods, bolsters, rollers, rail-bearing plates and pins, are of steel. All other parts are of wrought iron except the heavy wall plates resting on the masonry, washers and ornamental work, which are of cast iron. The details of these spans are given on Plates 14, 15, 16, 17 and 18.

The deck spans are of the single system Whipple type. The pins, rollers, rail-bearing plates and eye bars are of steel. The other portions are of wrought iron except the heavy wall pedestals, which are of cast iron. The details of these spans are given in Plates 19 and 20.

The trusses of the long spans were proportioned to carry a uniform moving load of 3000 lbs, per lineal foot of bridge, the effect of a moving load in excess of that due to a uniform load of equal intensity being estimated on the basis of 5000 lbs, per foot.

The floor system is proportioned for a total load of 6000 lbs, per lineal

foot of track. The top lateral system is proportioned to resist a wind pressure of 300 lbs. per lineal foot and the bottom lateral system a wind pressure of 500 lbs. per lineal foot. The computed strains are given on Plate 22.

The deck spans are proportioned for a total load of 5000 lbs. per foot, all treated as moving load.

The steel compression members in top chords and end posts are made as nearly as possible of symmetrical section, the metal in the top cover plate being practically the same as in the two balance plates and the lacing below. The compression strains on these members was limited to 15,000 lbs per square inch of net section, the net section being obtained by deducting from the gross section the amount by which the cover plate exceeds the balance plates. The tensile strain in the bottom chord was limited to 14,000 lbs. per square inch and somewhat less in the web members.

In the approach spans the tensile strain on steel was limited to 13,000 lbs, per square inch, the largest strain being in the center panel of the bottom chord.

The weights of iron and steel in the through spans are as follows:

	Three	Spans.	Average per Span.			
Steel	ibs.	1hs. 1,554,168	Ibs.	1bs. §18,056		
Wrought Iron in Trusses	860, 137		286,709			
Wrought Iron in Floor	.183.867		161,289			
Total Wrought Iron		1.343,994		447,998		
Cast Iron	_	60,056	-	20,019		
Total	-	2,958,218		986,073		

The weights of iron and steel in the deck spans are as follows;

				_
	 Sex	Spacs.	\verage;	per Span
Steel	lbs.	lbs 170,379	Ibs.	1bs 28.396
Wrought Iron in Trusses	343:913		57-319	
Wrought Iron in Floor .	278,266		46,378	
Total Wrought Iron		622 179		103,697
Cast hon	_	7,094		1,182
Гоіаі		799,652		133,275
Four Towers	 -	313'441		
Total		1,013 093		

The specifications under which the superstructure was manufactured are given in Appendix F.

The steel was all open hearth steel, the total number of melts used being 16t, made by the following parties:

Cambria Iron and Steel Co. 82 melts.
Carnegie, Phipps & Co., Limited 52 "
Pennsylvania Steel Co. 5 "
Pittsburgh Steel Casting Co. 22 "

The work was manufactured by the Edgemoor Iron Co. at its works near Wilmington, Dela.

The first set of eye bars tested did not meet the requirements of the specifications, a considerable number of them breaking in the head; it seemed probable, however, that the breakage was not due to any defect in the bars, but to the fact that the machine was not strong enough to break them and that its own failings caused irregular strains. I therefore thought best to open the question anew and make the rejection of the bars depend on a second set of tests to be made in the large testing machine at Athens, Penna; the results of these tests are given in Appendix G and on these tests the eye bars were accepted.

The trusses were erected on pile false work, a large traveler taking the place of upper false work. Thedates at which the several parts were erected is shown in the following table:

East Approach Spans		First Iron Placed, Sept. 4, 1887.	Span Swung. Sept. 14, 1887.
Span I-II		Sept. 18, 1887.	Sept. 24, 1887.
Span II-III		Aug. 14, 1887.	Aug. 18, 1887.
Span III-IV		May 21, 1887.	May 27, 1887.
West Approach Spane		April 12 1992	Tulu00-

The last span would have been swung three days earlier but was delayed waiting for material.

The timber floor was placed on the superstructure by the company's own men, working under the direction of the Resident Engineer. The painting was also done by the company's own men working under the direction of the Resident Engineer.

The total cost of the superstructure is given in the following table:-

			1
THROUGH SPANS,			
Iron, Steel and Ornamental Work	\$103,827.91 4,001.34	\$107,829.25	
Labor,—Erection Miscellaneous Material: Cement, Iron Borings, Sal	-	29,730.00	
[Ammonia, Sulphur	_	57-75	- \$137.617.00
DECK SPANS,			4.3//
Iron and Steel	33.854.76 1,442.40	05 205 16	
Labor—Erection	_	35,297.16	
Filling Castings Jacking up End of Spans	_	13.85	
Boxing Bars	_	18.00 2.75	
Switching Charges Miscellaneous Material: Iron Borings, Sal Ammonia &c.	=	55.84	
			41,918.41
FLOOR.			
Material	_	7.771.23	
Freight Labor	_	922,48 2,802.12	
		-	11,495 83
PAINTING.			
Material	_	1,340.95	
Labor	_	4,107.87	5,448.82
Total Superstructure,	-	_	\$196,480.06

V.

APPROACHES.

The east approach to the Rulo Bridge is 2.88 miles long from the connection with the track of the St. Joseph & Nebraska R. R. to the east end of the iron work. Of this approach the 2000 feet next to the bridge was built originally as a timber trestle and subsequently filled in with material hauled from the great cut on the west approach. The remainder of the approach was built as an earth embankment, the material being generally borrowed from the sides, though a portion of the higher embankment was taken

with a steam shovel from a pit in the bottom land. The total amount of earth in this Approach is as follows:—

The west approach is 3.64 miles long from west end of the iron work to the connection with the Atchison & Nebraska track in the Nemaha bottom. There are on this Approach three cast iron pipe culverts and four small trestes having an aggregate length of 495 feet. All the remainder is earth work the total quantity being as follows:—

I	Sarth	excavation	hau	led	to	еа	st:	sid	e		326,970	cubic	yards.
	£ +	44	usec	lia	e	nb	ınl	cm	ent		240,173	tt	22
	re	11	wasi	ted							32,358	11	11
	f.	t t	bor	row	ed						44.340	££	(t
F	lock	**									2,700	EE	es
		Total									646,541	cubic	yards.

These quantities include an extra width of grading for a distance of 1200 feet at the Rulo station grounds.

The Atchison connection is 1.36 miles long leading southward from the west approach, with which it connects 760 feet from the west end of the iron work. There is a trestle 680 feet long in the town of Rulo, and a pile trestle 48 feet long near the connection with the old track, the remainder of the line being earthwork. The amount of material handled on this line was 42,718 cubic yards.

The alignment and gradients on the Approaches are shown on Plate 2.

The contract for the earthwork of the Approaches was let May 19, 1886, to S. Dwight Eaton, of Burlington, lowa. This contract covered both the East and West Approaches and the Atchison connection; it was not signed till lune 26, 1886.

Grading was begun on the Atchison connection June 18, 1886, on the West Approach June 22d and on the East Approach July 13, 1886. The line from the east end of the great cut to the connection with the Atchison & Nebraska track was not built till after the bridge was opened, the line to the west being made by way of the Atchison Connection, trains running backwards between Rulo Station and Rulo Y, although the bridge was opened for traffic on the 2d of October.

No earth was taken across the river from the great cut till November 23, 1887. More than seven weeks of the best season of the year being lost through unnecessary delays; from that time forward the work was handled with rather more efficiency and the entire West Approach was completed and ready for the track on June 3d, 1889.

The great cut has a maximum depth of 82 feet. It is excavated through material of a mixed character, it being generally a sandy clay but containing some masses of hard blue clay and pockets of sand; these pockets of sand were generally filled with water, which water ran out when the excavation was made and did not reappear. The bottom of the cut is formed everywhere of hard clay, excepting near the summit at the west end. The cut was laid out with a 50 foot base with slopes of one horizontal to two vertical. In some instances these slopes were slightly increased, and some of the material was left in the base so that the finished width was generally not more than

40 feet. The cut was thoroughly ditched, left of sufficient width to be kept clean permanently, and the track was thoroughly ballasted. The character of the material, however, in this cut is such that it will require constant attention for a number of years in keeping the ditches clean, so as to avoid saturating the material in the sides of the cut by standing water; I regret to say that this has not been properly attended to since the work was completed.

A line nine tenths of a mile long was built connecting the East Approach with the dike above the bridge, the only object being to make a convenient line of access in case any repairs should be needed after the old tracks on the east side should be taken up; this line has at the east end a grade of three per cent. 158.4 ft. per mile.

VI. PROTECTION WORK.

The principal protection work is the dike on the east side. The position and plans of this dike are shown on Plate 23. The foundation of the dike was made of a woven willow mattress 125 feet wide, extending 100 feet inside and 25 feet outside of the center line of the track laid on the dike. On this as a foundation was built an embankment of brush and rock, which was carried to an elevation of 856 and on which a track was laid. This dike was built by the company's own forces in the winter of 1884 '85.

The dike was subsequently extended 700 feet down stream to Pier I by a permeable screen made by driving a series of piles through a woven mattress 50 feet wide and subsequently putting another mattress on the outside of the piles the lower edge resting on the first mattress and the upper edges wired to the piles. The effect of this screen was to allow the river to permeate through the screen so that there would be a current on both sides of the screen and thus entirely prevent the formation of an eddy at the lower end of the dike.

The dike worked perfectly, and during the high water season of 1885, a deposit was formed below it nearly as high as the top of the dike. A good growth of willows now covers the ground between the dike and the bridge line.

There was used in the construction of the dike 3571 cords of brush, 8712 tons of riprap stone, 2273 feet of piles and 4223 lbs. of wire.

The only protection work done on the west side was to riprap the shore in the neighborhood of Pier IV; for this 2530 tons of stone were

VII.

COST.

The cost of the bridge and approaches is given in the following table:

	Cost ex	clusive of Freight C	harges.		Preight Charges		Co	st, including Freigh	t.
Protection East Shore Protection West Shore Total Protection	\$ 14,495,02 2,048,42		\$16,543.44	\$4,028.63 956.85		\$4,985,48	\$18,523.65 3,005.27		\$ 21,528.92
Foundation Pier II Foundation Pier II Foundation Pier III Foundation Pier III Foundation Pier IVI Total Foundations	40,534.13 53,275.03 60.520.00 46,754.26	\$201,083.42		4,817.53 5,809.64 5,138.31 3,893.85	\$19,659.33		45,351.66 59,084.67 65.658.31 50,648.11	\$220,742.75	
Masonry Pier I Masonry Pier II Masonry Pier III Masonry Pier III Masonry Pier IV Total Masonry	16,192.12 36,998.07 37,845.10 23,506.30	114,541.59		942.82 2,198.92 2,197.32 1,393.97	6,733.03		17,134.94 39.195.99 40,042.42 24,900.27	121,274.62	
Approach Piers		21,786,11		_	1,478.55			23,264.66	
Total Substructure			337,411.12			27,870.91			365.282.03
Through Spans Deck Span Bridge Floor Painting Total Superstructure	133,615,66 40,476.01 10,573,35 5,448.82		190,113.84	4,001,34 1,442,40 922,48		6,366,22	137,617.00 41,918.41 11,495.83 5,448.82		196,480.06
East Approach, Grading and Masonry East Approach, Teestle West Approach, Grading and Masonry East Approach, The Company of the Company Archive Company East Archive Company East Company Ea	74,813.56 18,907.08 151,481.64 3,528.43 8,632.12 6,906.34 72,705.99			3,881,20 3,242,79 6,528,08 403,68 70,67 350,11 4,319,91			77,694.76 22,149.87 158,009.72 3,932.11 8,702.79 7,256.45 77,025.90		
			336,975.16			17,796.44			354,771.60
Service Tracks, Tools and Machinery Buildings	14,314.16 12,327.42 3,800.51		30,442.09	680.58 1,467.11 197.69		2,345.38	14,994.74 13,794.53 3,998.20		32,787.47
Engineering Salaries	45,400.64 4,134.03		49,534.67				45,400.64 4,134.03		49,534.67
Total cost			\$961,020.32			\$ 59.364.43			\$1,020,384.75

The item of freight includes freight only over the C. B. & Q System. In comparing the cost of this bridge with that of other structures the cost without freight forms the most correct basis for comparison.

This table may be condensed into the following:

	Cost, exclusive of Freight Charges	Freight Charges,	Cost, including Freight Charges
Substructure	\$337.411.12	\$27,870 91	\$365,282,03
Superstructure ,	190,113.84	6,366.22	196,480 06
Total Bridge Proper	527,524.96	34.237.13	561.762.00
Protection Works	16,543.44	4,1585.48	21,528.92
Approaches	336,975.16	17,796.44	354.771.60
Service Tracks, Tools and Buildings	30,442.09	2,345.58	32,787.47
Engineering and Expenses	49.534.67	_	49,534 67
Total Cost	\$961,030.32	\$59,364.43	\$1,020,384.75

This is the total cost of the Rulo bridge and approaches as built under my charge; the following additional items have, however, been charged to the cost of the bridge:

Land	Dama	ıge	S					\$46,721.35
Watch	ing							5,296.91
Prelin	inary	Е	хp	ens	es			340.25
Rulo	Yard							98.34
								\$52,456.85

Which makes the total cost \$1,072,841.60. Against this the construction cost is really entitled to a considerable credit for the amount of abandoned line on each side of the river. The item of watching covers the 25 months from October 1, 1887 to November 1, 1889, during which whole period the bridge was in use.

APPENDIX A.

LIST OF ENGINEERS, CONTRACTORS AND EMPLOYEES.

CONTRACTORS. ENGINEERS AND COMPANY'S EMPLOYEES. NATURE OF WORK. TIME OF SERVICE. Drake & Stratton Masoniy. Geo. S. Morison, Chief Engineer. James Doig, Supt. at Rulo. Benjamin L. Crosby, Resident Engineer Sept. 13, 1883, to Dec. 31, 1889. Edwin Duryea, Jr., Assistant Engineer Mat. 22, 1886 "Nov. 21, 1886. Mark A. Waldo, Assistant Engineer May 29, 1886, "June 16, 1887. Edgemoor Iron Co. Superstructure. Mark A. Watdo, Assistant Engineer May 29, 1886, " June 10, 1887. W. S. Macdonald, Assistant Engineer Aug. 9, 1886, " Feb. 25, 1889. A J. Himes, Assistant Engineer July 7, 1888, " Oct. 31, 1889. W. R. Johnson, Rodman Nov. vt, 1885, " Oct. 31, 1889. J. M. Richardson, Clerk July 18, 1886, " Oct. 31, 1889. R. F. Thayer, Timckecper June 23, 1886, " Aug. 13, 1887. Baird Bros., Sub-Contractors for Erection. J. S. Wattles East Approach Trestle. E. P. Butts, Inspector of Stone at Quarries . . . Sept. 1, 1886, " July 15, 1887. John Naegeley, Inspector of Superstructure Sept. 20, 1886, "Oct. 6, 1887. Paul Willis, Assistant Inspector of Superstructure . . . Dec. 15, 1886, "Jaly 13, 1887. Feb. 12, 1886, " Sept. 1, 1887 Dec. 10, 1885, " July 23, 1887. Dec. 11, 1885, " May 13, 1886. 5. S. Warrington, Foreman of Carpenters June 10, 1886, "Apr. 7, 1887. Bitton Reed, Inspector of Masonry Nov. 19, 1886, "Feb. 25, 1887. Charles Stears, Inspector of Masonry May 4, 1887, "July 18, 1884, "Feb. 25, 1887. John Newman, Foreman of Laborets Nov. 28, 1884, "Feb. 25, 1887.

APPENDIX B.

ACT OF JUNE 18, 1884, AUTHORIZING CONSTRUCTION OF RULO BRIDGE AND CONTRACT WITH WAR DEPARTMENT.

An act to anthorize the construction of a bridge across the Masouri Raver at some accessible point within ten miles north and ten miles south of the town of Rulo, in the county of Richardson, in the State of Nebrosler.

BE IT ENACTED BY THE SENATE AND HOUSE OF REPRESENTATIVES OF THE UNITED STATES OF AMERICA IN CONGRESS ASSEMBLED:

That the Atchison and Nebruska Railway Company, an incorporation organized under the laws of the State of Nebruska, as hereby arthorized to construct and maintain a bridge across the Missouri River at such a point as may be hereafter selected by said corporation within ten miles north and ten miles south of the town of Rulo, in the county of Richardson, in the State of Nebruska, as shall best promote the public convenience and welfare and the necessities of business and commerce, and also to construct accessory works to secure the best practicable channel-way for navigation and confine the flow of the water to a permanent channel at such point, and also to lay on and over said bridge a milway track for the more perfect connection of any milendash that are or shall be constructed to said river at or opposite said point; and said corporation may construct and maintain ways for wagons, carriages, and for foot-passagers, charging, and receiving reasonable toll therefor as may be approved from time to time by the Sceretary of Was.

SEC. 2. That said bridge shall be constructed and bulk without interference with the security and convenience of navigation of said river beyond what is necessary to carry into effect the rights and privileges hereby granted; and in order to secure that object the said company or corporation shall submit to the Secretary of war, for his examination and approval, a design and drawings of the bridge, and a map of the learton, giving, for the space of one mile above and one mile below the proposed location, the topography of the banks of the river, the shore-lines at high and low water, the direction and strength of the currents at latages, and the soundings, accurately showing the bold of the stream, the location of any other bridge or bridges, and shall furnish such other information as may be required for a full and satisfactory understanding of the subject, and until the said plan and location of the bridge are approved by the Secretary of War the bridge shall be for the bridge and the said plan and location of the bridge are approved by the Secretary of War the

PROVIDED, That if the said bridge shall be made, with unbroken and continuous spans, it shall have three or more channel spans, and shall not be of less elevation in any case than fifty feet above extreme high-water mark, as understood at the poant of location, to the bottom, ched of the bridge, nor shall the spans of said bridge be less than three hundred feet in length, and the piers of said bridge shall be parallel with the current of said river, and the main span shall be over the main chonnel of the river, and not less than three hundred feet in length.

AND PROVIDED ALSO, that if any bridge built under this act shall be constructed as a draw-bridge, the same shall be constructed as a pivot bridge, with a draw over the main channel of the river at an acessible and avigable point, and with spans of not less than one hundred and saxy feet in length in the clear on each side of the central or pivot pier of the draw, and the next adjoining span or spans to the draw shall not be less than three hundred feet, and the head room under such span shall not be less than ten feet these bubb acts mark.

anore uga water mark.

PROVIDED ALSO, That said draw shall be opened promptly upon reasonable signal for the passing of boats, and said company or corporation shall maintain, at its own expense from sunser till sunrise, such highs or other signals on said bridge as the Laght-House Board shall researche:

PROVIDED ALSO, That all railway companies desiring to use said bridge shall have and be entitled to equal rights and privileges in the passage of the same, and in the use of the machinery and fixtures thereof, and of all the approaches thereto, under and upon such terms and condutions as shall be prescribed.

by the Secretary of War, upon hearing the allegations and proofs of the parties, in case they shall not agree.

SIGE, J. That the Secretary of War is bereby authorized and directed, upon receiving such plan and man older information, and upon being satisfied that a bridge built on such plan and with such accessory works and at such locality will conform to the prescribed conditions of this act, to notify the company that he approves the same, and upon receiving such notification the said company may proceed to an erection of said bridge, conforming strictly to the approved plan and location; and should any change be made in the plan of the bridge or said accessory works, during the progress of the work thereon such change shall be sub-lect likewase to the approval of the Secretary of War, and in case of any lifegidion arising from any obstruction to the free analyzation of said river caused or alleged to be caused by said bridge, the case may be brought in the circuit court of the United States of the State of Nebraska or State of Iowa is which any certain of said obstruction to the freezing of said obstruction to the first may be located.

SEC. 4. That the said bridge and accessory works, when bull and constructed under this act and according to the terms and limitations thereof, shall be lawful structures and said bridge shall be recognized and known as a post-route, upon which also no higher charge shall be made for the transmission over the same of the mains, the troops, and the monitions of war of the United States than the rate per mile poid for the transportation over the railrander applied highways lending to such bridge, and said bridge shall enjoy the rugbos and privileges of other post-routes in the United States, and Congress reserves the right at any time to regulate by appropriate legislation the charges for freight and passenges over said bridge.

SEC. 5. That the United States shall have the right of way for such postal-telegraph lines across said bridge as the Government may construct or control.

Sec. 6. That Congress shall have power at any time to alter, amend, or repeal this act so as to prevent or remove all material and sustantial obstructions to the navagation of said river by the construction of said bridge and its accessory works, and the expense of altering said bridge or removing such obstructions shall be at the expense of the owners of or persons controlling such bridge.

Received by the President, June 6, 1884.

(NOTE BY THE DEPARTMENT OF STATE.—The foregoing act having been presented to the President of the United States for his approval, and not having been returned by him to the House of Coagress in which it originated within the time prescribed by the Constitution of the United States, has become a law without his approval.)

CONTRACT.

WHEREAS, By an Act of Congress of June 18, 1884,—23 Stats. 45 — entitled, "An Act to authorize the construction of a bridge across the Missouri River at some accessible point within ten miles north and ten miles south of the town of Rulo, in the county of Richardson, in the State of Nebrasia," was encaced that the Archison & Nebrasia Raulway Company, an incorporation against under the laws of the State of Nebrasia, is hereby authorized to construct and maintain a bridge across the Missouri River at a point within ten miles north and ten miles south of the town of Rulo, in the county of Richardson, in the State of Nebrasia, and to construct accessory works to secure the best practicable channel way for navigation, and also to lay on and ower said bridge a railway track to construct and maintain ways for wagons, carriages and foot passengers, churging and receiving reasonable toll therefor as may be approved from time to time by the Secretary of War and

WHEREAS, It was further enacted by the Act of Congress aforesaid, that the said bridge shall be constituted and built without interference with the security and convenience of navigation of said river beyond what is necessary to carry into effect the rights and privileges hereby granted; and in order to secure that object the said company or corporation shall submit to the Secretary of War for his examination and approval, a cleage and drawings of the bridge, and an part of the location, giving, for the space of one mile above and one mile below the proposed location, the topography of the banks of the river, the shore lines at least the space of the stream of the stream of the space of the stream of the space of the stream of the stream of the space of the stream of the space of the stream of the stream of the space of

WHEREAS, The Secretary of War is anthorized and directed by said Act of Congress, upon receiving such plan and map and other information, and upon being satisfied that a bridge built on such plan and with such accessory works and at such locality will conform to the prescribed conditions of said Act, to notify the company that he approves the sune, and

WHEREAS, The Atchison and Nebraska Railway Company in accordance with the provisions of the Act of Congress aforesaid, has submitted to the Secretary of War for examination and approval its design and drawing of the said bridge, and a map of the location of the same, as by said Act of Congress required, and

WHEREAS, The Chief of Engineers, United States Army, has reported, "That from the best and most reliable information attainable by this softion, contained an the papers herewith, at its believed that the plans of the bridge at Rulo are substantially in accordance with the requirements of the Act of Congress" aforesaid.

Now THEREFORE, I, William C. Endicott, Secretary of War, having examined and considered the plan and location of the bridge submitted by the said Atchison and Nebraska Railway: Company, as aforesaid, which said plan and map of location are hereto attached and form part of this instrument, do hereby approve the same.

But it is understood and agreed that this approval is given upon the express conditions following—

 That the said bridge shall be erected at the point indicated in the map of location submitted, and be constructed in accordance with the provisions of said Act of Congress, and the plan submitted and attached as a foresait.

That should any change be made in the plan of said bridge during the progress of construction such change shall be subject to the approval of the Secretary of War.

Witness my hand this 25th day of February, 1887.

WM, C. ENDICOTT, Secretary of War.

This instrument is also executed by the Atchison and Nebraska Railway Company, by its President G. W. Holdregs, thereto lawfully authorized this 11th day of February, 1887, in testimony of its acceptance of the provisions of the said Act of Congress, and the conditions therein imposed.

THE ATCHISON AND NEBRASKA RAILWAY CO.

In presence of
THOS, MILLER,
WM, A. HIGGINS
C. D. DORMAN,

G. W. HOLDREGE, President

Attest, J. C. Taylor, Secretary.

APPENDIX C.

SPECIFICATIONS FOR MASONRY

There will be four piers, numbered from east to west. Piers 1 and IV will be within the shore lines, and will contain approximately 350 and 800 cubic yards off massiony. Piers II and III will be switch on finished concerned foundations, and above the surface of the water. The massing of Piers II and III will give so from a dator by the plenum penumate pieces, the bottom of the messiony finishing about twelve feet below low water, and about the feet for the low low water, and about twenty-five feet of massing having to be laid while the sinking of the foundations is in progress.

The masonry will be first-class rock face work, laid in regular courses. The face stones, including enping, which shall be of uniform character throughout, and acceptable to the Engineer. The backung may be of any good, sould limestone.

The piers shall conform in all respects to the plans furnished by the Engineer.

No course shall be less that sixteen inches thick, and no course shall be thicker than the course below it. The upper and lower beds of every stone shall be at least one quarter greater in both directions, than the thickness of the course, and not face stone shall inemaster less than thirty inches in either horizontal direction, In general, every third stone of each course shall be a beader, and there shall be at least two headers in each ourse between the shoulders. No stone will be considered a header that measures less than five feet from the face. The beaders shall be so arranged as to form a bond entirely through the pier, either by bonding against a face stone in the opposets used of the course, or by bonding with a piece of backing not less than three feet square which shall bond with a face stone on the opposite side. In all cases the interier bonding shall be further secured by placing in the course above, a stone of the full thickness of the course, and at least true feet square, over the interior joints. Special ever shall be taken with the bonding of the ice breader cut water, the stones of which shall be so arranged that the face stones are supported from behind by large mores of largith.

All joints shall be pitched to a true line, and dressed to one quarter of an incb for at least twelve inches from the face. Beds, both upper and lower, shall be pitched to a true line, and dressed to one quarter of an inch. Joints shall be broken at least fifteen inches on the face. The bottom beds shall always be the full size of the stone.

The pointed up-stream starlings of Piers II and III, from the footing courses to the small coping at the offset, shall have a fine pointed face, with no projection exceeding one-half inch from the pitch line of the joints. There shall be a draft line three inches wide, around the lower edge of the belting course below the coping, and on the edge of the pointed starlings. The entire coping over the whole pier, and the small copings over the starlings, shall have a rough quarry-face, with no projection exceeding there inches from the pirch line of the joints.

The face stones of each entire course of Piers II and III above the footing courses and below the offset at the top of the pointed starlings, shall be doweled into the course below with round dowels of one and oneeighthinch roon, extending av inches intocach course. The dowels shall be from eight to twelve nethes back from the face and ax nothes on each side of every joint, the stones of the upper course shall be defilled through before setting after which the drill-hole shall be extended six inches into the lower course: a small quantity of mortar shall then be put into each hole, the dowel dropped in and driven home, and the hole failed with mortar and rammed. The three courses below the coping shall have the joints bound with eramps of g^* round iron, twenty inched long between the shoulders, the ends sunk three inches into each stone.

The stones in the coping under the bearings of the trusses shall be according to special plans, to be fornished. They shall have good beds for their entire size, and shall have a full bearing on large stones with dressed beds in the belting course below the coping.

The stones of the backing shall have dressed beds. The backing shall generally be of the same thickness as the face stones, but two thicknesses of backing may be used for one course of face stones, provided no backing is less than twelve inches thick.

All stones shall be sound, free from seams, and other defects, and the quarries shall be approved by the Engineer. All lumestone shall be laid on natural beds.

All stones shall be laid in full mortar bods. They shall be lowered on the bed of mortar, and brought to a bearing with a munl. No spalls will be allowed, except in small vertical openings in the backing. This mortar joints will not be insusted on, but the joints shall be properly cleaned on the face and pointed in mild weather, the policing to be driven in with a caulking upon.

The mortar will be composed of eement and clean, coarse sand, satisfactory to the Engineer, in proportions varying from one to three parts of sand to one of cement, as may be directed by the Engineer, for different parts of the work. When the stone is laid in freezing weather the contractor shall take such precautions to prevent the mortar from freezing as shall be satisfactory to the Engineer.

No material shall be measured, nor included in estimates, which does not form a part of the permanent structure.

The Railroad Coupany will furnish free transportation from Kansas City or Council Bluffs, or any intermediate point on the K. C., St. J. and C. B. R. R. to the bridge site, for stone actually used in the piers. Free transportation will also be furnished from an approved lineaction quarry in Nobesia, not more than 75 miles from Rulo, on a railroad openated by the C. B. & Q. R. R. Co. This free transportation is given on the assumption that the stone is to be cut at the quarry, and if the contractors prefer to cut at the bridge set, a clarge will be made at the rate of eight mills per ton per mule for the difference between the weight of the stone transported and the finished weight, as lud in the piers. Any stone transmitted free, and not used, will be the property of the Railroad Company.

The Railroad Company will furoish cement for mortar, which must be handled from the cars, or storehouses by the Contractor, who will be held responsible for any loss or waste.

The Contractor will be required to furnish all tooks of every description and all materials, except cement, and will be responsible for all damages which may occur from the conduct of the work embraced in his contract.

APPENDIX D.

RECORD OF SINKING CAISSONS. PIER I.

	Brava	ATIONS	ог Сотт	ING EDG	E	Sunl		F	EI EVATIO	ONS OF G	ROUND		Average Penc-		Death				WE	IGHTS.			_		AIR PRE		Reaction	Net	Surface.	Average Weight per sq. ft.	REMARKS.
N. E	, N.	. w.	8, E,	8. W.	Averag	Hour	s. -	E. 3	N. W.	5. E.	s. w.	Average,	tration of Caisson	Gauge.	Depth Immersed,	Cai	sson.	cle. Timbe	Crib		Lock, 1 Shaft, etc.	Masonry.	Sand.	Total. I	indroated.	Calcula led.	due to Air Pressure.	Weight		surface exposed to friction.	
844.3 844.6 841.3 837.9 830.6 828.3 825.6 825.6	12 84 02 84 24 84 95 83 35 83 35 83 64 83 26 82	44.15 44.00 41.47 37.56 34.37 30.76 30.47 27.96 24.87	838.25 834.53 831.70 830.28 828.30 824.45	844.12 842.44 837.92 834.65 831.67 830.16 828.00 824.40	844.1 844.6 841.8 837.9 834.4 831.2 830.3 828.1 824.6	1 0.21 0 0.1 3 2.1; 2 3.9 7 3.4 5 3.2; 9 0.8 3 2.2;	1 847 7 851 851 852 852 852 853 854 846	1.50 8 2.70 8 2.30 8 2.10 8 2.10 8	849.04 851.80 851.20 849.90 850.50 850.50 849.00	851.02 850.29 850.40 850.01 850.60 850.60 848.90	850.57 853.02 852.40 851.00 851.00 851.00 849.50	851.65 851.67 850.80 851.05 851.05 849.33	20.66 22.92 24.64	G 848.35 848.55 848.90 849.30 849.90 850.20 850.35	H G—C Ft, 4-35 6.82 10.98 14.83 18.45 19.51 21.92 25.51 25.66	155 155 155 155 155 155 155	5 16 5 30 5 5 50 5 50 5 50 5 50 5 50 5 50 5	as. Ton:	2 2 3 4	Tons	Tons. 6 6 6 6 6 7 8 8 9 9 9 9 9	Tons.	3 24 38 58 69 93	Tons. 186 352 583 773 1020 1264 1486 1515 1050 1799 1890	K Lbs 4.75 6.75 6.78 9 10 10.4	L Lbs. — 1.88 2.95 4.75 6.42 7.98 8.44 9.48 11.04	NI L × A Tons. ————————————————————————————————————	N I	O 8q. Ft. — 460 1047 1560 2621 2951 3113 343 3694 4060	P N+O Ubs. — 1756 940 738 498 492 457 455 404 410	Began Concreting at 8 a. m. Started Water Pumps at 8 a. m. Started Water Pumps at 8 a. m. Stegan building crib. Material passed through, river sand.
816. 808. 806. 804. 804. 804. 803. 803. 803. 803. 803. 795. 792. 791. 791. 791. 791. 789. 789. 789. 789. 789.	37 81 82 82 82 82 82 82 82 82 82 82 82 82 82	19,30 14,69 14,69 16,57 08,83 06,68 05,01 05,01 05,00 04,98 03,48 03,48 01,07 98,60 1992,08 991,12 991,14 891,54 891,54 891,54 891,54	834.40 819.56 814.64 809.72 807.43 805.28 805.28 803.46 803.45 803.45 803.45 803.45 803.45 803.45 803.29 798.16 799.16 799.16 799.19 791.22 791.22 791.20 791.88 792.79 791.88 792.79 791.88 792.79 791.77 785.79 788.79 788.79 788.79	819, 58 814, 70 809, 89 807, 65 805, 50 803, 70 803, 70 803, 70 803, 20 803, 70 803, 20 803, 70 803, 20 798, 27 798, 27 791, 98 791, 90 791, 91 791, 01 791, 01 791, 01 791, 01 791, 01 791, 97 789, 57 788, 57 788, 57 788, 57 788, 57	819.4 814.6 816.1 805.0 805.9 804.2 804.2 804.2 804.2 804.2 804.2 795.8 793.5 791.2 791.1 791.2 791.1 789.6 789.6	1.7 4 4.5 4.5 4.5 4.5 4.5 4.5 4.5 4.5 4.5 4	4 855-52 2 8 844 8 855-52 8 855-52 2 8 855-52 2 8 855-52 8 8 855-52 8 8 855-52 8 8 855-52 8 8 855-52 8 8 855-52 8 8 855-52 8 8 8 855-52 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	2.66 8 6.90 8 8.50 8 8 6.90 8 8 50 8 8 6 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	852.10 \$47.40 \$47.40 \$46.40 \$45.35 \$64.30 \$64.30 \$64.30 \$65.40	852-200 851-80 851-80 855-4	852.50 850.60 850.60 855.60 855.60 855.60 854.87 853.60 854.87 853.50 852.40	850, 85 848,61 854,8-61 854,4-15 854,4-15 854,4-15 854,4-15 854,4-15 853,3-2 853,4-3 851,7-2 851,97 850,97 851,97	32: 90 36:17 38:62 40:49; 48:18 50:17 50:22 50:53 50:53 50:60 51:20 60:56 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76 60:76	845.40 845.40 845.40 845.40 845.40		1555 1555 1555 1555 1555 1555 1555 155		78 9278 1007	77 78 89 99 100 100 100 101 101 101 101 101 101	1735 1735 1735 1735 1895 2053 2379 2610 2610 2610 2610 2610 2610 2635 2835 2835 2835 2835 2835 2835 2835 28	9 10 10 10 12 12 12 12 12 12 12 12 12 12 12 12 12		196 2-256 2-256 4-466 4-467 4-473 4-473 5-558 6-668 6-688 8-83 8-83 8-83 8-83 8-83 8	4176 4224 4285 4364 4532 4536 4538 4538 4546 4538 4546 4568 4680 4695 4775	11 13 15 17 17 19 19 19 19 19 19 20 21 22 23 24 25 25 25 25 25 25 25 25 25 25 25 25 25	11.43 13.29 14.89 16.67 17.35 18.04 17.58 17.58 18.18 18.18 18.58 19.59 24.50 25.53 25.53 25.63 26.63 26.67 26.77 26.49	1089 1259 1418 1588 1653 1707 1779 1779 1779 1779 1789 1779 2034 2186 2386 2411 2581 2481 2481 2481 2481 2481 2481 2481 24	9,46 1000 1061,4106 11329 1,402 1377 1386 1,406 1,416	4430 4000 5377 5734 5989 7022 7053 7338 7338 7338 7338 7396 8149 8770 8749 8749 8770 8749 8749 8752 8803 8818 9302 9411 9453 9403	427 409 395 312 378 378 378 378 383 383 383 384 460 460 460 470 485 447 451 464 470 485 464 470 485 464 470 485 464 470 470 485 464 470 470 470 470 470 470 470 47	Sand. Reached clav. Here pressure off to 9 lbs. in setting. Began excercation for test pit. Referred to bottom of "Test Pit" column $H = 68.7$ ft.), $L = 29.7$ lbs., $M = 283$, $LN = 1912$ tons, $P = 497$ lb.
786. 786.	83 78 85 78 14 75 15 78 100 78	87.55 87.51 86.81 85.82 85.82 85.76 85.76	787.74 787.76 787.05 786.04 786.04 785.97 785.97	787.50 787.53 786.82 785.80 785.80 785.72	787.6 787.6 786.6 785.6 785.6 785.8	66 0.0 66 0.0 65 0.7 15 1.0 15 0.0 18 0.0	0 853 0 853 0 853 0 853 0 853 0 853	2.30 2.70 1.30 2.20 2.20 4.10	854.40 854.60 853.80 853.80 853.80 854.30 854.30	853.10 853.20 852.90 852.20 852.20 852.40	853.20 853.10 852.50 851.80 851.70 851.70	853.25 853.40 853.40 852.50 852.50 852.50 853.12 853.12	65.59 65.74 65.67 66.55 66.55 67.24 67.24	848.20 847.80 847.60 848.20 848.65 848.80	60.54 60.14 60.65 62.25 62.70 62.92 62.67	155 155 155 155 155 155	25 5 25 5 25 5 25 5	78 23- 78 23- 78 23- 78 23- 78 23- 78 23- 78 23-	13 13 13 13 13	2835 2835 2835 2835 2835 2835	20 20 20 20 20 15 15	50 50 50 50 50		4743 4743 4747 4745 4772 4717 4739 4739	25 34 36 26 25 25 25 24 26	26.32 26.19 26.02 26.24 26.93 27.13 27.22 27.11 26.96	2507 2495 2478 2499 2505 2584 2593 2582	2236 2248 2369 2246 2207 2133 2146 2157	9495 9493 9423 9413 9528 9528 9618 9618	475 478 481 477 463 447 446 448	H=35,4 ft., L=33.66 lbs., M=3107 tons, N=1656 tons, P=348 lbs. Tost pit reached graved. H=65.6 ft., L=32.66 lbs., M=3336 tons, N=1413; H=62.3 ft., L=35.61 lbs., M=3392 tons, N=1555 tons, P=286 lbs., (=360 lbs., M=360 lbs

*Rubble Stone Piled on Crib.

RECORD OF SINKING CAISSONS. PIER II.

Date.		ELEVATIO	NS OF CU	TING EDG	P.			ELEVA	TIONS	or Grou			Average							WE	GHTS.					_	AIR PRI	SSURE				Average Welghi	
Date		1				Sunk in 24	_	1	-				Pene- tranon of	Waler Gauge.	Depth Immersed.	C	nisson.			Crib.	Tic.	ir ock,							Reaction due to Air Pressure.	Nei Weighi	Surface In Conlac	persq. fl. on surface	Remarks.
	N. D.	N. W.	8. E.	8. W.	Average	Hours	N. E.	N W	S. I	8. 1	Sou	of 12 indings.	Casson			Timber,	Iron. Co	oncreie.	Timber.	Iron. Co.	screte. e	nota Atms	sonry.l b	iand, V	Water. To:	al Tr	ndicated.	Calculated.				to friction	
1887 Feb. 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28	850.88 843.37 838.65 837.75 837.73 837.13 829.86 829.83 829.87 829.87 829.87 829.87 829.87 829.87 829.87 829.87	843.32 838.65 837.75 837.13 830.23 830.22 830.22 830.22 830.26 830.26	843, 32 838-65 837-79 837-15 837-13 836-04 829-89 829-89 829-89 829-89 824-32 820-37 820-36	843.32 838.65 837.79 837.13 835.98 830.30 830.31 830.27 830.09 820.32	850.88 843.32 838.65 837.79 837.15 837.15 830.08 830.06 830.03 830.03 830.03 830.04 830.04 830.04	0.00 0.03 0.10 5.59 3.91 0.01	847.00	843.40 843.75 844.00 843.30 841.50 847.60 847.60 846.80 846.80 846.60	837. 837. 837. 837. 839. 847. 847. 847. 847.	20 8411 40 842 70 842 60 841 40 842 40 841 80 848	70 83 10 81 00 84 80 84 30 84 30 84 70 84 40 84 40 84 60 83	42.02 41.76 41.56 46.04 46.82 46.37 46.19 39.78 46.19	F E—C FI. — — — — — — — — — — — — — — — — — — —	846.30 846.45 846.45 846.45 846.35 846.35 846.45 847.55 847.65 847.65 847.65 847.65 847.65 847.65 847.65	H G—C F1. 3.08 7.80 8.66 9.15 9.22 10.31 16.37 16.94 17.27 17.27 27.22 27.22	Tons. 177 177 177 177 177 177 177 177 177 177	Tons, 32 32 32 32 32 32 32 32 32 32 32 32 32	Tons. — — — — — — — — — — — — — — — — — — —	Tons	1 2 33 3 3 4 5 6	Fons. To	77777777777888899999999999999999999999	Fons. 1	Cons. 7	- 21 21 2 33 5 73	16 16 16 16 16 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	K Lbs.	L Lbs,	M L×A Tons. — — — 476 — — 816 1076	N I—M Tons — — — — — — — — — — — — — — — — — — —	1861	2054 	Carson built at elev. \$50.88. Began lowering at \$30 a. m Began concreting at 12.40 p. m. Stopped at \$30 p. m. Put on air at 6.45 p. m. Started water pumps at 8.45 a. m. Fegan building crib in P. M. Stopped water pumps 4:10 a. m. Resumed work in Pier III at 7.a. m., 17th Resumed work in Pier II at 6.45 a. m. Storted water pumps at 6:55 a. m., Coanse sand and rubbath Coanse sand and rubbath Water B. T. Coanse sand and rubbath """ """ """ """ """ """ """
Mar. 1 2 3 4 5 6 7 8 9 10 11 12 13 144 116 17 18 19 20 21 23 24 25 25 20 30 31	820. 5:4 820. 5:5 820. 48 820. 48 820. 48 820. 48 816. 7:3 815. 3:8 815. 3:8 80. 4. 805. 49 804. 5:4 804. 5:4 804. 5:3	5 820.43 5 820.43 6 820.46 8 820.33 6 820.46 8 820.33 8 820.	1 820, 33 1 820, 38 1 820, 28 1 820, 28 1 820, 28 1 820, 28 1 820, 27 1 817, 02 1 817, 02	820.28 820.27 820.24 820.24 820.21 820.21 820.21 820.21 820.35 811.15 809.35 808.01 807.05 816.53 816.53	820.39 820.38 820.35 820.35 820.35 820.31 820.31 820.22 817.02 815.516 811.13 809.20 808.65 805.97 804.61 804.33 804.43	0.02 0.01 0.03 0.00 	846-30 846-50 847-10 843-90 843-90 843-90 845-90 838-72 834-90 826-30 826-30 826-90 826-90 826-90 826-90 826-90 826-90 826-90	846.20 846.30 8412.70 842.20 843.00 820.30 820.30 820.30 821.70 825.70 826.20 825.40 826.40 827.40 824.80	846.8 846.8 846.8 846.8 847.8 847.8 847.8 847.8 848.8	70 838 20 840 80 837 00 839 40 839 40 839 90 839 80 839 80 829 814	20 8. 30 8. 60 8.	45.38 45.39 45.42 46.32 31.09 27.12 28.74 32.40 29.00 32.11 33.17 33.17 33.17 30.97 16.80 20.90 85	25.49 25.49 25.42 25.03 25.03 25.04 ————————————————————————————————————	849,40 848,95 848,80 852,20 854,00 854,75 855,30	26.79 26.16 27.22 27.05 27.25 27.05 27.05 27.05 27.05 27.05 27.05 26.84 20.30 38.34 13.34 69.30 49.30 49.30 49.30 49.30 50.43 49.30 50.43 49.30 50.43 50.98 50.98	177 177 177 177 177 177 177 177 177 177	**************************************	\$24 \$45 \$55 \$55 \$55 \$55 \$55 \$55 \$55 \$55 \$5	92 92 92 92 92 91 116 116 116 116 116 116 116 116 116	6 0 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	752 892 1033 3346 1346 1346 1346 1346 1346 1346 13	9999999999999999990222213131313131313131313131313131313131		20 26 516 172 1150 1670 248 388 388	162 34 194 35 188 37 244 38 289 38 381 38 381 39 387 39	31	15. 16. 5 18. 18. 19. 20. 21. 21. 22. 5 22. 5 22.	14.76 15.00 15.30 17.39 17.39 17.39 21.30 21.30 22.20 22.20	1576 1002 11673 11		2188 2920 3458 3994 3458 3733 3837 2009 1994 2646 2646	930 811 801 968 942 913 1510 1565 1171	All work suspended—ice too weak, " " " " " " " " " " " " " " " " " " "
April. 1 2 3 4 5 6 6 7 8 9 10 112 13 14 15 16	804-5- 804-4- 804-4- 803-5- 803-5- 803-5- 800-35- 799-8- 799-8- 799-45- 791-35- 791-34- 791-33- 791-34- 791-33- 791-34-	804.13 804.11 803.78 800.31 799.63 797.93 794.97 791.44 791.44	8 804.06 803.94 802.79 802.07 1 799.79 1 799.79 1 798.61 7 792.55 791.23 791.21 791.21	802.79 802.17 799.70 799.42 798.72	804.10 804.02 803.30 800.04 799.47 798.27 795.07 792.66 791.35 791.33 791.33	0.17 0.08 0.72 1.30 1.96 0.57 1.20 3.20 2.41 1.31 0.01 0.01	820.40 \$23.50 825.70 823.90 825.10 825.50 827.40 826.50 829,00	816.40 821 50 821,70 825,10 824.10 824.50 826.40 825,50	822. 821. 822. 823. 821. 823. 821. 824. 825. 827.	40 821 50 825 70 825 70 825 10 825 10 828 40 828 40 828 40 828 40 828 50 833 90 833	40 81 50 82 70 82 90 83 10 82 00 82 40 82 50 83 40 83	19.74 22.33 23.03 23.02 23.02 24.63 26.32 26.31	16.63 15.64 18.31 19.73 21.190 22.98 25.16 28.05 31.24 38.51 38.72	854 30 853-40 853-250 851 75 850-90 851 50 851 50 849-50 849-50 849-65 848-60 848-80 849-80 849-80	50. 03 49. 30 48. 48 48. 45 48. 90 51. 11 52. 03 52. 13 54. 43 56. 39 57. 55 57. 26 57. 27 57. 83	177 177 177 177 177 177 177 177 177 177	32 32 32 32 32 32 32 32 32 32 32 32 32 3	856 856 856 856 856 856 856 856 856 856	178 178 178 178 196 196 198 198 198 198 198 198	11 11 12 13 13 13 13 13 13 13 13 13	2810 2810 2810	13 13 14 14 14 14 14 14 14		38 35 54 77 112 129 166 216 276 423 427 —	337 40 323 41 341 42 333 42 323 44 299 45 300 46	67 39 48 05 80 76 86 86	21-5 21-5 21-5 21-5 21-5 23-5 23-5 22-5 24-5 24-5 25-	21.64 21.32 20.97 20.96 21.15 22.51 22.55 23.54 24.39 24.99	2311 22777 2239 2238 2268 2361 2404 2404 2504 2668	1576 1600 1835 1939 1971 1887 2001 2172 2162 2182 2118	2493 21920 3122 3420 3556 3857 4240 4673 5632	1283 1257 1235 1152 1061 1038 1024 925	Pumps started at 7:20 a. m. Sand, gravel and balls of clay. Finished orb. Pimps stopped at 6:50 p. m. and started at 10:50 p. m. Pumps stopped at 5:50 p. m. Casson on clay in north end. Started at 12:45 p. m. Pimpshed concerting crib. Pumps started at 8:5; a. m. and hinshed at 5:10 p. m. Cut off air at 6:5; a. m. and hinshed at 5:10 p. m. Cut off air at 6:5; a. m. and iresumed work in Pier III. Working in Pier III.

PIER II.—CONTINUED.

	ELI	EVA FIONS		TING EDG	 ie,	Sank		ELEV	ATIONS O	r GROUND		Average Pene-		Donath					WEIGH	ITS.				Air Pi	RESSURE	Reaction	Net	Burface	Average Weight per sq. ft	REMARKS.
Date	В.	N. W	8. E.	s w.	Average	Hours.	N. E.	N. W.	S E.	8. W	Average of 12 Soundings.	of Calsson.	Gauge.	Depth Immersed		Iron. Cor	nerete.	Timber 1	Crib. Lron Concre	Local Shaf etc.	Masonry	Sand-	Water. Total-	Indicated	Calculated.	due to Acr Pressure	Weight	Confact	Surface Exposed to fricton.	DENARG.
24 25 26 27 791 28 791 29 791 30 790	.30	791 -43	791.14	791.23	791 .28		835.10 836.20 829.10 829.10 833.90	827.10	838.70 	836.10 837.20 	837-25 — — — — 829-85 830-60	F E—C F1.	850, 10 850, 20 849, 85 849, 70 849, 50 849, 10 848, 95 848, 95 848, 45 848, 45 848, 45 848, 45 848, 45 848, 45	H G—C Ft. 58.79 — — 57.78 — — 57.17 56.89 56.86 57.85	Tons. 177 177 177 177 177 177 177 177 177 17	32 32 32 32 32 32 32 32 32 32 32 32 32	Fons. 856 856 856 856 856 856 856 856 856 856	Tons 198 198 198 198 198 198 198 198 198 198	Fons. Tons. 13 3816 13 2816 13	15 15 15 15 15 15 15 15 15 15 15 15 15 1	480 480 540 540 540 540 540 540 540 540 540 54	Tons	Tons. Tons.	K Lbs	L Lbs	M L×A Tous ————————————————————————————————————	N I—N Tons. — — — — — — — — — — — — — — — — — — —	O 8q. Ft		Worlang on Pier III. """ """ """ """ """ """ """ """ ""
Many 783, 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	.42 .56 .53 .39 .48 .67 .62 .67 .63 .67 .63 .67 .63 .67 .63 .67 .63 .67 .69 .69 .69 .69 .69 .69 .69 .69 .69 .69	789.62 789.53 787.58.28 787.79 788.28 787.79 785.79 785.72 784.15 785.72 784.15 783.80 778.380 779.42 779.42 779.43 779.43 779.43 779.79.43 777.56 777.56 777.57 774.77 774.77 774.77 774.77 774.77 774.77 774.77 774.77 775.38	789, 40 789, 28 787, 94 787, 51 787, 34 787, 52 786, 29 785, 85, 51 785, 45 783, 71 783, 44 778, 48 779, 24 778, 98 777, 52 776, 76 776, 58 776, 776, 776 776, 776, 776 776, 776, 7	789, 41 788, 67 787, 67 787, 48 787, 48 787, 48 785, 40 785, 50 783, 95 783, 95 779, 90 779, 90 779, 90 776, 93 776, 93 776, 93 776, 93 776, 93 776, 93 777, 10 773, 45	789, 41 788, 11 787, 67 787, 50 787, 56 786, 63 785, 63 785, 63 784, 66 783, 26 781, 57 781, 57 779, 72 779, 72 779, 72 779, 72 779, 72 776, 59 776, 69 775, 12 773, 73 773, 73	0 10 1.30 0.44 0.14 0.92 0.46 0.35 0.01 1.53 0.25 0.01 1.69 0.05 0.01 0.32 0.03 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.01 0.05 0.05 0.01 0.05 0.05 0.01 0.05	858.50 838.50 839.40 839.40 838.75 838.10 839.40 839.40 839.40 838.20 838.20 834.70 835.35 836.20 835.35 835.40 835.25 835.25 835.25 835.25 835.25 835.25 835.25 835.25 835.25 835.25 835.25 835.25 835.25	\$33.50 833.50 833.40 838.60 836.95 836.95 836.95 836.95 836.70 836.70 836.80 836.80 836.80 836.80 836.80 833.70 833.70 834.25 834.25 834.25 834.25 834.25 834.25 834.25 834.25	8 831.502 8 830.404 8 830.404 8 830.404 8 830.104 8 830.104 8 831.106 8 831.606 8 831.706 8 831.706 8 831.706 8 831.706 8 831.706 8 831.706 8 831.706 8 831.706 8 831.706 8 831.706 8 831.506	833 50 833 40 833 40 833 40 833 40 833 40 833 40 834 40 834 40 834 50 834 50 834 50 834 50 834 50 834 50 834 50 834 50 834 50 835 50 836 70 836 70 837 70 836 70 837 80 837 80 80 80 80 80 80 80 80 80 80	831. 04 834. 87 835. 18 834. 10 835. 10 835. 10 836. 03 837. 836. 03 837. 837. 837. 837. 837. 837. 837. 837.	43.71 44.89 46.77 47.17 48.08 46.77 47.17 48.08 50.05 51.25 51.35 51.35 51.35 51.35 51.45	847.50 847.55 847.55 847.40 847.40 847.40 847.40 847.40 847.30 847.30 847.30 847.30 847.70 847.70 847.70 847.70 847.70 847.25 847.70 847.80 847.70 847.80 847.70 847.81 847.70 847.81	\$8.09 \$8.09 \$9.73 \$9.74 \$9.73 \$9.74 \$6.61 \$1.25 \$6.12 \$6.23 \$6.23 \$6.23 \$6.23 \$6.23 \$6.23 \$6.23 \$6.23 \$7.25 \$6.23 \$7.25	177 177 177 177 177 177 177 177 177 177	***************************************	6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	198 198 198 198 198 198 198 198 198 198	13 2816 13 281	220 220 220 220 220 220 220 220 220 220	\$\frac{5}{40} \$\frac{5}{	536 550 590 628 604 644 645 691 774 774 774 840 840 814 905 916 905 917 907 918 919 919 919 919 919 919 919 919 919	192 5450 181 5457 177 5461 186 5463 180 5443 180 5443 182 5523 187 5526 204 5024 204	27. 26. 5 26. 5 26. 5 26. 5 27. 28. 5 27. 28. 5 27. 28. 5 28. 5 28. 5 28. 5 29. 5 29	25-13 25-13 25-13 25-13 25-14 25-14 25-14 25-14 25-14 26-63 27-33 27-77 28-63 27-37 28-63 29-34 31-92	2683 2683 2683 2745 2757 2757 2850 2860 2843 2844 2842 2915 2905 2905 3905 3905 3905 3134 3133 3341 3341 3341 3341 3343 3350 3356	2707 2713 2690 2702 2683 2619 2683 2619 2681 2662 2659 2612 2627 2628 2632 2632 2648 2718 2783 2783 2783 3052 3052 3053 3131 3131	6311 6358 65957 6787 6787 6883 7152 7128 7365 7365 7355 7365 7359 7368 7819 7849 7849 7849 8158 826 8158 8826 8834 8854 8854 8854 8855 8854 8855 8856 8856	791 7762 759 751 724 714 714 707 693 682 670 677 667 676 676 672 706	Clay hoss broke down at 5 a. m.—Sacking out. Clay hoss started up at 10:40 a. m. Put on section supply shaft—Stopped clay hoss from 7:25 a. m. to 6:45 p. m. Stopped clay hoist from 10:20 a. m. to 11 40 a. m. Clay. Clay. Causeon in gravel
June. 770. 2 768. 3 768. 4 767. 5 766. 6 7766. 8 765. 10 765. 11 765. 12 3 14 765. 12	.12 .80 .64 .57 .10 .95 .66 .49 .47	770 33 768.63 768.63 768.69 767.76 766.59 765.89 765.62 765.42 765.42	765.58	766 62 766.16 766.00 765.68 765.53 765.51	768.18 767.84 766.68 766.61 765.98 765.58 765.54 765.49	1.68 0.55 1.16 0.07 0.47 0.47 0.16 0.30 0.17 0.002 —	829.15 830.80 831.60 831.00	833.15 831.80 832.60 832.00	835.15 838.80 837.60 835.00 835.00	835.20 834.15 834.36 834.60 833.30 832.20 832.20	833.32 832.63 833.68 832 p8	63.81 64.59 64.45 65.40 63.83 67.64 66.80 67.10 67.27	848.20 848.15 849.15 849.15 849.95 849.95 851.40 851.40 851.40 851.40 853.50 853.55 853.55 853.55 853.55	77.79 79.42 80.62 81.31 82.37 82.59 83.31 84.52 85.77 86.39 87.11 ———————————————————————————————————	1777 1777 1777 1777 1777 1777 1777 177	32 32 32 32 32 32 32 32 32 32 32 32 32 3	######################################	198 198 198 198 198 198 198 198 198 198	13 281c 13 281c	220 220 220 230 230 230 230 230 230 230	2 1101 2 1162 1162 1255 3 1255 3 1311 1311 1311 1311 1311 1311 1311	1111 1153 1148 1218 1117 1253 1277 1287 	402 6722 431 6793 470 6827 451 6939 468 7035 550 7036 469 7171 525 7208 550 7235 509 7270 — — — — — — — — — — — — — — — — — — —	32. 33.5 33.5 33.5 34.2 34.5 34.5 37.5 36.3 36.3 36.3 32.3 32.3 32.3 32.3 32.3 32.3	33.65 34.36 34.87 35.17 35.63 35.77 36.04 36.59 37.10 37.37 37.38 38.33	3593 3669 3725 3855 3855 3848 3964 3964 3964 4023 — 4093 —	3124 3104 3184 3250 3216 3323 3291 3280 ————————————————————————————————————	\$\$09 \$885 \$875 \$052 \$659 \$914 \$141 \$161 	703 700 703 724 730 730 725 725 720 715	Stopped clay horst from 2 p. m. to 4 p. m. Put on section supply shaft—Stopped clay horst from 7,23 a. m. to 645 p. m. Stopped clay horst from 7 a. m. to 11:30 a. m. Clay at 767,72. Stopped clay horst from 440 p. m. to 8:30 p. m. Began claims clay bost at 143 p. m. Began claims owned claims of the clay claims of the clay p. m. Thermometer 100 o in lock. Cause working a hour sper day, account of heat. Persaure varying from 30 bs. to 34 lbs. Any pressure down to 35 lbs. at 540 p. m. Took off air at 705 a. m. Took off air at 705 a. m. That cleation of cutting edge 765,47.

APPENDIX D.-Continued.

RECORD OF SINKING CAISSONS. PIER III.

	E	EVATIONS	or Cuti	ING EIGE			_	ELBVAI	rions of (GROUND.		Average	-						V	Veights.					Ази Р	RESSURE,				Average Weigh!	
Date	N E.			S. W. /		Sunk in 24 Hours	N. E.	N.W.	- 1		Average.	Average Pene- Iration of Cabson,	Water Guage	Depth Immersed		Caisson		Tumber.	Crib.	Concrete	Air Lock, A Shaft,	iasonry.	Sand.	Vater. Total.	Indicated	Calculated.	Reaction due to Air Pressure.	Weight.	Surface	per sq. fl. on Surface Exposed to freetion	Remarks.
1886 Nov. 23 24 25 26 27 28 39	846.80 842.10 833.30 830.70 828.80	842.10 833.30 830.70 829.30	833.30 830.70 829.80	842.10 833.30 830.70 829.80	C 846.80 842.10 833.30 830.70 829.42 829.10 829.10 829.10	D Ft. — 4.70 8.80 2.60 0.28 0.32 0.00 0.00	831.10	- - - - - 831.60	830.10	839 10	E	F E—C F1.	G 842.20 842.05 842.70 841.80 843.00 843.00 842.60	H G=C Ft. - 9-40 11.20 12.38 13.90 13.50	Tons. 177 177 177 177 177 177 177 177			Tons.			Tons. 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	Tons.	Tons.	Tons. Tons 218 218 5 312 9 341 11 343 15 438 15 667 15 838	K Lbs.	L Lbs.	M L×A Tons.	N I—M Tons.	O Sq. Ft.	P N+O Lbs.	Caisson built at elev. \$45.80 Began lowering coloson in P. M. Began concreting in A. M.
Dec. 1 2 3 +566 78 910 112 134 1566 177 188 190 201 222 234 256 278 299 300 311 8886.	828.70 828.30 	827. 60 825. 90 824. 70 825. 72 825. 72 826. 72 826. 70 827. 72 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 21 820. 70 821. 20 821. 20 822. 20 823. 20 824. 25 824. 25 826. 52 826. 52 826. 52 826. 52 826. 52 826. 52 826. 33 826. 52	829,20 829,70 828,10 825,30 825,06 823,79 821,32 820,90 821,32 820,90 811,60 820,744 817,64 815,63	827, 50 825, 46 825, 46 825, 46 825, 46 821, 40 821, 40 821, 40 821, 40 821, 40 831, 40 831, 68 831, 68 831	828.32 828.32 827.75 826.60 825.60 825.40 825.40 820.80 820.73 820.80 820.73 817.95 815.60 815.60 815.61 815.60 81	0.08 0.00 0.57 1.15 0.98 0.22 0.80 0.48 0.00 0.00 0.00 0.00 0.00 0.02 2.33 0.00 0.02 2.16 0.00 0.15 1.56 0.00 0.13 1.56 0.13 1.56 0.13 1.56 1.56 1.56 1.56 1.56 1.56 1.56 1.56	828.90 827.75 826.70 830.20 830.20 834.20 834.40 835.80 824.40 822.10 822.80 821.40 822.80 823.90 823.90 823.90 823.90 823.90 823.90 823.90 823.90 823.90 823.90 823.90 833.90 833.90 833.90 833.90 833.90 833.90 833.90 833.90	827,10 827,25 824,470 826,30 827,80 837,90 833,75 833,10 821,80 821,80 821,90 821,90 821,90 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 821,80 831,80 8	828.490 829.457 829.470 828.770 829.490 839.555 839.555 839.560 831.80 837.40 8	829.40 838.90 831.25 831.25 837.70 836.60 837.10 831.20 831.20 831.20 831.50 831.50 831.50 831.50 832.40 833.40 833.40 833.40 833.40 833.40 833.40 833.40 833.40 833.40 833.40 833.60 833.40	828.83 828.85 829.29 827.49 828.45 829.71 839.82 839.71 833.36 833.36 837.77 836.66 837.77 837.69 837.73 83	00.43 00.53 00.97 00.15 1.82 1.220 1.231 1.205 1.2	842.90 841.25 841.25 841.70 843.00 843.00 843.00 843.40 843.40 843.40 843.40 843.40 843.40 843.50 843.50 843.50 843.50 843.50 843.50 843.50 843.50 843.50 843.50 843.50 843.50	14.50 14.58 12.93 13.95 16.10 17.58 17.59 18.60 22.24 23.82 24.10 22.53 27.78 28.18 30.76	177 177 177 177 177 177 177 177 177 177	34 34 34 34 34 34 34 34 34 34 34 34 34 3	605 605 605 762 763 830 830 830 830 830 830 830 830 830 83	8 8 16 30 30 30 42 45 57 57 63 63 63 70 78 88 89 99 99 114 120 127 127 148 151			7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7			15 838 17 846 13 844 14 15 855 22 1094 44 1216 53 77 1696 69 1916 61 1794 90 1916 111 2010 118 2025 119 2221 119 2221 124 2485 82 2517 110 2863 101 2863 101 2863 101 2863 101 2863 101 2863 101 2863 101 2863 101 2863 102 3643 103 3649 103 3649 104 3640 105	5.66. 7.5 8.8 9.510.55111.5511.55111			291 413 5516 565 565 6218 6218 6218 728 739 730 730 7312 878 988 988 1106 11166 11267 11261 11262 11262 11262 11262	69 86 159 244 299 694 694 694 697 697 192 207 1001 1480 207 207 209 237 2595 2595 2595 2595 2400 2406 2595 2595 2595 2595 2595 2595 2595 259	1946 1218 1752 1314 744 682 600 1514 1000 827 822 769 836 793 745 825	Began building crib. Put on air at ao a. m. 4 lbs. Pumps stopped at 935 p. m. Sand. Pumps started at 6.55 a. m. Pumps started at 5.55 a. m. Pumps at a life in a lif
Jan. 1 2 3 4 5 6 7 8 9 10 11 12 3 14 5 16 6 17 8 19 20 21 22 23 4 25 6 27 28 29 30 31	796.44 796.16 796.17 796.17 791.25 791.03 791.03 791.03 781.94 788.94 788.94 788.99 786.91 786.62 786.61 786.62 786.63 786.43 786.43 786.43 786.43 786.43 786.43 786.43 786.43	796.46 796.62 796.18 794.21 795.22 799.10 791.68 791.07 791.08 791.07 780.92 780.90	796.78 796.51 796.51 796.51 794.38 793.87 799.88 799.62 789.62 788.71 788.69 788.71 788.69 786.51 786.51 786.51 786.54 786.95 78	794.38 799.95 799.95 790.93 790.93 790.93 785.85 785.87 785.87 786.57 786.40 784.48 784.48 784.48 784.48 784.48 784.48	796.63 796.36 796.36 794.31 795.199 790.99 790.99 790.99 789.89 788.88 788.86 786.77 786.87 786.59 786.59 786.59 786.59 784.27 784.25 785 785 785 785 785 785 785 785 785	0.13	835,40 834,40 834,40 834,40 838,30 838,30 838,40 840,50 840,50 840,50 841,50 841,50 841,50 840,50 841,50 840,50 841,50 840,50 84	831,60 835,00 835,00 836,00 836,00 836,00 837,60	839. 100 831. 40 831. 100 831. 40 832. 70 832. 70 832. 70 832. 80 830. 60 831. 20 832. 70 832. 50 833. 70 833. 50 833. 70	833.00	838.38	38 - 50 58 - 25 58 - 59 40 - 82 41 - 63 41 - 63 42 - 63 43 - 63 44 - 63 45 - 73 46 - 63 51 - 63 53 - 63 53 - 63 53 - 63 54 - 63 55	845.70 846.05 846.10 846.10 846.10 847.10	59.86 60.89 61.06 61.26 61.21 61.48 61.75 62.78 62.78 62.62 63.65 63.68	177 177 177 177 177 177 177 177 177 177	******************************	830 830 830 830 830 830 830 830 830 830	172 172 178 178 196 198 198 198 198 198 198 198 198 198 198	11 11 12 13 13 13 13 13 13 13 13 13 13 13 13 13	1906 2048 2115 2186 2298 2298 2439 2439 2541 2541 2541 2541 2541 2541 2541 2541	12 12 12 12 12 12 12 12 12 12 12 12 12 1	62 146 223 275 377 378 439 439 473 473 633 633 633 633 633 633	423 449 474 575 585 585 660 660 675 7711 779 774 779 880 886 883 883 883 883 883 883 883	137 3702 147 3830 147 3847 147 4947 148 4940 149 4450 152 4442 153 4442 159 400 159 400 159 400 159 400 159 400 160 5140 167 5007 160 5147 160 5147 167 5007 167 5007 167 507 167 507 167 507 168 5147 181 5388 181 5488 181 5488 181 5488 185 5488 196 5482	23. 24.5 25.5 25.5 25.5 25.5 25.5 25.5 25.	21. 22 21.47, 22.49 24.47, 25.53 24.12 24.43 24.44 24.49 25.44, 25.46 25.46, 25.46 25.46, 25.46 25.46, 25.46 25.46, 25.46 25.47, 26.97 26.97, 26.97 27.99 27.99 27.99 27.99 27.99 27.99 28.16	2266 2293 2392 2347 2513 2576 2598 2610 2662 2713 2718 2721 2761 2761 2812 2850 2850 2850 2850 2850 2850 2850 285	1436 1557 1634 1647 17.59 17.43 18549 1813 1854 1813 1853 1585 1585 1205 2011 2082 2150 22178 2243 2251 2358 2343 2491 2480 2475	5632 55932 5932 5932 6173 6443 65871 6736 6659 6639 6639 6639 7128 7128 7295 7228 7231 7331 7331 7331 7331 7331 7331 7331	505 553 557 538 570 553 550 543 561 566 584	Pourps stopped at 0.35 n. m. Pourps starred at 1.10 p. m. Pourps starred at 1.10 p. m. Pourps stopped at 2.30 n. m. Sacking out Casson on class. Finished concreting crib at 2.40 p. m. Began laying masonry in P. M. Set op class host in p. m. Starred clay host at 4.40 p. m. Starred clay host at 4.40 p. m. Sacking out. Starred clay host at 1.45 p. m. Stopped day host for repurs at 7.30 a. m. Stopped day host for repurs at 7.30 a. m. Starred clay host at 1.45 p. m.
Feb. 1 2 3 4 5 6 7 8	780.36 780.25 780.20 779.86 777.69 777.68 777.46 777.28	780.31 780.16 780.10 779.79 777.61 777.56 777.33 777.14	780.35 780.22 780.16 779.84 777.75 777.61 777.39 777.19	780.27 780.16 780.11 779.78 777.65 777.49 777.28 777.08	780.32 780.20 780.14 779.82 777.67 777.58 777.36 777.17	1.27 0.12 0.06 0.32 2.15 0.09 0.22 0.19	840.80 838.80 839.00 839.60 841.90 840.00 840.40	834.80 835.00 835.30 834.60 834.90	835.50 833.30 832.90 831.90 832.70	832,70	835.57	54.96 54.84 54.86 55.68 57.90 57.25 57.79 58.19	846.86 846.85 846.86 846.85 846.90 846.90 846.90 846.70	66.48 66.65 66.66 67.03 69.23 69.32 69.54 69.53	177 177 177 177 177 177 177 177 177	34 34 34 34 34 34 35 34	830 830 830 830 830 830 830 830	198 198 198 198 198 198 198 198	13 13 13 13 13 13 13 13	2541 2541 2541 2541 2541 2541 2541 2541	20 20 20 20 20 20 20 20 20 20 20	633 633 633 633 633 662 662 735	818 814 814 836 897 879 891 905	253 5517 235 5515 255 5515 249 5531 275 5618 288 5642 284 5650 311 5764	29. 30. 29.5 29.5 30.5 31. 30.5	28.76 28.83 28.83 28.99 29.95 29.99 30.08 30.08	3071 3078 3078 3096 3198 2202 3212 3212	2446 2437 2437 2435 2420 2440 2438 2552	7751 7736 7736 7843 8121 8040 8108 8158	631 630 630 621 596 607 601 625	Stopped clay host at 3:30 p. m. and started at 6:30 p. m., "5:00 p. m. "From 2:30 to 5:45 p. m. repairs. Kased clay hoist 5 p. n. Stopped clay hoist from 5:15 a. m. to 9:20 a. m. Repairs.

APPENDIX D.—Continued.

PIER III.—CONTINUED.

1	ELEVA	ATIONS	or Cut	TING E	DGE				ELRV	ATION	S OF G	ROUND.		Average								WRIGHTS						Air Pi	RESEURE.	Busclion		Surleea	Average Weight	
N. E.	N.	. w.	8. E.	9. W.	. Av	erage.	Sunk in 24 Hours.	N. E.	N. W.	. s.	Е	S. W,	Average	Pene- tration of Causson.	Water Gauge.	Depth Immersed		Causson		Total	Crit	Concrete		Masonry	Sand.	Water.	Total	Indicated	Calculated	due to Air Pressure	Weight	Surface in Contact	on surface exposed to friction	REMARKS.
776.02 775.92 774-77 774-74 773-63 773-34 772-08 772-08 772-03 770-70 770-72 770-70 770-72 770-71 769-36 768-61	7755 7744 774 773 773 771 771 771 771 770 770 770 770	5.80 4.61 4.62 1.62 3.31 1.98 1.98 1.99 1.99 1.99 1.83 5.64 5.65 5.64 5.65	774-74 774-73 774-73 773-63 773-44 772-65 772-09 772-03 772-09 770-70	775.8. 775.8. 774.6. 774.6. 773.4. 773.3. 772.0. 772.0. 771.9 771.8. 770.6. 770.6. 770.6. 770.6. 770.6. 770.6. 770.6.	4 77 6 77 77 9 77 77 77 77 77 77 77 77 77 77 77 77 77		D FI 1.25 0.08 1.16 0.00 1.119 1.30 0.05 0.04 0.00 0.00 0.00 0.00 0.00 0.0	835.00 836.40 836.40 835.30 834.20 837.10 836.40 836.40 835.60 835.60 835.60 836.60 836.60 836.60	835.00 835.40 830.50 840.32 830.16 834.77 835.86 834.16 834.46 834.66	0 833 0 833 0 0 0 833 0 0 0 833 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5.30 7.50 5.40 5.30 6.40 5.30 6.30 6.30 7.40 7.10 7.10 7.10 7.10 6.50	33.60 32.30 33.40 33.40 33.40 36.30 37.40 37.20 36.80 37.20 36.80 35.30 35.30 35.30 35.30 35.30 35.30 35.30 35.30 35.30 35.30 36.80 35.30 36.80 36	836.17 836.79 837.126 838.25 837.42 837.42 837.25 836.37 836.37 836.37 836.37 836.37 836.48 836.21 836.71 836.71	F F-C F1 59-93 59-64 61-49 62-45 64-88 65-35 65-06 64-88 65-27 65-12 65-54 67-37 67-35 67-35 67-37	\$46.60 \$46.30 \$49.40 \$46.45 \$46.45 \$46.35 \$46.35 \$46.45 \$47.70 \$47.65 \$47.65 \$47.65 \$47.65 \$47.65 \$47.65	H G—C Pt. 70-46 70-46 71-72 71-78 71-78 72-74 72-74 72-74 73-74 74-73 75-40 76-98 76-98 76-98 78-41 76-98 78-41 78-81 79-96	Tons. 177 177 177 177 177 177 177 177 177 17	Tons. 34 34 34 34 34 34 34 34 34 34 34 34 34	Tons. 830 830 830 830 830 830 830 830 830 83	Ton- 198 198 198 198 198 198 198 198 198 198	Tons: 13 13 13 13 13 13 13 13 13 13 13 13 13	. Tons 2541 2541 2541 2541 2541 2541 2541 2541	Tons. 20 20 21 21 21 22 22 22 22 22 22 22 22 22 22	Tons. 807 866 916 916 916 916 916 1007 1142 1239 1239 1239 1249 1259 1259 1259	Tons., 952 944 1015 1040 1055 1127 1169 1177 1208 1218 1222 1292 1292 1284 1277	279 297 325 321 321 331 318 322	Tons. 5858 5997 6025 6035 6053 6104 6134 6258 6326 6408 6528 6587 6593 6595 6674 6688 6699 6703	K Ubs: 31. 31. 32. 32.5 32.5 32.5 32.5 33.5 33.5 33.5	L 1,bs., 30.57 30.48 31.02 31.05 31.47 32.17 32.17 32.32 32.45 33.39 33.39 33.39 33.39 33.39	M L×A Tons. 3254 3315 3315 3315 3353 3452 3455 3455 3455 3455 3455 345	N 1—M Tons. 2594 2052 2713 2720 2738 2741 2763 2823 2823 2891 2956 3001 3045 3037 3037 3037 3059 3050 3051	8376 8340 85544 8625 8659 8834 8962 8936 8936 8936 9052 9052 9052 9053 9173 9173 9173	P N+O Lbs. 620 636 634 631 622 620 639 647 651 620 647 651 668 681 671 671 671 671 671 671 671 671 671 67	Clay. Stopped clay husst from 330 p. m. to 7 45 p. m. Gravel and Coarse Sand at 12.0 p. m. Stopped at 15.0 p. m. and started at 10.20 a. m. Stopped at 15.0 p. m. and started at 10.20 a. m. Started at 2.0 p. m. and stopped at 6 30 p. m. Started at 3.0 p. m. and stopped at 6 30 p. m. Started clay bost at 45 p. a. m. Stopped at 18.45 a. m. and started at 3 p. m. Working to Pter II. Started clay bost at 9.0 a. m. Stopped at 3.5 a. m. and started at 12 m. Stopped clay host at 8 a. m. and started at 12 m.
768.02 767.56 767.54 767.52 767.52 767.52 767.52 767.54 766.16 763.71	767776776776776776776776776776776776776	7-45 7-46 7-47 7-47 7-47 7-47 7-47 7-45 5-67	767 - 50 767 - 49 767 - 47 767 - 47 767 - 47 767 - 48 — — — — — — — — — — — — — — — — — — —	768.00 767.4 767.4 767.4 767.4 767.4 766.3 766.3 765.7 765.6	6 76 76 76 76 76 76 76 76 76 76 76 76	97.50 97.49 97.48 97.48 97.48 97.48 97.46 97.46 97.21	0.02 0.25	836.00 835.70 835.60 836.00 836.00 835.70	\$32.76.28 \$37.66.28 \$33.48.28 \$32.46.28 \$32.36.28 \$32.76 \$32.36.28 \$32.76 \$32.36.28 \$32.76 \$32.36.28 \$32.36 \$32.3	o 53,000	1,20 5,20 5,50 5,50 5,30 5,30	633.10 333.40 332.80 333.60 333.70 330.30 		67.64	847 - 25 847 - 25 847 - 35 847	79-25 79-70 79-86 80,07 80,07 79-99	177 177 177 177 177 177 177 177 177 177	34 34 34 34 34 34 34 34 34 34 34 34 34 3	\$300 \$300 \$300 \$300 \$300 \$300 \$300 \$300	11) file (15) fi	13 13 13 13 13 13 13 13 13 13 13 13 13 1	2541 2541 2541 2541 2541 2541 2541 2541	22 22 22 22 22 22 22 24 25 16 16 16 16 16 16 16 16 16 16 16 16 16	(259) (259)	247 247 267 329	339)	6169	35.5 35.5 35.5 35.5 36.5 ————————————————————————————————————	34.28	3682 3682 3742 3742 3742 3742 3889	3025	1466 4606	673	Stopped clay hoist at 2.55 p. m. Cut off air at 5.55 p. m. Work subjected. River breaking up. **Top of the state of the s
764-58 764-59 764-59 764-54 764-54 764-54 764-54 763-55 763-57 763-54 763-54	764 764 764 764 764 763 763 763	1-54 1 1-54 1 1-51 7 1-50 7 1-50 7 1-50 7 1-50 7	63.62	764.66 764.66 764.66 764.65 764.65 763.63 763.65 763.65	76: 76: 76: 76: 76: 76: 76: 76: 76: 76:	4.58 4.59 4.47 3.95 3.57	0.00								854.30 853.40 853.75 851.75 851.59 851.59 851.59 851.59 853.50 85	87.15 86.54 86.54 84.50 84.50 85.63 85.63 85.63 85.63 85.63 86.25 86.25	177 177 177 177 177 177 177 177 177 177	34444444444444444444444444444444444444	850 850 850 850 850 850 850 850 850 850	198 198 198 198 198 198 198 198 198 198	(3) (3) (3) (3) (3) (3) (3) (3) (3) (3)	2541 2541 2541 2541 2541 2541 2541 2541	16 16 16 16 16 16 16 16 16 16 16 16 16 1	1259 (259) (394 379 379 379 345 355 355 377 377	1096 1040 1043 1068 1077 1077 1077	6558 6533 6443 6462 6489 6504 6504 6528 6528	36.5 36.5 36.5 36.5 36.5 36.5 36.5	36.43 36.59 36.59 37.03 37.03 37.03 37.03 37.03	38907 38907 38907 3896 3986 3986 ————————————————————————————————————	6558 6533 6533 25555 2599 2543 2543 2543	5382 	2404 2427 2427 	Put on arrat 335 p. m. Started clay hout at 1745 a. m. Sacking out from 4 p. 10. to 530 p. n. Finished scaling causon at 1 a. m. Cut off air at 810 a. m.

RECORD OF SINKING CAISSONS. PIER IV.

16	LEVATION	es or Cur	TING ED	. Vr		1	ELRVA	TIONS OF	-=		Averuge			1				Wgg	GHTS					AIR P	RESSUEE.	T	T		Average Weight	
N E.			S, W	_	Sunk In 24 Hours	N. F	N W.		S, W,	2	Average Pene- tralion of Casson.	Waler Gauge	Depth Immersed.		Caisson			Crib	Concrete.	Air Lock, Shaft,	Masonry,	Sand.	Total.	Indicated.	Culculated	Reaction due to Air Pressure	Nel Weight	Surface th Conlact	per sq it on Surface Exposed Joinetian	REMARKS.
846.99 844.69 844.69 843.74 842.62 842.61 841.21 839.70 836.87 836.48 833.24 833.27 830.49	847 - 11 844 - 88 844 - 88 843 - 78 842 - 36 842 - 36 841 - 36 841 - 36 847 - 05 837 - 05 837 - 05 837 - 04 837 - 04 837 - 04 837 - 05 837 - 04 837 - 05 837 - 05 85 85 85 85 85 85 85 85 85 85 85 85 85	847.05 845.43 845.43 843.97 843.03 841.92 840.42 840.42 840.50 836.50 836.54	847.05 845.72 845.72 844.01 842.99 841.91 840.52 840.34 836.93 836.93 833.77	847.0 845.1 845.1 843.8 842.7 841.6 838.6 838.6	D F1. 58 1.87 8 8 0.00 3 1.35 5 5 0.00 0 1.15 1 1 1.48 4 1.47 7 0.17 7 0.17 7 0.17 7 0.17 4 0.32 1	843.9x 844.8s	844.90		844.90	E 847.00 847.00 847.00 847.00 846.80 846.90 845.90 845.90 845.90 845.00 844.40 844.40 844.40	F E—C F1. ———————————————————————————————————	846.20 845.80 845.80 845.85 845.70 845.60 845.40 845.30 845.10 845.20 845.20 845.30	H G—C Ft 	% Tons. 155 155 155 155 155 155 155 155 155 15			Tons. — — — — — — — — — — — — — — — — — — —	П		Tons. 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	Tons	Tons.	1 Tons. 192 193 290 439 513 597 643 653 668 672 686 692 876 887	K Lbs. — — 2. 2. 3.7 4. 5. 9 5.9 b. 6.7 8.	L Lbs	K L×A Tons. 126 122 125 219 275 345 348 475 579	N IK Tons, 192 290 429 387 475 478 433 393 327 338 217 391 308	O Sq. Ft. 295 295 486 621 619 703 889 1879 1291 1285 1560 1714 2131	P N+O Lbs.	Started air pumps at 7-05 a.m. Waier pumps at 7:30 a.m.
827.99 821.78 817.32 817.32 817.39 810.06 804.43 801.99 798.59 798.59 798.59 795.50 795.50 795.50 794.18 794.07 793.99 794.74 792.74 793.95 790.95 789.55 789.55	8ay. 84 821.93 817.45 817.45 817.45 810.13 804.31 804.31 801.97 798.54 779.89 779.82 779.83 779.86 779.87 779.86 779.86 779.86 779.86 779.86 779.86 779.86 779.86 779.96 779.96 779.96 779.96 779.96 779.96 779.96 779.96 779.96 779.96 779.96	828.09 821.37.31 817.33 817.42 809.98 806.03 802.89 800.02 799.01 798.78 798.78 794.27 795.73	828.13 821.70 817.66 817.63 817.62 810.42 810.42 800.15 798.91 798.51 798.57 796.86 795.53 794.38 794.38 794.38 794.38 794.11 793.03 791.18	821.7.4 817.4.4 817.4.4 817.4.4 817.4.4 805.2 802.4 709.8.7 798.7 798.7 794.1 794.1 794.1 794.1 794.2 791.6 791.6 791.6 791.6	6.31 4.25 5.30 4.25 5.30 4.25 5.30 4.25 5.27	843.0	845-49 845-95 845-43 845-43 845-43 845-31 845-45 845-20 845-20 845-20 845-20 845-20 845-20 845-20 845-30 845-85 85	846.31 846.82 844.92 844.43 844.43 844.51 844.51 844.51 845.73 845.73 845.73 845.75 845.75 845.75 845.75 845.80 845.80 845.80 845.80 845.80 845.80 845.80 845.80 845.80 845.80 845.80 845.80 845.80	845.35 845.05 845.61 845.61 845.03 845.03 845.50 845.50 845.16 845.16	844-32 844-65 844-65 845-19 845-01 845-01 845-69 845-69 845-64 845-64 845-64 845-64 845-65 845-64 845-11 845-11 845-11	17.77 23.27 27.86 28.70 33.50 41.87 46.25 46.25 46.25 46.25 46.13 46.25 51.41 51.52 51.51 51.52 51.52 51.52 51.53	841,80 841,80 841,80 841,70 841,70 841,70 841,70 841,85 84	16.79 23.10 27.352 27.451 34.473 34.473 34.473 44.595 46.902 47.931 49.351 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.23 50.43 50.	155 155 155 155 155 155 155 155 155 155	30 30 30 30 30 30 30 30 30 30 30 30 30 3	443 443 443 443 443 443 443 443 443 443	85 85 86 107 116 138 1174 1196 1196 1196 1196 1196 1196 1196 119	55 66 67 78 99 100 111 111 111 111 111 111 112 122 122	286 309 514 808 1018 1185 1365 1522 1637 1750 1750 1750 1750 1750 1750 1750 1750 1750 1750 1750 1750 1912 1912 1912 1912 1912 1912 1912 1912 1973 1973 2069 2069 2069	7 9 9 10 10 10 10 12 12 12 18 18 18 18 18 18 18 18 18 18 18 18 18		45 75 113 128 195 298 350 350 424 429 447 464 4491 523 525 525 525 571 572 572 573 574 574 624 622	1056 1111 1368 1685 2164 2246 2246 2246 2246 2246 2246 2246	8. 8. 1 12. 13. 2 14. 16. 19. 5 21. 2 20. 5 20. 20. 6 21. 22. 22. 25. 3 25. 24. 24. 25. 25.	7.26 9.99 11.81 11.91 11.85 17.08 18.28 90.41 19.58 20.78 20.78 20.78 20.78 21.33 22.36 23.02 23.02 23.03 23.0	6;22 952 1127 1135 1130 1424 1627 1741 1849 16949 16949 16979 2692 2692 2692 2692 2121 2121 2193 2194 2194 2194 2194 2194 2194 2194 2194	364 159 241 550 777 740 834 955 1032 1014 1122 1113 1116 1088 1092 1076 1213 1221 1189 1189 1189 1183 1193 1221 1183 1193 1221 1183 1193 1221 1183 1193 1221 1183 1193 1221 1183 1193 1221 1183 1193 1221 1183 1193 1221 1183 1193 1221 1193 1221 1193 1221 1193 1221 1193 1221 1193 1221 1193 1221 1193 1221 1193 1221 1193 1221 1193 1221 1193 1221 1221	2684 3499 4167 4297 4325 4042 5801 6119 6679 6719 6719 6719 7446 7254 7431 77446 7793 77802 7802 7802 7802 7803 8181	271 90 115 256 359 287 312 303 334 331 325 312 325 320 325 321 321 321 321 321 321 321 321	Sand. Clay in north end. Put on clay heist. Stopped day heist at 7:20 a. m. Cracked on one side. Clay hoist repaired at 8:15 p. m.
787, 796, 358, 786, 368, 786, 386, 386, 386, 387, 788, 378, 388, 388, 388, 388, 388	786.39 786.39 786.37 784.70 782.99 780.03 781.30 775.86 777.90 775.86 773.74 773.74 773.74 770.28 770.28 770.28 770.40 770.40	0 786.37 786.37 784.64 784.64 784.64 784.64 784.64 784.77 784.77 784.77 784.77 785.77 786.37 786.	788. \$2 784.82 784.79 784.79 782.93 782.93 782.93 781.14 781.13 779.94 7778.60 7778.60 7773.94 7773.94 7771.80 7771.80 7771.80 7771.81 770.49 770.49 770.49	786 786 786 786 784 784 782 782 782 781	1. 0.01 9 0.02 9 0.03 1. 65 1. 70 1. 65 1. 70 1. 65 1. 70 1. 20 1. 20	843-7-7-8-15-09-8-14-7-7-8-8-14-3-8-8-14-3-8-8-14-3-8-8-14-3-8-8-14-3-8-8-14-3-8-8-14-3-8-14-	845,499 845,499 845,350 844,590 844,590 845,300 845,300 845,27 84	844-77 845-04 841-4-1 841-4-1 841-4-1 841-98 843-98 843-98 843-98 843-93 842-93 842-93 842-39 842-39 842-39 842-44 842-48 842-48 842-48 842-48 842-48 842-98	844-52 844-56 844-66 844-44 844-37 844-37 844-37 844-37 843-53 843-53 843-53 843-53 843-53 843-53 843-53 843-53 843-53 843-53 843-53	845,18 844,60 844,60 844,51 844,51 844,51 844,33 844,33 844,33 844,25 844,29	56.86 58.64 58.65 58.57 60.197 60.197 61.71 63.33 64.52 66.35 66.40 68.31 70.47 70.48 72.45 72.45 72.45 73.33 73.34 74.40 75.81 76.73 77.33	844-60 844-75 841-15 84	56. 86 33 58. 135 58. 135 58. 135 58. 135 58. 135 58. 135 58. 135 58. 135 58. 135 58. 135 59. 49 3 61. 05 75 59. 01 135 75 75 75 75 75 75 75 75 75 75 75 75 75	155 155 155 155 155 155 155 155 155 155	88888888888888888888888888888888888888	443 443 443 443 443 443 443 443 443 443	214 218 218 232 232 232 254 254 254 254 257 272 272 272 273 285 293 393 393 393 393 393 393 393 393 393	13 13 13 13 13 13 13 15 15 15 15 16 16 16 16 16 16 17 17 17 17 17 17 17 17	2060 2148 2205 2205 2305 2305 2306 2309 2407 2607 2607 2607 2607 2607 2607 2607 26	19 19 19 19 19 19 19 20 20 20 20 21 21 21 21 22 22 22 22 22 22 22 22 22		622 655 655 654 681 681 681 681 681 710 740 740 740 831 831 831 831 906 902 922 922 942 942 943 943 944 944 945 946 947 947 947 947 947 947 947 947 947 947	3565 3681 3738 3751 3773 3884 4132 4235 4439 4439 44737 44737 44737 44737 5084 4911 5086 4911 5086 5085 5086 5085 5086 5086 5086 5086	25. 25. 25. 25. 25. 25. 25. 25. 25. 25.	24,60 25,24 25,16 25,16 25,77 25,77 26,43 26,43 27,11 27,16 28,43 28,43 28,43 31,35	2343 2104 2106 2106 2106 2152 2152 2152 2152 2153 2153 2153 2153	1222 1277 1332 1354 1353 1453 1453 1465 1654 1666 1652 1663 1670 1683 1851 1858 1954 1994 1994 1995 2006 2013 2007 2007 2008 2007	8133 8129 8320 8319 8852 8853 8853 9276 9240 9763 9240 9763 10061 10061 10061 10158 10158 10158 10158 10158 10158 10158 10158 10158 10161 10191 10191 10193 11012	300 303 316 321 366 3217 325 338 338 338 338 368 368 368 368 368 368	Raised clay hoist, Stopped clay hoist at 5 p. m. Sand and gravel. Sturted water pumps at y a. m. Started elay hoist at 8 p. m. Gravel. South lock crucked at 3 a. m. in corner of inside well. Stopped clay fenst y. a. m. to 9 p. m. Crucked on other side. Stopped work II p. m. Outside well of lock badly cracked. Resumed at 9 a. m. Well lined, with sale, South lock abandoned. North lock bracest. Reached clay on west side.
766.83 766.19 766.19 766.07 765.15	766.13 766.02 765.08	766.14 766.13 766.00 765.07	766,28 726,27 766,14	765.1	0.12	845-3; 845-19 845-19 843-45 842-99	844.12	844.00	843.93 842.98 842.98 843.44 842.91	843.88 843.69 843.69 843.76 843.20	7705 7750 7751 7770 78.07	843.95 843.85 843.86 843.85 843.85 844.00 844.00 844.10	77.12 77.66 77.63 77.79 78.72 — — — — 79.01	155 155 155 155 155 155 155 155	30 30 30 30 30 30 30	443 443 443 443 443 443 443	303 303 303 303 303 303 303 303	17 17 17 17 17 17 17 17	3223 3223 3223 3223 3223 3223 3223 322	22 22 22 22 16 16 16 16	- = = = = = = = = = = = = = = = = = = =	990 998 998 1002 1009 —	5183 5191 5191 5195 5196 —	35·5 35·5 35·5 35·5 35·5 35·5 35·3 35·5	33.37 33.50 33.58 33.66 34.06	3179 3201 3199 3206 3245 — —	2004 1990 1992 1989 1951 —	10970 11032 11033 11059 11110 —	365 360 360 359 351 — —	Took off clay bond, Began concreting at 1125 n, m, Finished dilling calson at 8:50 p, m, Took off ur at 7:50 n, m.

APPENDIX E.

TIME, COST AND MATERIALS USED IN FOUNDATIONS. PIER I.

Date, Princ	ipal N	fight reman, 1	Sub foremen.	Loek Tenders.	Press	sure C	offeehous Men.	e c	offee.	Sug	ar.	Candles.	Re	rd ad	Wiek.	Cond for Heating	r g. 1.6	nserd Oil,	Dilehin	ug E	Day Engineer.	Nigi Engine	ht Fi	iremea	Conl Passers,	Pump Men.	Lah	orers on fossi	Coal for Boolers	Blac	k Sig	nad Tal	low,	Waste. C	Conl Oil,	Totals for Each Day	Feet Sunk per day	Ma- ier- ial. R	irs No	of ol A	ST	
Days. 9881	Amount. Days.	Amount-	Amount.	Days. Ameunt.	Days.	Amount.	Amount.	Lbs.	Amount.	Lbs.	Amount.	Amount.	108	Amount	Amount	Boxes.	Pants.	Amount.	Days.		рауз Атошит	Days	Amount Days.	Amount Days.	Amount ,	Days	Days.	&mount.	Tons.	Punts.	Parts.	Amount. Lbs.	Amount:	Amount.	Amount.							REMARKS.
21 1 6 22 1 6 23 1 6 24 1 6 25 1 6 26 1 6 27 1 6	1 000 I	3.57 3 3.57 3 3.57 3	9.00 9.00	2 4.50 2 4.50 2 4.50 2 4.50	22 4	49.50	3.0	3 3 %	0.28	10	0.63	23 0.28	11/4	0.00		I 0.	31 - 32 - 31 1 32 1 31 -	0.00			2 7.3	D I 3	3.30 2	4.80 2	2, IQ 4, 20 4, 20	2 4.	80 — 80 —	=	4 14. 6 10.	49 8 0	.11 8	0.34 4	0.26 -		11.0	116.78	2.17 3.91 3.45 3.22 0.86 2.26 3.44	4 2 4 2 4 2 4 2 4 2 4 4 2 4 4 4 4 4 4 4	2 ¹ / ₁₂ 2 ¹ / ₁₂ 1 ¹ / ₁₂ 0 ¹ / ₁₂ 3 ¹ / ₁₂	38 — 38 36 88 29 38 32 38 28 39 27 40 15		
2 1 6 6 1 6 6 6 1 6 6 6 1 6 6 6 1 6 6 6 6 1 6	100 1 100 1 100 1 100 1 1	3.22 3 3 3 4 4 4 4 4 4 4 5 3 2 2 4 4 4 4 5 3 2 2 4 4 4 4 5 3 2 2 2 4 5 3 3 2 2 2 4 4 5 3 2 2 2 4 5 3 2 2 2 4 5 3 2 2 2 2 4 5 3 2 2 2 2 4 5 3 2 2 2 2 4 5 3 2 2 2 2 4 5 3 2 2 2 2 2 4 5 3 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	6.50 13.00 13.00 13.00 13.00 13.00 13.00 16.25 19.50 19.50 19.50 19.50 19.50 19.50	2 4.50 2 4.50 2 5.00 2 5.00 2 5.00 2 5.00 2 2 5.00 2 5 5 5.00 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	27 27 27 27 35 33 33 33 33 33 33 34 46 11 13 13 13 13 13 13 13 13 13 13 13 14 15 15 15 15 15 15 15 15 15 15 15 15 15	61:50 61:50 70:75 87:50 87:50 82:50 82:50 82:50 45:00 40:00 86:00 86:00 86:00 86:00 86:00 87:50 87	2 3.00 2 3.00	224252522 425655555555555555555555555555555555555	0.32 0.25 0.50 0.52 0.52 0.53 0.32 0.32 0.57 0.57 0.57 0.57 0.50 0.63 0.63 0.63 0.67 0.67 0.70 0.70	\$ 50 10 10 5 10 5 10 5 10 5 10 5 10 5 10	0.32 0.63	29 0.36 27 0.34 42 0.53 32 0.49 42 0.53 38 0.45 88 0.45 88 0.23 31 0.39 27 0.09 13 0.16 6 0.08 6 0.08 4 0.05 5 0.06 6 0.07 5 0.06	5	0.30 } 0.30 } 0.30 } 0.30 } 0.30 }	6 0.10 6 0.09 7 0.10 9 0.10	I O.	2 - 2 - 1 - 1 - 1 - 2 - 1 - 2 - 1 - 1 -	0.13 0.12 	9' 13 19 19 19 19 19 19 19 19 19 19 19 19 19	.50 .60 .62 .44 .41 .61 .61 .61 .62 .62 .63 .61 .62 .62 .63 .63 .63 .63 .63	2 2 7 7 3 3 2 2 7 7 3 3 3 2 7 7 3 3 3 2 7 7 3 3 3 2 7 7 3 3 3 2 7 7 3 3 3 2 7 7 3 3 3 2 7 7 3 3 3 2 7 7 3 3 3 2 2 7 7 3 3 3 2 2 2 2	0 1 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	1.30 2 1.30 2 1.	4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 5.40 2 5.	2 4,20 2 4,20 2 4,20 2 4,20 3 4,20 4,20 4,20 4,20 5 4,20 6 4,20 6 4,20 7	2 4. 4. 2 4. 4. 2 2 4. 4. 2 2 4. 4. 2 2 4. 4. 2 2 4. 4. 2 2 2 4. 4. 2 2 2 4. 4. 2 2 2 4. 2 2 2 4. 2 2 2 4. 2 2 2 4. 2 2 2 4. 2 2 2 4. 2 2 2 4. 2 2 2 4. 2 2 2 4. 2 2 2 2		4.47 9.39 1.40 6.30 5.74 4.99 6.16 10.22 14.72 16.07 15.82 13.44 14.00 10.08 12.88 13.58 13.44 14.10	7 22. 22. 65 21. 55 66 21. 55 20. 55 21. 55 20. 55	55 8 6 6 7 7 8 8 6 7 7 7 9 16 2 18 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	.1122	- 9 - 10 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	0.59 0.78 0.26 0.26 0.59 0.59 0.59 0.33 0.52 0.59 0.33 0.52 0.13 0.26 0.13 0.26 0.13 0.26 0.13 0.26 0.13 0.26 0.13 0.26 0.13 0.26 0.27 0.13 0.26 0.13	1 0.07 1 0.07 1 0.07 1 0.07 1 0.07 1 0.07 1 0.07 1 0.07	8 0.11 8 0.11 8 0.11 8 0.11 8 0.11	149, 90 154, 94 156, 19 169, 28 170, 62 173, 12 170, 41 172, 40 52, 43 176, 82 176, 83 176, 83 176, 95 174, 42 177, 96 218, 80 217, 31 223, 177 216, 08	4.777 4.52 2.03 2.14 0.03 1.72 0.00 0.02 0.00 1.44 0.02 2.25 2.13 2.54 2.29 1.44 0.28 0.60 0.01 0.00 0.01 0.00 0.01 0.00 0.01 0.00 0.01	6 2 6 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	70 38 44 32 18 18 18 14 24 27 27 27 29 29 24 28 26	Ci	lay hoist substituted for Cameron unp men running clay bosst.
2 I 6 3 I 6 4 I 6 5 I 6 7 I 6 8 I 6	.00 I .00 I .00 I .00 I .00 I	3.33 6 3.34 6 3.33 6 3.34 6 3.33 6	19.50	2 5.00 2 5.00 3 5.00 2 5.00 2 5.00 2 5.00	54 13 55 13 55 13 55 13 56 14 53° 13	35.00 37.50 37.50 37.50 37.50 40.00	3.00 3.00 3.00 3.00 3.00 3.00 3.00	5 5 % 5 5 % 5 4 %	0.70 0.73 0.74 0.63 0.64 0.35	10	0.63 0.63 0.63 0.32	10 0.13 2 0.03 6 0.08 12 0.15 — — 14 0.30				I 0,	31 -	=	= =	- 1	2 7.3	O I 3	3.30 2	4.80 2 4.80 2	4.20 4.20 4.20 4.20 4.20 4.20 4.20 4.20	2 4	40 5 ⁸	2.80	5" 18, 3" 12, 5" 16, 3" 10, 2" 9, 2" 9,	27 16 0 29 — 07 — 40 — 14 16 0	.22 8	- 4 - 2 - 4 - 2 0.34 2 - 2	0.26 0.13 - 0.26 - 0.13 - 0.13	I 0.07 -	8 0.11	210.98 230,26 224.12 221.58 206.74 197.14	0.00 0.71 1.00	66 68 68 68 68 68		- 40 - 37 - 37 - 27 - 31 - 28 - 20	7 7 1 B	eached gravel in test pit. egan sealing caisson at 945 p. m. finished sealing caisson at 5 a. m. [Took air off at 221 p. m.
29	4.00	162. 13	690.25	239.00	46	18.37	145.50	,	24.31	:	32.10	8.86		6.03	0.57	t4.	35	2.32	251	1.62	367.7	155	9.90	241.80	203.70	187.	20	286.85	737		96	.88	16.99			\$693.44 \$2229.48	11					1200k au Oli at 2.13 ja an

TIME, COST AND MATERIALS USED IN FOUNDATIONS. PIER II.

Date.	Principa Forema	d. Ni	ight eman,	Sub Foreuter	Loc	k er-	Prussure Men	Coff Itou Me	tee aso	Coffee.	Sugar.	· -m-	dles R	ed Lead.	Wick.	Conl for heating 200 lbs.	Linseed	Da Luga	y 1 cers, En	Night	Firemen	Con) Passers	Pump Men	La	porers Hoist	Coal for Boilers.	Signal O	al- 19ack	oa. T	nllow	Waste.	o d Od,	Total for Each Day	Feet Sunk per day.	Mate- rial.	Camer	ron is 1	Acr Pumps	Renarks.
1887.	Days.	Days.	Amount.	Amount.	Days.	Amount.	Amount	Days.	Autount	Amount	Amount	ž	Аточис	Amount	Amount.	Boxes Amount.	Pints.	Days.	Amount. Days.	Amount	Amqunt.	Days.	Days.	Days.	Amount.	Amount.	Plats.	Fints.	£.85x	Amount Lbs.	Amount.	Amount.				Hours re	o, of evolu- ions.	No, of revolu- lions.	I INDIVARIA
Feb. 10 11 12 13	1 4.4 1 4.4 1 4.4	17 - 17 - 17 -		A (.6	3 -	- 54 - 15 - 2	41 54.17 9 40.45 8.00	7 -					_ 4	2.4p			16 1.00			Ē								16 o. 16 o.	24 - 24 - 24 -			=	62.28 46.79 12.71	7.56 4.67 0.86 0.64 0.02	=	= '	ΞΙ	Ξ.	Carpenters and laborers began lowering caisson.
15 16 17 18	6.6	00 I	3.57 3.57	3 9.0 3 1.9	p - p 2 4 p 1 2	.50 28 .25 I	5 (1.3) 8° 64.13 5 33.7!	5 - 2 3 3 5 []	3.00	3 0.65 3 0.67	5 0.30 4 0.24 	45 18	0.56	0.18	五 元 0.02 元	3 [.03	1 0.00	5 - 7	1 00.	3.30	2 4.80 1 2.40	2 4.20 [2.10	2 4.8 1 2.4 	000	= 4	14.28	8 0.3	8 0.	12 8	0.50	0.07 -	0.11	17.11 32.57 169.69	0.99 6.00 0.02 —	Sand			_	Working in P. III.
20 21 22 23 24 25 26 27 28	I 6.0 I 6.0 I 6.1	00 - 00 I 00 I - I	3. 58 3. 57 3. 57	0° 1.3 2 6.6 3 9.6 1 3.6 	O I 20 I 2	.25 4 .25 17 .25 3: .25 16	4 9.00 7 38.7 2 72.00 0 32.50 	5 2 3 5 0 2 5 0 1 1	1 50 3.00 3.00 1.50	1 0.32 4 0.90 2 0.45 2 0.45 	2 0.12 10 0.60 5 0.30	30 14 1	0.37 3	0.12	9.05 0.05	68 	% o.o.	1 2 7 5 2 7 2 7	1.30 - 1.30 I 1.30 I - I	3.30 3.30 3.30 	1 2.40 2 4.80 2 4.80 (2.40 	1 3.10 2 4.20 2 4.20 1 2.10	2.48		- 6 - 6 - 6	23.46 22.78 23.46 23.46	8 0.3	8 0.	12 8 13 - 12 8 12 - - - - -	0.50 1	0.07		62.28 46.79 12.71 17.11 32.57 16g.69 50.75 107.73 145.60 07.29	5.59 3.91 0.01	coarse sand & rub-bish	21%	40 35 — — —	33 25 —	Working in P. III.
Mar 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15																											8 0.33 8 8 0.33 8 8 0.33 8 0.33 8 0.33							3,:30 1,:51 1,:55 2,773 1,:36 0,:25 1,:93		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	344 40 40 337 337 38 36	344 277 233 225 2319 118 266	Work suspended. River breaking up. "" "" "" "" "" "" "" "" "" "" "" "" "
30 31 Apr 2 3 4 5 6 7 8 9 10 11 12	[6, 1 6, 1 6, 1 6, 1 6, 1 6, 1 6, 1 6,	00 I 00 I 00 I 00 I 00 I 00 I 00 I 00 I	3.22 3.33 3.33 3.33 3.34 3.33 3.33 3.34 3.33 3.33 3.34 3.33 3.33 3.34	1 3 3 6. 4 13 4	25 3 2 2 1 2 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2	.00 - .00 - .00 - .00 4 .00 4 .00 4 .00 4 .00 4 .00 4 .00 4	1 27.5 2 55.0 6 115.0 6 115.0 6 115.0 6 115.0 6 115.0 6 115.0 6 115.0 6 115.0 6 115.0 6 115.0	0 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	3.00 3.00 3.00 3.00 3.00 3.00 3.00 3.00	2 0.47 4 0.94 4 0.94 3 0.70 3 0.71 5 1.17 4 0.94 3 0.71 3 0.71 4 0.94 3 0.70 3 0.71 4 0.94 3 0.70 3 0.71	8 0.52 6 0.39 8 0.52 6 0.39 8 0.53 8 0.53 8 0.39 6 0.39 6 0.39 6 0.39 6 0.39 6 0.39	3770 - 6630 5	0.03 0.04 0.09 0.13 0.01 0.07 0.08 0.25 0.05	0.24	§: 0,16	3 1.0234 1 0.34 4 1.36 4 0.89	2 0.1		7.30 I 7.30 I 7.60 I 7.60 I 7.60 I 7.60 I 7.60 I 7.60 I 7.60 I 7.60 I 7.60 I	3.30 3.30 3.30 3.30 3.30 3.30 3.30 3.30	2 4.80 2 5.80 2 4.80 2 5.80 2	2 4.30 2 4.30 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 1 2.40 1	D I 2.4 D I 3.4 D I 3.4 D I 2.4 D I 2.	10		23.4 23.4 23.4 23.4 22.4 23.4 24.4 25.5	5 8 0.3 5 8 0.3 5 8 0.3 5 9	4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	00 - 00 - 00 - 00 - 00 - 00 - 00 - 00	0.50	0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 - 0.07 -	0.11	64.68 64.76 139.17 195.46 184.61 173.94 189.27 187.65 187.41 150.14 69.34	0.72 1.30 1.96 0.57 1.20 3.20 2.41 1.31 0.01	sand	23°/12 2334 2323 18°/12 20°/2 23°4 23°4 23°4 23°4 23°4 23°4 23°4 23°	_	25 24 24 26 27 18 21 27 21	Pressure men concreting. Wurking in P. III.

APPENDIX E.—Continued.

PIER II.—CONTINUED.

Dale Foreman, Foreman, Foreman	I.ock Pressure λlen.	Coffee house Men.	Coffee.	Sugar,	Candles. Red L	ead Wick	Coal for heating, goallis.	Linsced Oil.	Day Engineers,	Night Engineer.	Firemen.	Coal Passors.	Pump Men.	Laborers on horst.	Coal for Boilers,	Signal Oil.	Black Oil-	Tallow.	Waste.	Coal Orl.	Totals for each day.	Feet sunk per day.	Mate- rial.	Camerou Pumps.	Air Pumps	Remarks.
Days. mount. Days.	Days. Days	Days.	Lbs.	Libs. mount.	No. mount. Lbs.	Lbb.	Boxes.	Pints.	Days.	mount.	mount.	Days.	mount.	Days.	Tons.	Pints.	Plats.	Lbs.	Lbs imount.	tmount.			1	Hours No. i	No. of revolu- tions.	
April,	75 2 5.00 32 80.00 23 80.00 24 105.00	2 3,00 2 3,00 2 3,00 2 3,00	2 0.47 3 0.70 4 0.91	- 4 	5 0.06 4 0.01 10 0.13 - 6 0.07 -	224 1/4 0.0	2 0.45	2 0.12	2 7.60 2 7.60 2 7.60 2 7.60		3 4.80	2 4.20 2 4.20 2 4.20 2 4.20 2 4.20	2.40		5 11.35 5 11.35 5 17.00	8 0.34					145.87 188.85 150.71 194.70	0.02 0.03 1.19 0.54	=			Working in Pier III.
Mays. 1 6.00 3-32 4 33 2 1 6 6 1 3-32 4 3 3 1 6 6 1 3-32 4 3 3 1 6 6 1 3-32 4 3 3 1 6 6 6 6 6 6 6 6 6	, 25	2 3,25 2 3,00 2	4 0.94 3 0.70 3 0.71 4 0.94 3 0.70 8 1.27 5 1.28 5 1.28 6 1.53 7 1.78 5 7 1.78 5 1.26 1 1.02 4 1.02 4 1.02 4 1.02	6 0.38 6 0.37 8 0.59 6 0.38 6 0.35 6 0.35 6 0.37 8 0.59 6 0.37 8 0.59 6 0.37 8 0.59 8 0.59 6 0.37 8 0.59 8	6 0.05 - 2 0.03 - 2 0		3 1.02 3 1.02 3 1.02 1 0.34 1 0.34 1 0.34 1 0.34 1 0.34 1 0.34 1 0.34 1 0.34 1 0.34 1 0.34	2 0.12	2 7.60 2 7	1 3 30 1 3 3 3 1 3	2 4.80 4.80 4.80 4.80 4.80 4.80 4.80 4.80	2 4,20 2 4,20 3 4,100 4,20 2 4	4 80 4 80 4 80 1 2 40 1	9 ⁸ 16.32 9 ⁹ 16.32 9 ⁹ 16.32 9 ¹ 16.25 11 19.74 12 21.72 12 21.06 11 ⁴ 19.44 9 ⁹ 16.32 9 ⁹ 16.32 9 ⁹ 16.32 9 ¹ 16.32 9 ¹ 16.32 9 ¹ 16.32	\$\frac{4}{4}\$ \ \\ \text{15.98}\$ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\	8 0.34 8 0.34 8 0.34 8 0.34 16 0.68 5 0.34 16 0.68	8 0.10 8 0.11 8 0.10 8 0.11 8 0.10 8 0.11 8 0.10 8 0.11 8 0.10 8 0.11 8 0.11 8 0.12 8 0.12 8 0.12 8 0.12 8 0.12		I 0.07 - I 0	0.11	248.33 254.42 252.38 257.19 256.34 252.55 239.29 230.81 241.09 256.11	0.17 0.92 0.46 0.35 0.21 1.53 0.21 1.69 0.05 0.01 0.05 0.03 0.23 1.25 0.32 0.34 0.11 0.44 1.69 0.05 0.05 0.01 0.05 0.04 0.05	eccecccccccccccccccccccccccccccccccccc		26 21 22 22 14 13 16 16 23 22 25 31 21 21 26 28 28 22 25 28 22 22 22 22 23 24 24 26 26 27 28 28 28 28 28 28 28 28 28 28 28 28 28	
2 1 0.00 1 3-33 5 10. 3 1 6.00 1 3-33 5 10. 4 1 6.00 1 3-33 5 10. 5 1 6.00 1 3-33 5 10. 6 1 6.00 1 3-33 5 10. 6 1 6.00 1 3-33 6 10. 7 1 6.00 1 3-33 6 10. 9 1 6.00 1 3-33 6 10. 10 1 6.00 1 3-33 6 10. 11 1 6.00 1 3-33 6 10. 12 1 6.00 1 3-33 6 10. 13 1 6.00 1 3-33 6 10. 14 1 6.00 1 3-33 6 10. 15 10 10 10 10 10 10 16 10 10 10 10 10 10 17 10 10 10 10 10 10 18 10 10 10 10 10 10 19 10 10 10 10 10 10 10 10	15 1 5 000 59 115,357 15 2 5 000 59 115,357	2 3.25	6 1.65	8 0.50	10 0.13 4 0.0 16 0.20 - 3 3 0.04 - 8 8 0.10	224 1/2 0.00	4 1.36	2, 0.12	2 7.60 2	1 3.30 1 3.30	4.80 4.80 4.80 4.80 4.80 4.80 4.80 4.80	2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20	2 4.80 2 5.80 2	14' 24.48 14' 24.48 14' 24.48 13' 22.41 9' 16.32 7' 12.24 7' 12.24 7' 12.24	5 17.00 5 17.00 5 17.00 5 17.00 4 15.64 17.00 5 17.00 5 17.00 5 17.00 5 17.00 5 17.00 5 17.00 5 17.00 5 17.00	8 0.34 	8 0.12 8 0.12		1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 - 1 0.07 -	0.11	250.19 250.42 246.26 239.98 235.91 237.89 240.90 230.95 227.93 233.40 221.45 217.50 214.29 205.24 211.38 216.21 211.54	1.16	cclay;		33 33 34 33 26 27 26 24 18 27 29 25 23 21 22 23	Began scaling crisson. Took off air at 7:05 a. m.
505.41 276.39 1131.		258.25		34.70		.56 0.6		1.88	625.75	279.40	399.00	349.65	334.80	819.06	533,25		10.29	6.30	6.02 0 smking		16259.70					
																	Japonoco					\$18386.8	4			

TIME, COST AND MATERIALS USED IN FOUNDATIONS. PIER III.

Dale,	rincipal oreman, P	Night oreman,	Sub Foremen	Lock Tenders.	Pressure Men.	Coffeeho Men	ouse C	oftee	Sugar.	Candles	R	nd.	Nick	Coal for Brating. 200 lbs.	Linser	d Do Engir	av 1 ioer En	Night ighter	Piremen	Cord Passers	Pump Men.	Lahorers on Hoist.	Coal for Boilers,	Signal Oil-	Black Oil.	allow.	Waste. C	onl Oil.	lulals for ach Day.	Feel Sunk per day	Ma- ter- ial.	Water Pumps.	Air Pumps.	
1886	Amount.	Amount.	Amount	Onys Amount.	Days.	Days	Amount.	Amount	Lbs	No.	Lbs.	Amonnt. Lbs.	Amount.	Boxes.	Pents Amount.	Days.	Amount Days.	Amount.	Antount.	Amount.	Amount.	Days.	Tons.	Fints.	Amount.	Amount	Amount.	Amount.			H	Irs. revo	No. of revolu- tions.	Remarks.
Nov. 23 24 25 25 27 28 29 30	6.00	3.33									40	2.40 —			16 1.0	xo -									16 0.20 - 16 0.20 - 16 0.20 -				110.80 105.14 80.03	4.70 8.80 2.60 0.28 0.32 0.00 0.00			=======================================	Lowering casson, Began concreting in a. m.
Dec. 1 2 3 4 5 6 7 8 9 10 11 12 13 14 14 16 16 17 18 22 23 24 25 27 30 31 387	1 6.00 1	1 3.23 1 3.22 1 3.22	1 5-25 5 9.00 1 3.00 1 1 3.00 1 2 0.00 1 3.00	2 4.50 2 4.50 2 4.50 2 4.50 2 4.50 3 6.75 2 4.50 2 4.50 2 4.50 2 4.50 2 4.50 2 4.50 2 4.50	— — — — — — — — — — — — — — — — — — —					20 0.2 11 0.1 1 0.0 1 2 0.0 0 0.2 1 0.0 0 0.2 1 0.0 0 0.2 1 0.0 0 0 0 0.2 1 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0.12 M = 0.1	0.05	4 1.36 4 1.36 4 1.36 4 1.36 5 1.70 5 1.70 4 1.36 4 1.36 4 1.36 4 1.36	\$ 0.0	A 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7.30 I 7.30 I	3 30 3 30 3 30 3 30 3 30 3 30 3 30 3 30		1.93 4.20 4.20 4.20 4.20 4.20 4.20 4.20 4.20	2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		15 56-75 15 56-75 15 56-75 15 56-75 15 56-75 15 15 56-75 15 15 15 15 15 15 15 15 15 15 15 15 15						36.75 36.75 81.39 81.39 84.39 54.86 54.86 149.15 149.15 149.15 149.19 149.39 157.19 149.39 157.19 149.39 157.19 149.39 157.19 149.59 157.19 15	0.08	_]	= =	16 13 18 16 27 22 22 30 21	
Jani. 1 2 3 4 5 6 7 8 9 10 11 12 3 14 5 6 7 8 9 10 11 12 3 14 5 10 20 21 22 22 24 25 27 8 29 30 31	1 6.00 1 6.00	1 3.23 1 3.22 1 3.23 1	3 9,75 3 9,75 4 13,00 4 13,00 4 13,00 4 13,00 4 13,00 4 13,00 4 13,00 4 13,00 1 13,	2 5.00 2 5.00	13 33.75 8 8 2 20.00 13.15 13.		25 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0.40 0.40 0.60 0.60 0.40 0.80 0.40 0.80 0.60 0.60 0.60 0.60 0.60 0.60 0.60 0.70 0.80 0.70	4 0.24 4 0.24 5 0.38 6 0.38 8 0.39 8 0.39 8 0.39 8 0.39 8 0.39 8 0.02 4 0.24 4 0.24 4 0.24 4 0.24 4 0.24 5 0.30 10 0.60 10 0.6	2 0.0 6 0.0 6 0.0 7 0.0 10 0.1 3 0.0 6 0.0 6 0.0 6 0.0 6 0.0 10 0.1 3 0.0 9 0.	3 - 2 - 3 - 4 - 5 - 2 - 5 - 7 - 7 - 7 - 7 - 7 - 7 - 7 - 7 - 7	0.12	0.05	3 1.02 4 1.36 4 1.36	\$ 0.0	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7,30 I	3.30 3.30 3.30 3.30 3.30 3.30 3.30 3.30	2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 4 80 2 2 2 2 4 80 2 2 2 2 4 80 2 2 2 2 4 80 2 2 2 2 4 80 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	4.20 4.20	2 4.80 2 4.80 2 4.80 2 3.80 2 3.80 2 4.20 1 2.40 1 2.40 1 2.40 1 2.40 1 2.40 1 2.40 1 2.40 1 2.40 2 4.80 2 4.80	81 11.756 11.756 12 20.40 14 83.60	6' 20.74 5' 19.72 6' 20.74 6'	8 0.34 8 0.34 8 0.34 8 0.34	\$ 0.10 8 \$ 0.10 - \$ 0.10 4 \$ 0.10 8 \$ 0.10 - \$ 0.10 -	0.52 	1 0.07 - 1 0.07 -	0.11	102.38 93.66 117.39 181.20 187.22 169.36 201.20 204.24 201.27 192.73 193.13 195.51 195.51 193.13 195.51 193.13 195.51 245.36 245.36 255.72 245.30 255.72 256.24 257.90	0.26 0.01 2.05 1.16 s 1.16 s 1.00 0.03 0.07 1.03 0.09 1.89 0.02 0.02 0.02		36/m 455 5/m 366 5/m 366 5/m 366 367 367 367 367 367 367 367	19 14 12 13 14 18	Pump men running day hoist.
Feb. 1 2 3 4 5 6 7 8 9 10	I 6.00 I 6.00 I 6.00 I 6.00 I 6.00 I 6.00 I 6.00 I 6.00 I 6.00	1 3.57 1 3.57	5° 17.88 4 13.00 6 19.50 6 19.50 5° 17.87 6 19.50 6 19.50 6 19.50 6 19.50 6 19.50	2 5.00 2 5.00 2 5.00 2 5.00 2 5.00	62° 156.25 61° 153.75 61° 153.75 61° 153.75 62 155.00 61 152.50 61 152.50 68 145.00 68 145.00 69 146.25 60° 151.25 59° 148.75	2 3 3 2 3 3 2 3 3 2 3 3 2 3 3 3 2 3 3 3 2 3 3 3 3 2 3	.00 5 .00 5 .00 5 .00 5 .00 5 .00 5 .00 5	1.13 0.67 1.12 1.13 0.90 1.12 1.13 0.90 1.35	10 0.60 10 0.60 6 0.30 10 0.60 5 0.30 10 0.62 10 0.63 10 0.63 10 0.64	2 0.0 4 0.0 4 0.0 4 0.0 2 0.0 3 0.0 1 0.0	3 - 2 3 - 4 - 1 - 2 3	0.12 ½ 0.18 ½	0.05	3 1.02 4 1.36 4 1.36 3 3 1.02 - 3 1.02 2 0.68	1 0.0	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7.30 1 7.30 I 7.30 I 7.30 I 7.30 I 7.30 I 7.30 I 7.30 I 7.30 I 7.30 I	3.30 3.30 3.30 3.30 3.30 3.30 3.30 3.30	2 4.80 2 2 4.80 2	4.20 4.20 4.20 4.20 4.20 4.20 4.20 4.20	2 4.80 2 4.80	9 ⁴ 16.32 10° 18.36 12 20.40 12 20.40 12 20.40 12 20.40 12 20.45 8 ⁴ 14.28 13 ⁴ 22.36 12 20.40	5° 19'72 6' 20.74 5' 19:38 6' 20:74 6' 20:74 6' 20:74 5' 19:72 6' 30:74 5' 19:72 5' 20:06	8 0.34	8 0.12 - 8 0.12 -	=	0.07 8	0.11	255, 56 249,74 257, 57 258, 32 258, 50 249,03 257, 15 244,92 257, 88 253, 85	0.12 0.06 0.32 2.15 0.09 0.22 0.19 1.25 0.08	clay.			

APPENDIX E.-Continued.

PIER III.—Continued.

	_			-				Coffor						Coal for	Lineard	I Dan	Nurhi		Cnal	Prom	1.aborers	Co.ll for			Ţ.			0.103	Totals for Each Day.	Feet	Mate-	Water Pamps,	Air	
Date.	rincipal	a. Fore:	man, E	oremen.	Lock Tenders.	Pressu Men	re	Coffee house Men.	Coffee.	Sugar.	Candle	Red Lead	i. Wiek.	heating,	Oil	Engineers.	Engineer.	Firemen,	- :		1.aborers on Holst	Boilers.	Signal	Oil. Blac	k Oil 1	i mom	waste.	Cost Oil.	Each Day.	per day.	-		Pumps.	REMARKS.
ı88y.	Amount	Days.	Amount Days.	Amount	Days. Amount	Days.	Days.	Amoun	Атопи	Lbs.	No. Amoun	Lbs. Amoun	Lbs.	Boxes	Pints.	Days.	Days. Amoun	Атови	Amoun	Days.	Days	Tons	Pints	Pluts	Amour	Amour	Amour	Amour			117	ours No. of revolu- tions.	revolu- tions.	
Feb. 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28	6,00 6,00 6,00 6,00 6,00 6,00 6,00 6,00	1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3	1.57 6 6 1.57 5 5 1.57 6 6 1.57 6 1.5	19.50 19.50 17.87 16.25 9.75 19.50 19.50 19.50 1.63 8.12 19.50 17.88 16.25 16.25	1 2,50 2 5,00 2 5,00 1 2,50 	61 152 576 143 538 133 57 142 	.50 2 .75 2 .75 2 .50 2 .50 2 .25 1 .00 2 .25 3 .25 3 .25 3 .25 3 .25 3 .25 3 .25 2 .25 2 25 2	3.00 3.00 1.50 — — 3.00 3.00 3.00 3.00	5 1.13 4 0.90 3 0.68 2 0.45 4 0.90 4 0.90 5 1.12 4 0.90 3 0.67	5 0.32 5 0.32 5 0.32 5 0.32 10 0.63 10 0.63 5 0.32 10 0.63	3 0.0 - 3 0.0 - 5 0.0 2 0.0 3 0.0	4		1 0.34 4 1.36 		2 7.30 2 7.30 	1 3.30 1 3.30 1 3.30 1 3.30 1 3.30 1 3.30 1 3.30 1 3.30 1 3.30	2 4.80 2 4.80 1 2.40 1 2.40 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80	2 4.20 2 4.20 1 2.10 1 2 10 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20 2 4.20	2 4.80 2 4.80 1 2.40 1 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80	12 20.40 10° 18.36 12 20.40 12 20.40 12 20.40 13 20.40 14 20.40 17.47 18 20.40 10.20 10° 17.47 20.40 10° 20.40 10° 20.40 12° 20.40 12° 20.40 12° 20.40 12° 20.40 12° 20.40 12° 20.40 12° 20.40 12° 20.40	6 20.40 6 20.74 5 19.38 1 4.60 6 20.74 5 19.72 22.10 5 22.10 5 22.78	8 0	8 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	0.12 - 0.12 - 0.12 - 0.12 - 0.12 - 0.12 - 0.12 - 0.12 - 0.12 - 0.12 - 0.13 - 0.14 - 0.15 - 0.		0.07		252.48 248.73 236.38 239.83 4.79 142.44 236.37	1.11 0.19 1.30 0.00 0.05 0.04 0.08 1.23 0.00 0.00 1.33 0.15 0.59	66 68 66 60 60 60 60 60 60 60 60 60 60 60 60		27 24 26 25 21 25 26 26 24 23 26 25 26 26 26 26 26	Working in Pier II. Working in Pier II,
Mar. 1 2 3 4 4 5 6 7 8 8 9 9 10 11 12 12 12 12 12 12 12 12 12 12 12 12	1 6.00 1 1 6.00	0 1 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	1.23 6	19.59 14.62 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50 14.63 19.50	2 5.00 2 2 5.00 2 2 5.00 2 2 5.00 2 1 2.59 2 5.00 2 5 5.00 2 7 5.00 2 7 5.00 2 8 5.00 2 9 5.00 3 7 5.00 4 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7	66 1666 1666 1666 1666 1666 1666 1666	5.25 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	3.00	3 0.68 1 0.23 1	\$ 0.32 5 0.33 5 0.33 6 0.39 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6 0.30 6	12 O.1.1	5		1 0.34		2 7.58 2 7.30 	1 4.12 4.12 1 3.39 3.49 1 1 3.39 1 1 3.39 1 1 3.39 1 1 3.39 1 3.39 1 3.39 1 1 3.39 1	2 ¹ 5.00 2 4.80 2 4.80 3 4.83 2 4.83 2 2 4.80 2 2 4.80 1 3.60	2 4.37 4.30 4.30 4.30 4.30 4.30 4.30 4.30 4.30	2 4,80 4,80 1 2,00 1 2,00 1 1 2,00 1	13° 20.97	6 21.42 6 22.78 6 22.7	8 0	34 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8			0,07		239, 24 200, 79 62, 63 238, 24 246, 26 133, 20	0.01	clay		24 24 21	Work suspended. River breaking up.
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Other expenses charged to sinking . . \$1633.86 \$22458.98

APPENDIX E.-Continued.

TIME, COST AND MATERIALS USED IN FOUNDATIONS. PIER IV.

Date.	Principal Foreman.	Night Foreman.	Sub Foremen.	Lock Tenders.	Pressure Mea.	Coffee house Men.	Coffee.	Sngar.	Cand	lles. Red	Lend W	Cor ick. her	al for Lin	nseed Oil E	Dny Engineers.	Night Engineer,	Firemen.	Coal Passers,	Famp Men.	Laborers on boist	Coal for lieders,	Signal Oil.	Black Oil	Tallow.	Waste.	Coal Oil.	Totals for each day.	Feet sunk per	Mate-	Camero	on n	Air	- Comment
1886.	Amount	Amount.	Amount.	Days.	Days Amount.	Days.		Lbs. Amount.							-	1 3	Languart.	mount.	mount.	Days.	Tons.	Pints.	Pints.		L.bs.	Pints.		day.	-	Hours No		No. of revolu- tions,	Remarks.
Aug. 22 23 24 25 26 27 28 29 30 31	1 6.00 1 6.00 1 6.00 1 6.00 1 6.00 1 6.00 1 6.00	1 3.23 1 3.23 1 3.23	3 9.00 3 9.00 3 9.00 3 9.00 3 9.00 3 9.00 3 9.00	2 4.50 2 4.50 2 4.50 2 4.50	32 ^b 73.12 32 72.00 29 65.75 31 ^f 70.88 34 76.50 34 76.50 34 76.50 33 ^f 75.37 36 81.00 33 ^f 75.38	2 3.2 2 3.2 2 3.2 2 3.2 2 3.2 2 3.2 2 3.2 2 3.2 2 3.2	2 0.28 4 0.56 4 0.56 4 0.56 3 0.42 4 0.56 3 0.42 6 0.84 3 0.42	3½ 0.20 5 0.31 5 0.31 4 0.25 5 0.31 5 0.31 8 0.50 3½ 0.22 8 0.50 4 0.25	13 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	0.16 40 0.05 - - 0.07 - 0.08 - 0.04 - 0.07 - 0.07 - 0.07 - 0.07 -	2.40 -	2 2 2	- 16 0.63 - 0.63 - 0.63 -	1.00 2 -	2 7.30 1 2 7.30 1 3 7.30 1	3.30 3.30 3.30 3.30 3.30 3.30 3.30 3.30	1 2.40 2 4.80 3 4.80 2 4.80 2 4.80	1 2.10 1 2 4.20 2 2 4.20 2 2 4.20 2 2 4.20 2 2 4.20 2 2 4.20 2 2 4.20 2	1 2.40 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80 2 4.80		3 ³ 111.90 3 10.20 6 ⁶ 22.10 7 23.80 7 23.80 7 24.82 7 23.80 7 23.80 7 23.80	8 0.34	8 0,11	10 0.65	- 4	8 0.11	(30.87 134-33 139-91 146.63 152-26 153-95 151.07 157-29 148.23	1.15 1.51 1.48 1.77 0.17 3.21	gumbo s	20%/10 1234 13/4 121/6 11/4 13/4	32 33 37 41 40 38 40 41	30	Started air pumps at 7:05 a. m. Started Cameron pumps at 7:30 a. m.
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APPENDIX F.

SPECIFICATIONS FOR SUPERSTRUCTURE.

GENERAL DESCRIPTION

The superstructure will consist of three main through spans and six deck spans three of which will be at each end of the structure.

Each through span will be 375 feet long between centers of end pus, divided into fifteen panels of 25 feet ach. The trusts will be 50 feet deep and placed 22 feet apart between centers. The trup chord, end posts, bolsters, policis, bearing plates, pins and all eye have sevept counters and vertical suspenders, will be of steel; all other parts will be of wrought from except the wall plate pedestals and ornamental work, which will be cast. Each span will contain approximately 486,000 pounds of steel, 484,000 pounds of wrought from at 24,000 pounds of east ron.

Each deck span will be 13 feet long between centers of end pins, divided into five pank is of 35 feet canch. The trusts will be 7 feet to inches deep and placed 15 feet apart between centurs. The intermediate ends of each set of three spans will rest on iron towers measuring 25 feet long in the direction of the bridge, making a total length of iron work in each group of three deck spans of 425 feet. The pins, rollers, bearing plates and eye bears, excepting constres, will be of set, all other parts will be of wrought from except the vall plate pedicatals, which will be of east iron. Each span will contain approximately 20,000 pounds of wrought iron and 15,000 pounds of east iron.

The total estimated weight of the entire structure is approximately 4,000,000 pounds.

PLANS.

Full detail plans, showing all dimensions, will be furnished by the engineer. The work shall be built in all respects according to these plans. The contractor, however, will be expected to verify the correctness of the plans, and will be required to make any changes in the work which are necessitated by ercors in these plans, without extra charge, where such errors could be discovered by an inspection of the plans.

MATERIALS.

All materials shall be subject to inspection at all times during their manufacture, add the engineer and his inspectors shall be allowed free access to any of the works in which any postum of the material is made. Timely notice shall be given to the engineer so that his inspectors may be on band.

Steel.—The Steel used will be of two classes, viz.: High Steel, which will be used in compression members, bolsters, bearing plates, pure and rollers, and Low Steel, which will be used for tension members and rivets.

Steel may be made by the open hearth or by the Bessener process, but no steel shall be made at works which have not been in successful opention for at least one year; steel made by the Clapp-Griffiths process will not be accepted. All mules shall be made from uniform sock low in phosphorus, and the manificaturer shall furnish satisfactory evidence to the engineer that this class of material is being employed, it being moderatood that the fornished product is to be one in which the phosphorus does not average more than 8-too of one per cent, and not exceeded 1-to of not per cent.

A sample bar 3f of an inch in dismeter shall be rolled from every melt, the method of obtaining the piece from which this sample bar is rolled shall be the same for all samples, and the amount of work on this sample bar shall be as nearly as practicable the same as on the finished product. The laboratory tests shall be made on this sample bar in its natural state without annealing. The laboratory test of High Steel made on the sample bar shall show an elastic limit of not less than 50,000 pounds per square inch, an ultimate strength of not less than 80,000 pounds nor more than 9,000 pounds, an elongation of at least 15 per cent, in eight inches and a reduced area of at least 35 per cent, at the point of fracture. The sample bars shall bend 186° around its own dismetts without showing crack or flaw.

The laboratory tests of Low Steel made on the sample has shall show an elastic limit of not less than 40,000 pounds per square inch, an ultimate strength of not less than 70,000 pounds nor more than 80,000 pounds, an elongation of at least 18 per cent. in a length of eight unches, and a reduction of at least 42 per cent. at the point of fracture. In a bending test the sample har shall bend 180° and close back against itself without showing crack or flow on the outside of the curve.

The softest melts shall be selected for rivers, the only requirement as to elastic limit and ultimate strength will be that the ultimate strength shall be at least 60,000 pounds per square inch.

Facilities for testing sample horsshall be furnished by the contractor at a point convenient to the steel works, and the tests shall be made at the express of the contractor and under the direction of the languager. Tests may also be made from time to time on samples cut from finished plates, shapes and bars which shall slaw results substantially conforming to those shown by the sample tests of the same melts.

All sheared edges or punched holes in steel work shall be subsequently planed or drilled out, so that none of the rough surface is ever left upon the work. Steel for pins shall be sound and entirely free from pining.

Wrought From.—The iron used in reusion members shall be double refined thigh test) iron; muck basis may be used at the center of the pile, but shall not constitute more than one-third of the total pile. Small samples having a minimum length of eight inclus, shall be furnished by the contractor for testing, as directed by the engineer, these samples shall above an elseute limit of at least 26,000 pounds, and an ultimate strength of at least 26,000 pounds per square fich, shall clongrae at least 15 per cent, and show a reduced zero, of at least 25 per cent, at the point of fractine. The fracture shall be of miliform fibrous character, free from crystalline appearances.

Simil simples, having a minimum length of eight inches, shall be furnished by the contractor from the iron need in shapes, places and other miscellaneous forms as directed by the Engineer, these samples will show an clastic limit of at least 4,000 pounds, pard an ultilimate strength of at least 4,000 pounds per square inch, shall elongate at least 10 per cent. before breaking, and show a reduction of area of at least 15 per cent, at the point of fracture. In places more than threty inches wide an clongation of 8 per cent, and a reduction of 12 per cent, at the point of fracture will be considered satisfactory.

Cast iron shall be of the best quality of tough, grey iron.

RIVETED WORK,

All plates, angles and channels shall be carefully straightened before they are laid ont, the river holes shall be carefully spaced in truly straight lines, the river heads shall be of hemispherical pattern and the work shall be finished in a next and workman-like manner. Surfaces in contact shall be painted before they are put together. The dimensions given for rivers on the plans are the diameters of the rivers before driving.

Power riveters shall be direct acting machines, capable of exerting a yielding pressure and holding on to the rivet when the apsetting is completed.

Steel.—The several parts of each steel member shall be assembled and the holes shall be drilled, the sharp edge of the drilled hole shall be trimmed so as to make a nlight filler under the rivet head, and the pieces shall be riveted together without taking apart. Should the contractors desire, the parts may be punched with holes not exceeding 4-5 the diameter of the finished hole, and this punching shall be so agecurate that at least $^{3}/_{10}$ of an inch of metal is taken out all around in drilling the hole. All rivers in steel members shall be of week the river holes shall be of such size that they will fill the hole before driving, and whenever possible the rivers shall be driven by power. All bearing surfaces shall be rively faced. The chord pieces shall be fitted together in the shop is lengths of at least five panels and marked, when so fitted there shall be no perceptible wind in the length hidd out. The pin-holes shall be bored truly so as to be at exact distances, parallel with one another, and at right angles to the axis of the member,

Wrought from —All wrought-tron shall be punched accurately with holes \(\frac{t}{t}_0 \) of an inch larger than the size of the rivet, and when put together a cold river shall pass through every hole without reaming. So far as possible all rivers shall be driven by power. The holes for the rivers connecting the flow-brans with the posts and bolsters and the stringers with the flow-beams, and, in general, the holes for all rivers which must be driven after erection, shall be accurately drilled to an iron templet. The holes for the rivers connecting the flow-beams with the posts shall be turchin distincter, and therivers of corresponding dismeter. The pass holes in the vertical posts shall be tuly parallel with one another, and at right andets to the axes of the posts. The posts shall be strucht and free from wind.

FORGED WORK

The heads of the steel eye-bars shall be formed by upsetting and forging into shape by such process as many accepted by the Engineer. No welds will be allowed. After the working is cumpleted, the bars shall be annealed by heating them to a underpr adult red not throughout their enture length and allowing them to cool slowly. The form of the heads of steel eye-bars may be modified to suit the process in use at the contractor's works, but the form of head adopted unust be such as to meet the requirements of the tests of full cloud bars.

The heads and the enlarged ends for screws in laterals, suspenders and counters shall be formed by uptiming or by an upsetting and welding process acceptable to the engineer. Welds in the body of the bar will not be allowed,

TESTS OF FULL SIZED BARS.

Ten full-sized steel eye-bras of sections and lengths used in the actual work shall be selected by the inspector for testings each of these full-sized bars shall be strained till an elongation of to per cent, is obtained, and if possible broken; if broken, the fracture shall occur in the body of the bar and shall show a uniform and doubtle mather of exterior.

The contractor will be required to furnish facilities for testing the full-sized bars within a reasonable distance of his works. Should the contractor be unable to furnish such facilities, he shall be required to furnish has at 20 per cent, larger section than those called for, without charge to the increased weight.

The full-sized bars shall be selected from time to time as the work proceeds, the last has not to be selected ill all the eye-bars are manufactured. The tests shall be made from time to time as the bars are selected. When three bars have been tested, the bars manufactured up to the time of the selection of these three test bars shall be accepted or rejected as the results of such tests, and the same shall be done again when three more bars are tested. In the tests, the failter of one bar to develop a stretch of 10 per cent, before breaking, shall be sufficient reason for rejecting the whole lot; bat a failure to break in the body of the bar shall nat be a sufficient ground for condemnation if it does not occur in more than one-third of the bars tested. Should the contractor on the first attempt fall to make bars coming up to the required specifications, the engineer may order bars of 20 per cent. Larger section than the plans called for, to be furnished by the contractor whose the section of the increased weight.

MACHINE WORK.

The bearing surfaces in the top chord shall be truly faced. The ends of the stringers and of the floorbearins shall be squared in a rotary facer. All surfaces so designated on the plans shall be planed. All pins shall be accurately turned to a gauge, and shall be of full size throughout, pin holes shall be bored to fit the pins with a play not exceeding 1.50 of an inch. These clauses apply to all lateral connections as well as those of the main trusses. Pins shall be supplied with pilot nuts for use during erection, four for each size of pin.

All screws shall have a truncated V thread, United States standard sizes,

MISCELLANEOUS.

All workmaship and material, whether particularly specified or not, must be of the best kind now in use instellars bridge work. Flaws, ragged edges, surface imperfections, or irregular shapes, will be sufficient ground for rejection, rough and irregularly finished work will not be accepted.

Machine-finished surfaces shall be coated with white lead and tallow before shipment; all other ports shall be given a coat of hit boiled linseed oil.

TERMS.

Monthly estimates will be made at the end of each month for the work done during that month. In these manufactury skinners, the material delivered at the contractor's shop, but not manufactured, shall be estimated at 50 per cent, of the contract price for finished material in Chacago, and manufactured material at 75 per cent, of the contract price for finished material in Chicago. Payments will be made on or about the 15th day of the following month, according to these estimates, deducting from the amount of the same ten per cent, as security, to be held until the completion of the entire contract.

No material will be paid for which does not form a part of the permanent structure.

All expense of testing shall be borne by the contractor.

TIME

The deck spans and towers shall be completed and shipped not later than January 1st, 1887. The three through spans shall be completed and shipped in February, March and June, 1887, respectively.

The rathroad company may exact a penalty not exceeding \$150 per day for failure to complete the work within these specified times.

PROPOSALS.

Separate proposals should be made for the deck spans (including towers) and the through spans,

The prices should be by the pound at separate rates for sted, awought from and east run. The prices shall material, and all patterns and uther work of every description, and are to be on the basis of finished material delivered on cars at Chengo. Separate proposals shift also be made for erection on the basis of a single gross sum for the erection of the six deck spans (including towers), and a single gross sum for the erection of the six deck spans (including towers), and a single gross sum for the erection of the three through spans. Erection will include setting wall paths and defilling the necessary holes for ancher belts. The contractors will be required to furnish all false work and tools of every description, and the plans of such false work shall be subject to the approal of the engineer.

The right is reserved to accept separate proposals for the deck spans and the through spans, to accept proposals for material without crection and to award the contract to other than the lowest bidder.

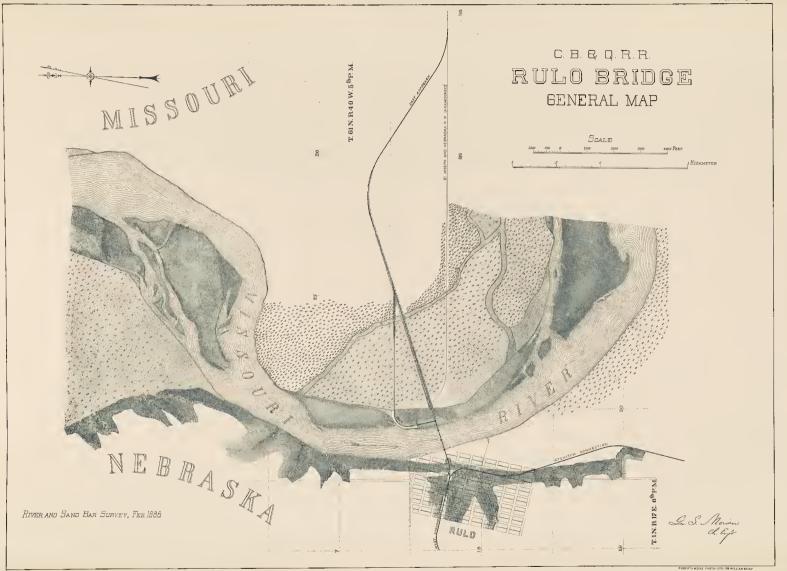
GEO. S. MORISON, Chief Engineer Rulo Bridge.

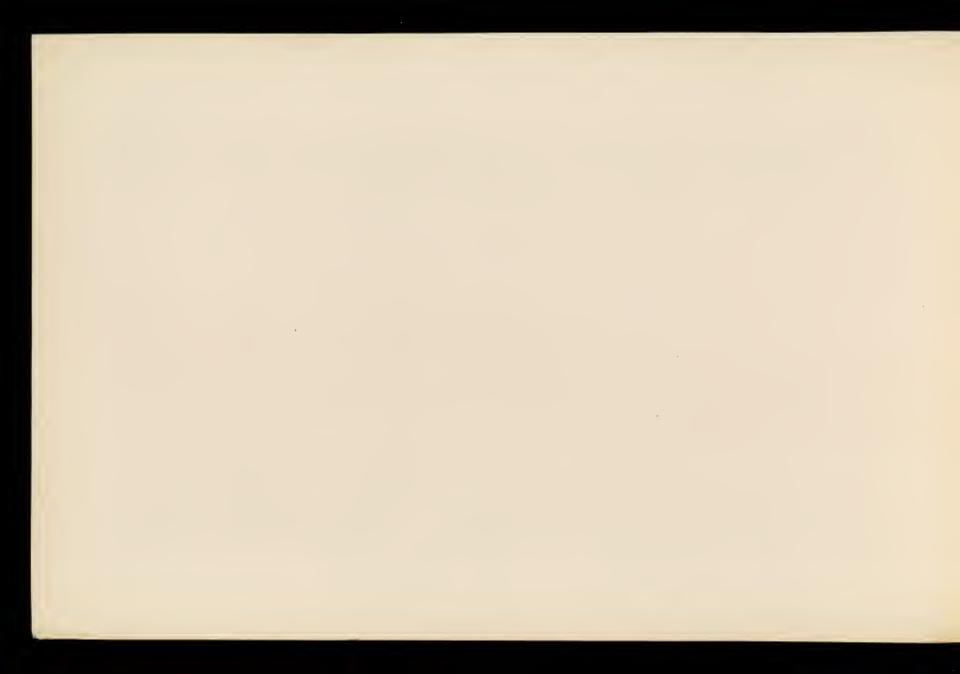
NEW YORK, JULY 20, 1886.

APPENDIX G.

TESTS OF STEEL EYE BARS.

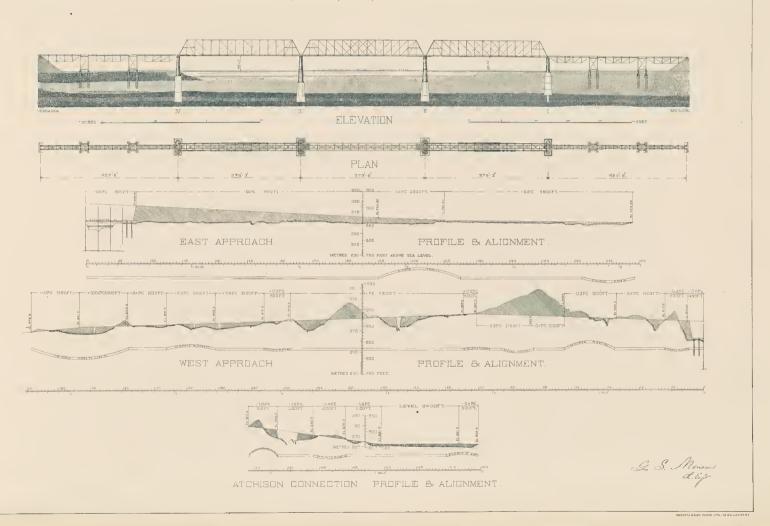
				TESTS (ON FUI	L-SIZED	TESTS ON SAMPLE BARS FROM SAME MELTS.													
		DIM	exsions,-	YNOHRS,			Б	ESULTS O	P MEGHAN	ical Test	18.		Diam	ETERS-						
		Origina	1.		Afron	Testing.		Evic	usion	on Blastic A		Place of			Reduction Per cent.	Extension in 8 m.	Elastic Limit. Liss per sq in.	Maximum Load Lbs. per sq. in.	rer cent-	Melt Number.
Nom	inal		Actual.		Attor	Testing.	Reduction of Area Per Cent.	Guaged	Sunged Leugth Fercent.		Maximum Load. Lbs per		Original. Inches	Testing Inches		Per cent,				
Vidth.	Thick- ness.	Length c. to c.	Width.	Thickness	Width.	Thickness.		Inches		sq. in.	sq. in									
7	34	300.03	6.97	0.76	_		-	264	_	34270	60450	Head	.739	- 545	45.6	22.80	47790	74840	,090	8485
7	I	300.03	6.97	1.01	5.51	0.74	42.08	264	13.77	35350	67230	Body	.750	-535	49.1	25.30	44140	73330	.090	8847
7	1 1/2	300.03	6.97	1.52	5.37	0.87	55.90	264	14.75	31570	65262	46	.750	. 530	50.1	26.75	47980	75140	120.	8415
7	134	300.03	6.97	1.76		_		26.1	-	30925	60020	Head	-743	-475	59.1	26, 30	45210	71040	.086	8345
7	I	300.03	6.97	1,01	5.46	0.64	50.36	264	13.67	35350	67800	Body	.750	535	49.1	25.30	44140	73330	, aga	8847
7	1 55	300.03	6.97	1.50	6,60	1.39	12.25	264	9.01	35120	67110	и	-739	. 510	52 4	24.70	47570	74610	.068	8352
7	15%	424.10	6.98	1.62	5.23	0.98	54.67	396	12.11	36070	67100	1.6	, 740	- 545	45.8	33.00	46040	75340	.076	8339
7	1%	424.10	7.00	1.61	5 - 57	1.22	39.70	396	11.43	33120	63340	et.	.741	.486	57.0	24.60	45460	74210	.075	8330
7	134	424.09	6,98	1.26	5.85	1.02	33.15	396	11.24	32930	6,1240	ěč.	-749	- 550	46.1	23.00	45170	758oo	.048	8419
5	34	422.97	5.01	0.76	4.00	0.54	43.27	396	8.65	38940	61340	u	.748	.510	53 · 5	24.37	48240	76690	. 062	836;
5	34	422.72	5.00	0.77	3.70	0.48	53.87	31/6	9.78	42365	69325	и	.746	.510	53 - 3	24.80	44390	71380	076	8223
7	134	300.03	7.00	1.75	5,20	1.23	47.62	26,1	13.37	33300	63270	čt	.758	. 560	45-4	24. 10	45200	72680	-	9303
7	34	399.61	6.97	.78	5.41	0.53	47.51	264	13.32	40140	67530	66	-755	- 535	49.8	23.30	46680	74370	. 086	9040





c.e.e q.e.e. RULO BRIDGE

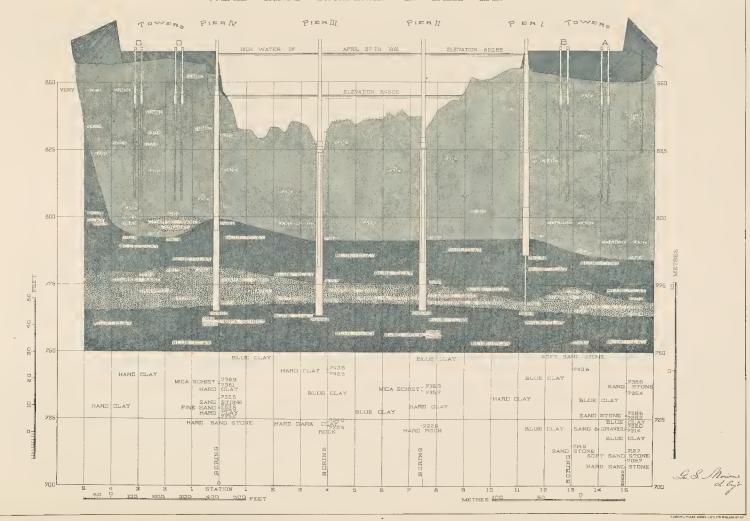
CENERAL ELEVATION, PLAN, PROFILE & ALIGNMENT



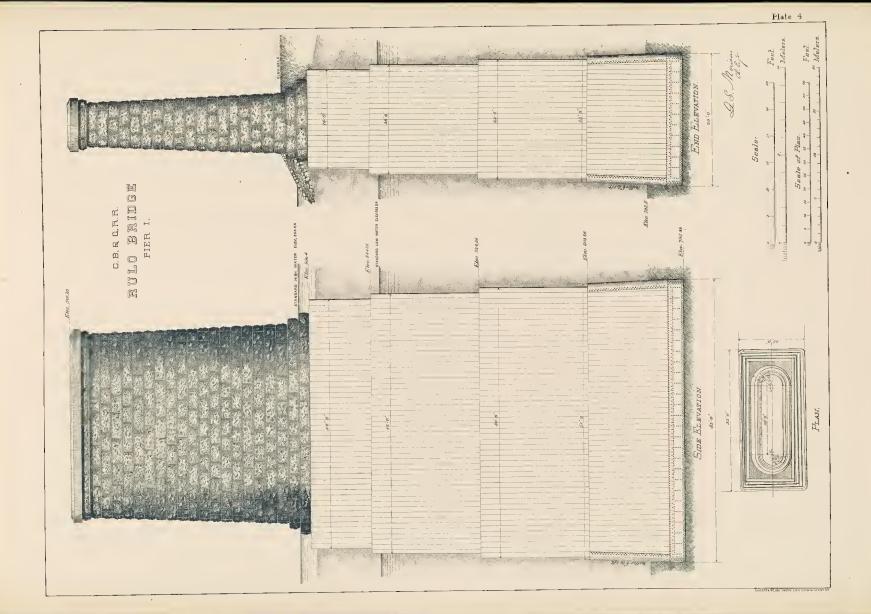


C.B. & Q.R.R. RULO BRIDGE

PROFILE SHOWING STRATIFICATION ON BRIDGE LINE.











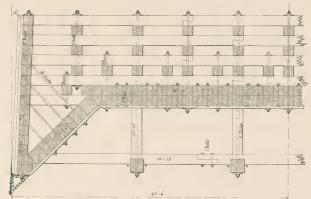




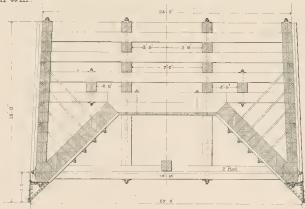
c. e. e. q. r. r. RULO BRIDGE

CAISSONS II & III.

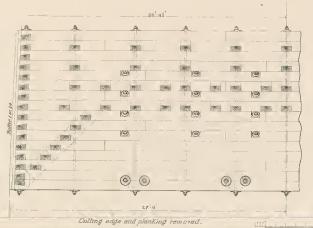
Scalà.



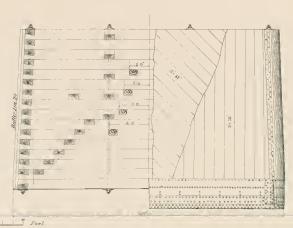
HALF LONGITUDINAL SECTION.



CROSS SECTION.



HALF SIDE ELEVATION.



END ELEVATION.

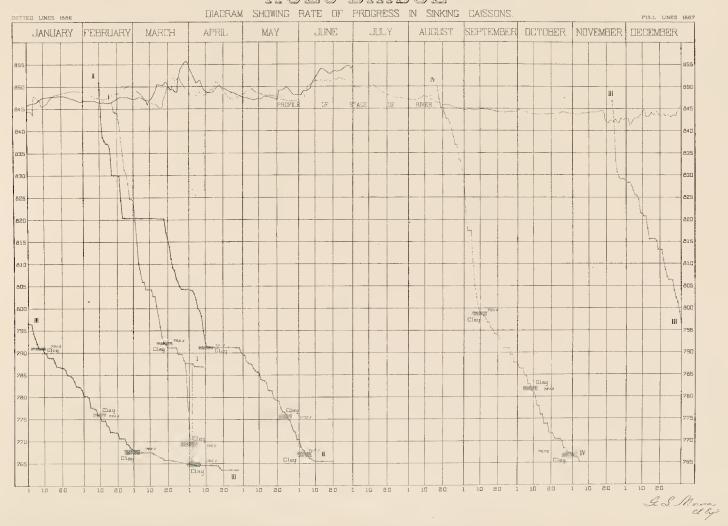
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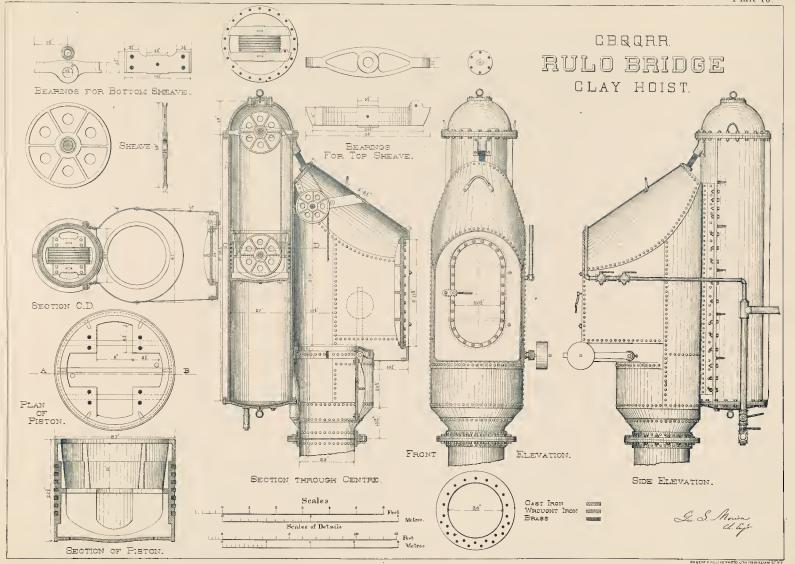


ROBERTA WELCHE PHOTO LITH ITS WILLIAM ST N V

c.b.g q.a.a. RULO BRIDGE



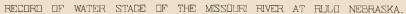


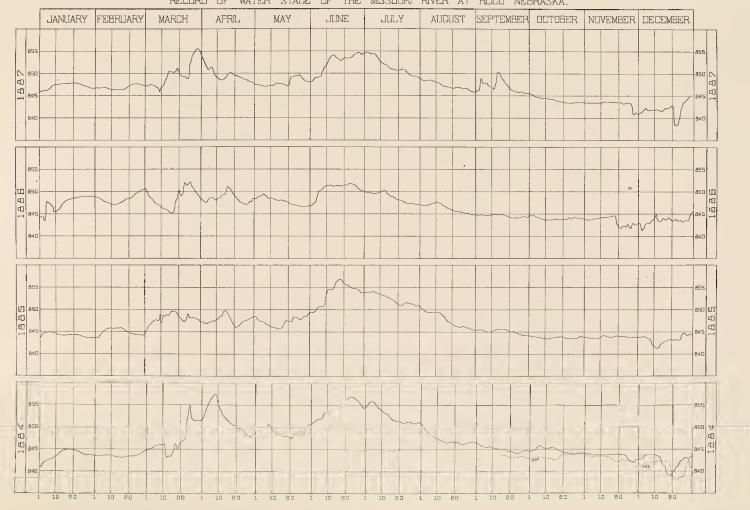




c.b.g q.r.r. RULO BRIDGE

La. S Morsons Cl. Eyer





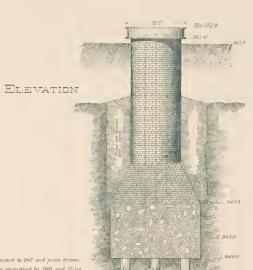


- Filled with Concrete when in place, before topplate was put on

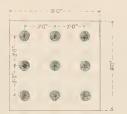
Plate 6'0"diam

C.B. & Q.R.R.

RULO BRIDGE



Excavated to 845 and piles driven. Then excavated to 840 and Piles cut all at 842 Piles driven generally to 810



PLAN OF PILING

APPROACH PIERS



Scale of Cross Section

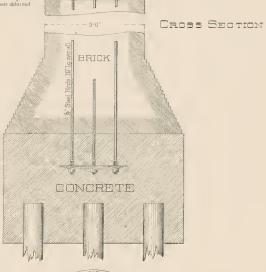
ANNULAR CAST WASHER



PLAN



Scale of Washer





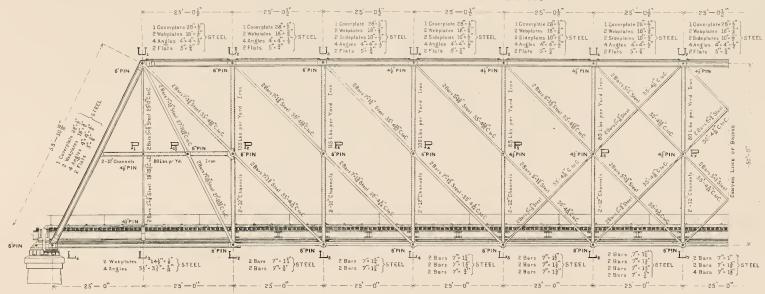
G. S. Morison Ch Enjs

Top View

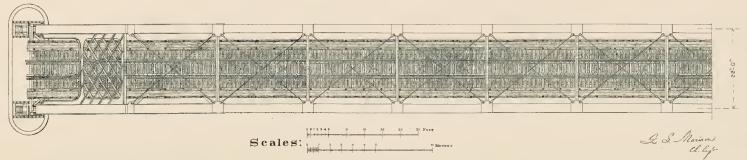


C.B. & Q. R.R. RULO BRIDGE THROUGH SPAN

GENERAL ELEVATION OF 375-0 SPAN



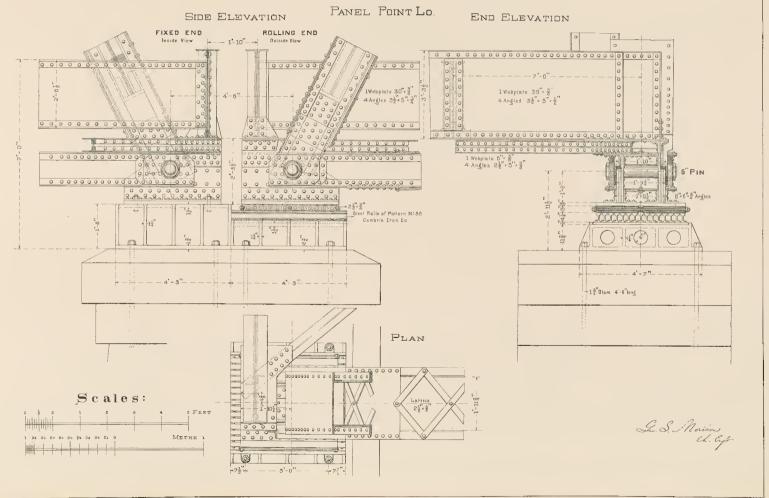
GENERAL PLAN OF 375-0 SPAN



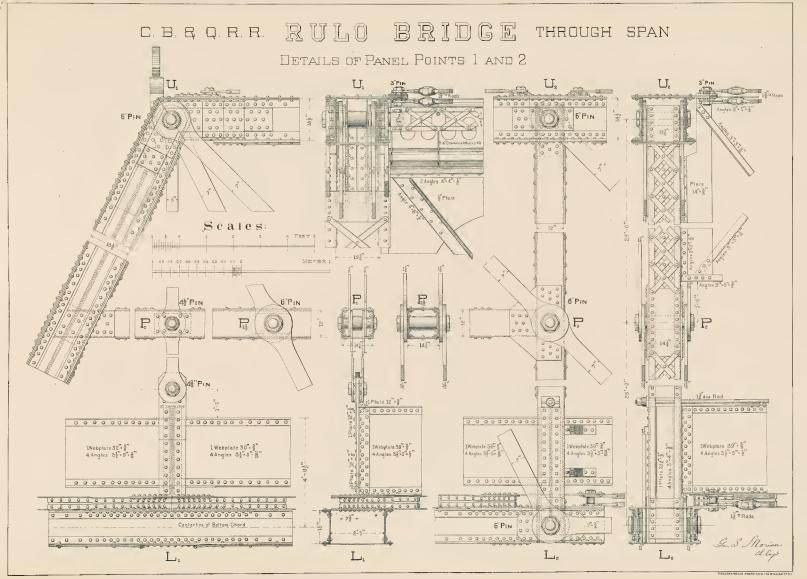


C. B. Q Q. R.R. RULO BRIDGE THROUGH SPAN

DETAILS OF 375-0"Span







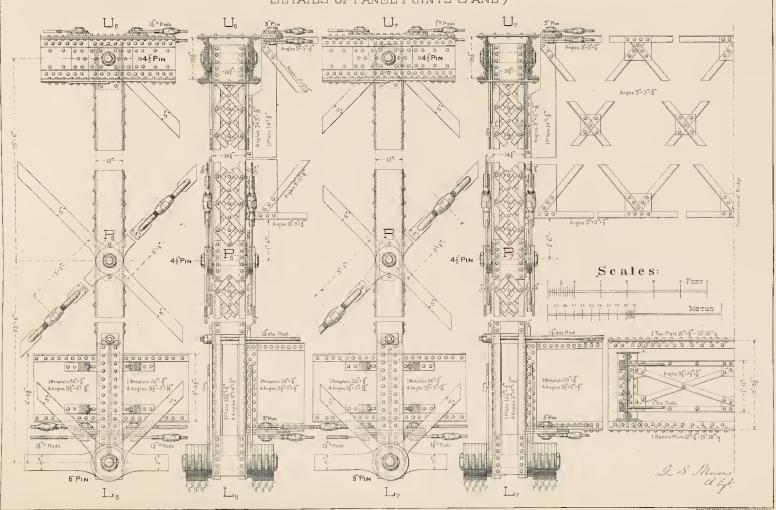


C.B.QQR.R.R.RULO BRIDGE THROUGH SPAN DETAILS OF PANEL POINTS 3,4 AND 5 Scales



C. B. & Q. R. R. RULO BRIDGE THROUGH SPAN

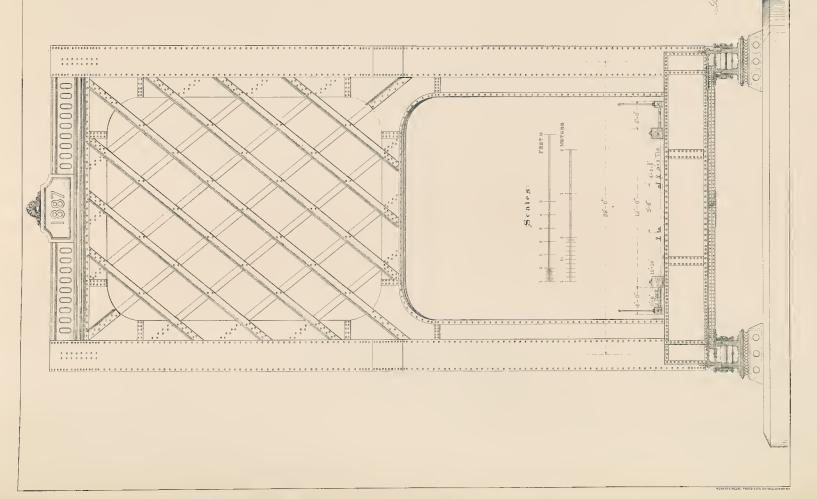
DETAILS OF PANEL POINTS 6 AND 7





C.B.R.Q.R.R. RULO BRIDGE THROUGH SPAN.

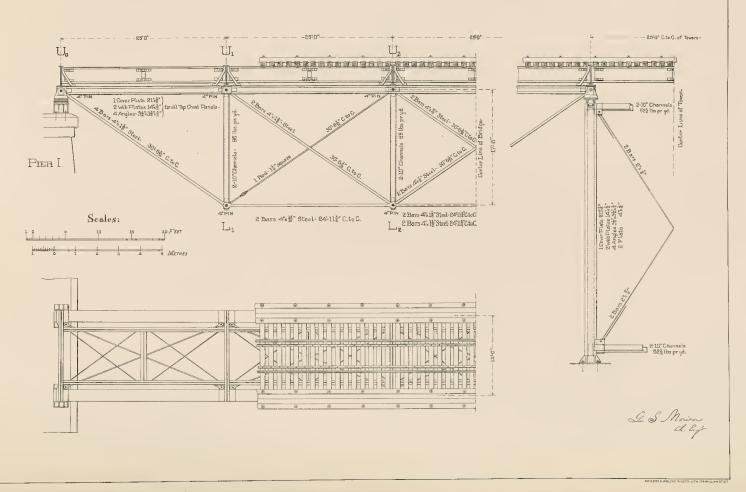
END ELEVATION.





C.B.Q.R.R. RULO BRIDGE 125F DECK SPAN

GENERAL ELEVATION AND PLAN





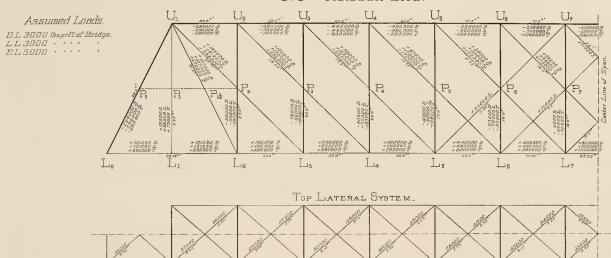
BRIDGE C.B.RQ.R.R. RULO 125 FT DECK SPAN DETAILS OF PANEL POINTS O, 1 AND 2 \bigcup_{0} 0 0 ROLLING END 0 0 1 Webplate 30% 3" 4 Angles 32 % 5 % 16" End View SIDE ELEVATION. 2 Bars 4"x 12" Steel 2 Bars 4"x 12" Steel PLAN OF ROLLING END. PLAN OF BOTTOM CHORD. Scales: 315 Pin



L. S. Morios Септет Line 522 lbs p.yd 2-10°Channels 52½ihspyd 125**DECK SPAN SECTION Choss Latince 24 x 3 Seales 2-10 Channels 5221bsp.yd 2 10 Channels - 10F" BRIDGE E*CestalqralliTS F*C*CestpnAS 00000000 Lathce 24", 3 SUPPORTING TOWER - <u>8</u>7 - 2эпід тэйпэЭ 6 3 Pin 3"Pin RULO ELEVATION 0000 0000 2-10°Channeis 522 bs.pyd IWebplate 30元音 4Angles 3秒×5×時 C.B.R.Q.R.R. SIDE 25-0"Or to Or of Floor -d5 2b-

C.B. R. Q.R.R. RULO BRIDGE STRAIN SHEET

375 THROUGH SPAN

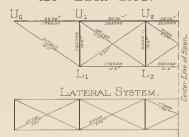


BOTTOM LATERAL SYSTEM.

125 FT. DECK SPAN

5000 lbs prft. all moving load.

500 lbs pr ft wind pressure



L. S. Morison CL. E.

